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TECK COAL LIMITED FORDING RIVER OPERATIONS

2017 Dam Safety Inspection for North Tailings Pond and South Tailings Pond

Submitted to: Teck Coal Limited Fording River Operations PO Box 100 Elkford, BC V0B 1H0

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REPORT



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Executive Summary

This report presents the 2017 annual dam safety inspection (DSI) for the North Tailings Pond (NTP) and South Tailings Pond (STP) facilities at the Teck Coal Limited, Fording River Operations (FRO) mine site, located near Elkford, British Columbia. This report was prepared based on a site visit carried out by Golder Associates Ltd. (Golder) from 3 to 5 October 2017, discussion with FRO staff, and a review of data provided by FRO. The reporting period for the data review is from September 2016 to September 2017, unless otherwise noted. The dam inspection reports and photographs from the site visit are presented with this report. The DSI report was prepared in accordance with Part 10 of the Health, Safety and Reclamation Code (HSRC) for Mines in British Columbia (MEM 2017), which sets out the frequency for inspection of dams and tailings storage facilities.

John Cunning, P.Eng., of Golder is the Engineer of Record for the NTP and STP dams. Kerr Wood Leidal Associates Ltd. (KWL) has historically been the Designer of Record for hydraulics-related works and has completed the most recent Fording River hydraulics assessment, including design for erosion protection along the dam toes following the 2013 flood event. Golder, FRO, and KWL are in the process of clarifying and documenting the roles of the Engineers of Record for the geotechnical and hydraulics-related works. The annual riprap inspection report by KWL is appended.

Summary of Facility Description

The FRO site is an open pit coal mine located near Elkford, BC. FRO's tailings storage infrastructure includes two tailings pond facilities: the NTP and the STP. The NTP has been essentially filled to its design capacity and is currently inactive. Tailings discharge from the wash plant is currently directed to the STP.

The NTP is a zoned earth fill dam located on the west side of a realigned reach of the Fording River across from the wash plant. The NTP was developed on a segment of the Fording River flood plain and has a surface area of approximately 32 ha and a minimum crest elevation of 1,652.6 m.

The STP facility is located south of the wash plant, on the east side of a realigned reach of the Fording River, and occupies a total area of approximately 67 ha and a minimum crest elevation of 1,637.8 m. The STP was developed on the flood plain of the Fording River.

Summary of Key Hazards

The key hazards for the NTP and the STP facilities are as follows:

- internal erosion (suffusion and piping)
- overtopping
- instability
 - static
 - seismic
 - erosion of toe from the Fording River





Dam Classification

Both the NTP and STP dams meet the definition of a "dam" as defined in the HSRC (MEM 2017).

Both of the dams are classified as Very High consequence, following the dam consequence classification guidelines from HSRC Guidance Document Section 3.4 (MEM 2016), which references the Canadian Dam Association (CDA) *Dam Safety Guidelines* (CDA 2013). The updated consequence classification requires a reassessment of the seismic stability and the freeboard (based on the inflow design flood). This reassessment is in progress at the time of writing.

An incremental inundation assessment was completed to assess the consequence of failure of the NTP and STP during a major flood event of the Fording River (Golder 2017g). The assessment concluded that the consequence of a failure occurring coincident with a major river flood event was High. A risk-informed assessment, which would be supported by a design level flood-induced dam break and inundation assessment, is recommended to determine the appropriate criteria for the flood protection requirements along the downstream toes of the NTP and STP dams.

Summary of Significant Changes, Changes to Instrumentation, Stability, and Surface Water Control

A probable maximum flood assessment was completed for the Fording River watershed above the NTP and STP facilities by KWL (2017b). The probable maximum flood peak Fording River flow estimate is 790 m³/s for the Fording River watershed with consideration of planned future mining activities. Additional details are available in the KWL report (2017b).

North Tailings Pond

The Liverpool outlet channel and fish barrier at the north abutment area of the NTP facility were completed in late 2016. They are not considered part of the NTP facility, but as part of the Liverpool Sediment Pond system. A figure outlining the NTP facility boundary has been prepared and should be included in the OMS for each facility. Discussions between Engineers of Record for each facility and FRO should be held to review the boundary.

Riprap upgrade works to accommodate the revised Q200 design flow plus 0.5 m were completed in 2017 at the NTP under the direction of KWL. In 2017, the setback riprap between Sta. 0+165 and 0+205 was constructed to complete the riprap revetment and the riprap elevation between Sta. 0+205 and 0+930 was increased to meet the 2016 design basis, however, there remains areas that are about 0.1 to 0.2 m below the design elevation. Riprap has been placed along a section approximately 1,030 m long, from the Fording River Multiplate haul road along the Fording River to approximately Sta. 1+080 (KWL 2017c).

There were no significant changes in visual monitoring records, instrumentation, dam stability, or surface water control for the NTP since the 2016 DSI. Draft quantitative performance objectives (QPOs) for the inclinometers have been developed.



South Tailings Pond

There was a potential tailings water discharge event in late June 2017 when FRO Environmental personnel noticed water seeping back into the STP dam in the area of the north abutment. Water was seeping into an incomplete section of the dam embankment, presumably into granular fill around the FortisBC gas line. No seepage was observed exiting the downstream slope of the north abutment and no soft spots were found on the crest of the north abutment. A sandbag berm was constructed near the STP discharge as a temporary measure to exclude tailings water from the area, and a till berm was later constructed directly behind the sandbag berm (Golder 2017e). Golder does not characterize this event as a "spill" as no tailings water was discharged to the environment, although FRO personnel pre-emptively reported to Emergency Management BC as a precautionary measure.

The berms along the West Dam of the STP's lower access road were repaired in August 2017 in the area adjacent to the Fording River bank. The ditches along the base of the West Dam were also cleaned out and regraded.

Riprap was placed along the upstream slope of STP between Sta. 0+700 and 1+700 from 20 to 29 September 2017.

An emergency riprap stockpile is typically maintained at the south end of the STP with an approximate volume of 4,500 m³. The stockpile was removed to be used in a rock drain project for the Swift expansion. The riprap stockpile was replaced with better quality material in late 2017. The removal of the stockpile was completed in consultation with the tailings engineer and replaced within a timely manner.

There were no significant changes in visual monitoring records, instrumentation, dam stability, or surface water control for the STP since the 2016 DSI. Draft QPOs for the inclinometers have been developed.

Review of Operation, Maintenance, and Surveillance Manual

The most recent update of the operation, maintenance, and surveillance (OMS) manual was completed by FRO in 2015 (FRO 2015b). As required by HSRC Section 10.5.2 (MEM 2017), FRO is currently completing a review and update of the OMS manual for the NTP and STP which is expected to be issued in Q2 2018.

Review of Emergency Preparedness Plan and Emergency Response Plan Manuals

An update to the 2015 version of the emergency response plan (ERP) is in draft for the NTP and STP (SP&P EP.009) (FRO 2017a). The updated ERP now includes the in pit tailings storage facilities on site. This document was updated to meet the guidelines provided by the HSRC (MEM 2016, 2017), the CDA (2013), the Mining Association of Canada (MAC 2011, 2017), and Teck Resources Limited guidelines (Teck 2014).

FRO has also developed a *Tailings Impoundment Flood Response Protocol for the Fording River*. This document was issued on 26 September 2017 (FRO 2017c).

The emergency preparedness plan (EP.008.R1) was last updated on 15 December 2015 (FRO 2015a). FRO plans to update the emergency preparedness plan once the ERP is finalized as the document will outline the warnings FRO will issue and the expected actions of local authorities and other responders for dam breach flood emergencies.





The emergency planning documents should continue to be reviewed at least annually, with updates incorporated when required. The ERP should be tested annually. FRO plans to tabletop test the ERP in 2018.

Dam Safety Review

The most recent dam safety review (DSR) of the NTP and STP was completed in 2014 (KCB 2014). A DSR is required every five years for all water and tailings storage facilities regardless of dam consequence classification according to HSRC Section 10.5.4 (MEM 2017). The next DSR is required in 2019.

Annual Dam Inspection

The NTP and STP facilities were observed to be in good condition at the time of the 2017 annual inspection.

Status of 2016 Dam Safety Inspection Recommended Actions

A number of recommended actions were prepared as part of the 2016 annual DSI (Golder 2017b). A summary of the status of the 2016 annual DSI recommended actions is presented in Table E-1. Recommendations that are noted as complete can be closed out. Items from the 2016 DSI that are incomplete have been brought forward into the 2017 DSI recommendations (Table E-2).

There are a number of recommendations that are in progress and some that are incomplete, however Golder feels the work is being appropriately prioritized based on good communication between the EoR team and the FRO tailings engineer.





Table E-1: Current Status of 2016 Dam Safety Inspection Recommendations for the North Tailings Pond and South Tailings Pond

Facility	ID Number	Deficiency or Non-conformance	Recommended Action
		No passive emergency system against overtopping;	Assess the need for spillway after finalizing the closure plan NTP.
	2015-05a,b	emergency system requires active response	If required, determine a construction schedule.
	2015-06a,b,c		Perform risk-informed assessment to determine appropriate flood protection requirements for downstream toe of dam along Fording River and timeline to implement
		Risk-informed criteria for flood erosion protection along toe of dams not defined	Implement required protection measures for the operational phase according to the as- defined schedule.
			Execute the flood risk mitigation plan until the flood protection requirements defined by the risk-informed assessment are in place.
	2015-07a,b	Buried pipes passing through crest locations	Complete review of pipe abandonment timeline as part of feasibility investigation into NTP decommissioning.
NTP			If required, execute abandonment plan.
		North abutment excavated without input or approvals from Engineer of Record or Qualified Person	Assess and revise required internal and external communication for work and construction activities carried out near the site TSFs.
	2016-05		Sediment pond designer to sign off on reconstruction of NTP dam north abutment.
			Engineer of Record to review final NTP dam re-construction summary.
	2016-06	No closure plan for NTP	Develop closure plan for NTP based on results of feasibility investigation into NTP decommissioning.
	2016-7a,b		Fill voids in downstream slope created by burrows.
		Additional animal burrows in toe of south abutment	Perform regular inspection for new burrows. Burrows to be marked or identified by GPS during inspection to improve tracking of ongoing efforts.
	2016-11	Discrepancy within the documentation regarding the minimum elevation of the NTP crest. The lower elevation value has been reported here (i.e., more conservative)	The NTP dam, particularly the dam crest, be surveyed to confirm the minimum dam elevation.
	2013-16	No passive emergency system against overtopping; emergency system requires active response	Assess the best combination of active and passive emergency systems during various stages of the pond life cycle. If the assessment determines that passive systems are warranted, then develop a construction schedule for the selected system(s).
	2015-11	Over-steepened relative to design and susceptible to erosion from wave action	Regrade upstream slope to design (1.75H:1V) and place riprap on regraded slope.
			Perform risk-informed assessment to determine appropriate flood protection requirements for downstream toe of dam along Fording River and timeline to implement
STP	2015-12a,b,c	Riprap erosion protection along downstream toe north of STP Stn. 0+680, no riprap south of STP Stn. 0+680; risk-informed protection requirements not yet defined	Implement required protection measures for the operational phase according to the as-defined schedule.
		informed protection requirements not yet defined	Execute flood risk mitigation plan until flood protection requirements defined by the risk-informed assessment are in place.
	2016-08	Ditch adjacent road on West Dam bench filled in by haul truck traffic	Reinstate ditch to functional condition.
	2016-12	Hydroseeding in the repaired sections of the STP downstream slope did not root. No records available	Hydroseeding should be incorporated into the tailing management system and records hydroseeding, including the success rate, should be kept.

	Updated Status as of March 2018
	Incomplete – see Table E-2 for updated recommended action and deadline
	Incomplete – see Table E-2 for updated recommended action and deadline
nt.	Incomplete – see Table E-2 for updated recommended action and deadline
;-	Incomplete— see Table E-2 for updated recommended action and deadline
/	Ongoing
	Complete – Golder provided recommendations for pipe abandonment in November 2017, pipes are currently capped on upstream side and are inspected regularly
	Incomplete – see Table E-2 for updated recommended action and deadline
	In Progress – to be included in OMS manual
	Complete (AMEC 2017)
	Complete – Liverpool outlet channel and fish barrier should now be considered outside of NTP facility and part of Liverpool Sediment Pond System.
	In Progress – prefeasibility design completed
	Complete
S	Ongoing, include in OMS manual update
	Complete
5	Incomplete – see Table E-2 for updated recommended action and deadline
	Complete
nt.	In Progress – completed drilling program for widening of Fording River channel
	Incomplete – see Table E-2 for updated recommended action and deadline
	Ongoing
	Complete
s of	Incomplete – recommendation moved to OMS manual updates (2016-03)





Facility	ID Number	Deficiency or Non-conformance	Recommended Action
	2016-13	The GoldSIM water balance model is not accurately accounting for the change in available volume in the STP due to dredging (increase in available volume) and tailings depositions (decrease in available volume)	The change in available volume be included in future water balances to improve the STF water balance.
	2015-03	Roles of Geotechnical and Hydraulics Engineers of Record undocumented	Golder, FRO, and KWL to document the roles of the Engineer of Record for the geotechnical and hydraulics related works in the OMS manual.
			Complete updated seismic stability assessment and liquefaction based on revised desig criteria.
	2016-01a,b	Seismic design criteria for stability out of date due to dam reclassification from High to Very High	Update QPOs based on revised stability assessment.
	2016-02	IDF and freeboard out of date due to dam reclassification from High to Very High	Update the IDF and freeboard assessment for the NTP and STP.
NTP/STP	2016-03	OMS manual requires updating	 Update OMS manual as follows: Update all references to consequence classification of structures—change from High to Very High. Include design criteria. Review the manual using the updated HSRC and Guidance Document (MEM 2017 2016). QPOs to be included for surveillance. The dredging section needs to be updated to identify that dredging is currently operating to the Turnbull Tailings Storage Facility. Include safe work plans. Include incident reporting procedures. Complete minor updates identified in the 2015 DSI report (Golder 2016b).
	2016-04	EPP & ERP require updating	Reference to the QPOs needs to be included for actions required based on instrumentation warnings and alarms.
	2016-09	No QPOs set for inclinometers	QPOs and frequency of readings should be set for the inclinometers.
	2016-10	Warning level QPO for piezometers exceeded. Based on review of data, this is not a failure concern	Update warning level QPOs for piezometers based on review of data all available data (2014 to present).

IDF = inflow design flood; FRO = Fording River Operations; KWL = Kerr Wood Leidal Associates Ltd.; NTP = North Tailings Pond; STP = South Tailings Pond; HSRC = Health, Safety and Reclamation Code; DSI = dam safety inspection; TSF = tailings storage facility; OMS = operation, maintenance and surveillance; EPP = Emergency Preparedness Plan; ERP = Emergency Response Plan; QPO = quantitative performance objectives.

	Updated Status as of March 2018
STP	Complete
	In Progress – to be included in OMS manual
esign	In Progress – draft assessment complete, seismic stability meets or exceeds new design criteria
	In Progress – pending completion by Golder, updated GPS & prism QPOs in this DSI report; updated piezometer QPOs pending completion by Golder
	In Progress – draft assessment complete.
017,	In Progress – under review by FRO, QPO update pending completion by Golder
	In Progress – pending completion by FRO
	In Progress – draft complete.
ta	In Progress – pending completion by Golder, recommendation incorporated in 2017-06





2017 Dam Safety Inspection Findings and Recommended Actions

Table E-2 summarizes the 2017 findings and recommended actions for the NTP and STP, along with incomplete and in progress items from previous DSIs. Previous recommendations have been reviewed and updated according to the information included in the 2017 DSI.

Table E-2: 2017 Dam Safety	Inspection Recommende	d Actions for the North a	nd South Tailings Pond Facilities
Table E-2. 2017 Dalli Salely		a Actions for the North a	nu south rainings ronu racinties

Structure	ID Number	Deficiency or Non-conformance	Applicable Guideline or OMS Reference	Recommended Action	Priority Level	Recommended Timing for the Action
	2015-05a,b	No passive emergency system against		Assess the need for spillway after finalizing the closure plan NTP.	4	Q3 2018
		overtopping; emergency system requires active response	n/a	If required, determine a construction schedule.	4	Q4 2018
				Perform risk-informed assessment to determine appropriate flood protection requirements for downstream toe of dam along the Fording River and the timeline to implement.	2	Q3 2018
	2015-06a,b,c	Risk-informed criteria for flood erosion protection along toe of dams not defined	CDA 2013 §6.2	Implement required protection measures for the operational phase according to the as- defined schedule.	2	2019
			Execute the flood risk mitigation plan until the flood protection requirements defined by the risk-informed assessment are in place. 1 Ongoing	Ongoing		
NTP	2015-07a,b,c	7a,b,c Buried pipes passing through crest locations	n/a	Inspect steel pipes as part of regular dam inspections until NTP closure plans are finalized. Include inspections in OMS manual update.	3	Q2 2018 (tentatively complete since December 2017, pending final OMS revision)
				Execute abandonment plan for PVC pipes.	3	Q4 2018
	2016-05a	North abutment excavated without input or approvals from Engineer of Record or Qualified Person	n/a	Assess and revise required internal and external communication for work and construction activities carried out near the site TSFs	1	Q2 2018 (tentatively complete since March 2018, pending final OMS revision)
	2016-06	No closure plan for NTP	n/a	Develop closure plan for NTP based on results of feasibility investigation into NTP decommissioning.	4	Q4 2018
	2013-16	No passive emergency system against overtopping; emergency system requires active response	n/a	Assess the best combination of active and passive emergency systems during various stages of the pond life cycle. If the assessment determines that passive systems are warranted, then develop a construction schedule for the selected system(s).	4	Q4 2018
	2015-12a,b,c	015-12a,b,c Riprap erosion protection along downstream toe north of STP Stn. 0+680, no riprap south of STP Stn. 0+680; risk-informed protection requirements not yet defined	HSRC §10.1.8	Perform risk-informed assessment to determine appropriate flood protection requirements for downstream toe of dam along Fording River and timeline to implement.	2	Q3 2018
				Implement required protection measures for the operational phase according to the as- defined schedule.	2	2019 or as determined by schedule
				Execute flood risk mitigation plan until flood protection requirements defined by the risk- informed assessment are in place.	1	Ongoing
STP	2017-01	North abutment construction deficiencies	HSRC §10.5.1(3)	Address construction deficiency, finish dam construction.	2	2019
	2017-02	Tailings that were excavated along upstream slope to place the riprap zone impede water flow towards main pond	n/a	The tailings should be regraded with an excavator so that water will preferentially flow into the pond.	4	Q3 2018
	2017-03	Inspection frequency inadequate for active, Very High consequence facility	HSRC §10.1.12	Increase geotechnical inspections to weekly from April to October and twice per month from November to March for STP	3	Q2 2018 (in progress, pending final OMS revision)
	2017-04	Planned dredging of Tailings to Turnbull TSF is behind schedule and the result is a very high level of tailings in STP which is causing operational issues (e.g., disposition line	n/a	Dredging to Turnbull TSF should be started as soon as possible with an increased annual dredging target.	2	Q2 2018





Structure	ID Number	Deficiency or Non-conformance	Applicable Guideline or OMS Reference	Recommended Action	Priority Level	Recommended Timing for the Action
		backing up and reclaimed process water with too much sediment)				
	2015-03	Roles of Geotechnical and Hydraulics Engineers of Record undocumented	HSRC §10.1.5	Golder, FRO, and KWL to document the roles of the Engineer of Record for the geotechnical and hydraulics related works in the OMS manual.	4	Q2 2018
	2016-01	Seismic design criteria for stability out of date due to dam reclassification from High to Very High	HSRC §10.1.4 & 8	Complete updated seismic stability assessment and liquefaction based on revised design criteria. Check effects of upward gradient noted in STP piezometers.	2	Q2 2018 (draft in review)
	2016-02	IDF and freeboard out of date due to dam reclassification from High to Very High	HSRC §10.1.4 & 8	Update the IDF and freeboard assessment for the NTP and STP.	2	Q2 2018 (draft in review)
NTP/STP	2016-03	OMS manual requires updating	HSRC §10.5.2(4)	 Update OMS manual as follows: Update all references to consequence classification of structures—change from High to Very High. Include design criteria. Review the manual using the updated HSRC and Guidance Document (MEM 2017, 2016). Review the manual using most recent MAC guidelines QPOs to be included for surveillance. The dredging section needs to be updated to identify that dredging is currently operating to the Turnbull Tailings Storage Facility. Include safe work plans. Include animal burrow inspection and procedures. Include animal burrow inspections Include NTP pipe inspections Include hydroseeding records Include Liverpool and NTP boundaries 	4	Q2 2018 (draft in progress)
	2016-04	EPP & ERP require updating	HSRC §10.4.2(1)	Reference to the QPOs needs to be included for actions required based on instrumentation warnings and alarms.	4	Q3 2018
	2016-09	No QPOs set for inclinometers	HSRC §10.1.13	QPOs and frequency of readings should be set for the inclinometers.	3	Q2 2018 (draft in this document)
	2017-05	Potential overtopping hazard due to tailings liquefaction and redistribution during seismic event needs to be assessed	n/a	Complete liquefaction and overtopping assessment for tailings within facility.	2	Q4 2018
	2017-06 (supersedes 2016-01b and 2016-10)	Trigger-action-response plans (TARPs) and related QPOs not strongly tied to risk assessment results	HSRC §10.5.2	TARPs with related monitoring plans and QPOs should be reviewed with consideration of the results from the 2017 TSF risk assessment	3	Q3 2018

STP = South Tailings Pond; NTP = North Tailings Pond; FRO = Fording River Operations; KWL = Kerr Wood Leidal Associates Ltd.; OMS = operation, maintenance and surveillance; CDA = Canadian Dam Association; HSRC = Health, Safety and Reclamation Code; QPO = quantitative performance objectives; EPP = Emergency Preparedness Plan; ERP = Emergency Response Plan; Stn. = Station; n/a = not applicable; DSI = dam safety inspection; TSF = tailings storage facility.





Priority Level	Description
1	A high probability or actual dam safety issue considered immediately dangerous to life, health or the environment, or a significant risk of regulatory enforcement.
2	If not corrected could likely result in dam safety issues leading to injury, environmental impact or significant regulatory enforcement; or, a repetitive deficiency that demonstrates
3	Single occurrences of deficiencies or non-conformances that alone would not be expected to result in dam safety issues.
4	Best Management Practice – Further improvements are necessary to meet industry best practices or reduce potential risks.

Source: HSRC Guidance Document, Section 4.2 (MEM 2016).

tes a systematic breakdown of procedures.





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APPENDIX I

2017 Riprap Inspection by Kerr Wood Leidal Associates Ltd.





1.0 INTRODUCTION

1.1 Purpose, Scope of Work, Methodology

The report presents the annual dam safety inspection (DSI) for the North Tailings Pond (NTP) and South Tailings Pond (STP) at the Teck Coal Limited (Teck), Fording River Operations (FRO) site, located near Elkford, BC. The reporting period for the data review is from September 2016 to September 2017, unless otherwise noted.

This report was prepared by Golder Associates Ltd. (Golder) at the request of Teck, in accordance with the Teck Guidelines for Tailings and Water Retaining Structures (Teck 2014).

The report is based on a site visit carried out by Golder from 3 to 5 October 2017, discussions with FRO staff, and review of data provided by FRO. The report consists of the following:

- a summary of the site conditions and background information for the facilities
- a summary of the construction, operating, and/or maintenance activities for the 2016/2017 period
- review of dam consequence classification and required operational documents
- site photographs and records of dam inspection
- review of climate data
- review of water balance
- review of dredging data
- review of assessment of dam safety relative to potential failure modes
- findings and recommended actions

Photographs of NTP and STP from the site inspection are presented in Appendix A, and a summary of the observations is included in the inspection reports in Appendix B and C.

FRO switched coordinate systems on 25 October 2016 from FRO Mine Grid to Universal Transverse Mercator (UTM) with elevations referenced to the Elk Valley Elevation Datum. All coordinates presented in this report are in UTM with elevations referenced to the Elk Valley Elevation Datum, unless otherwise noted.

The previous annual DSI for this facility was carried out in September 2016, and is reported in the 2016 DSI report (Golder 2017b).

This report is to be read in conjunction with the Study Limitations provided at the end of the report.





1.2 Regulatory Requirements

1.2.1 BC *Mines Act* and Health, Safety and Reclamation Code

The DSI report was prepared in accordance with Part 10 of the Health, Safety and Reclamation Code (HSRC) for Mines in British Columbia (MEM 2017), which sets out the frequency for inspection of dams and tailings storage facilities. It is understood that this report will be submitted by FRO to the Chief Inspector of Mines.

The guidelines for annual safety inspection reports provided in the HSRC Guidance Document (Section 4.2, MEM 2016) were followed where applicable during the preparation of this report.

1.2.2 Permits and Licences

Specific sections and amendments to the permits concerning NTP and STP are as follows:

- Permit C-3 Amendment to permit approving work system South Tailings Pond tailings dredging project.
 Issued by Ministry of Energy, Mines and Petroleum Resources. 27 April 1995.
- Permit C-3 Amendment to permit approving work system and reclamation program Raising the South Tails
 Pond Dyke. Issued by Ministry of Energy, Mines and Petroleum Resources. 30 June 2008.
- Permit C-3 Amendment to permit approving work system and reclamation program Turnbull South Pit Tailings Storage Facility. Issued by Ministry of Energy and Mines. 14 November 2013.
- Permit C-3 Amendment to permit approving work system and reclamation program Turnbull South Pit Tailings Storage Facility East Pipeline Route. Issued by Ministry of Energy and Mines. 6 May 2015.
- Permit C-3 Amendment to permit approving work system and reclamation program Fording River Swift Mine Plan and Reclamation Program. Issued by Ministry of Energy and Mines. 15 December 2015.
- Permit 424 Amendment to permit to discharge effluent. Issued by Ministry of Environment. 6 December 2016.



2.0 BACKGROUND

2.1 Site History

The FRO site is an open pit coal mine located near Elkford, BC. This DSI report is for two of the site's tailings pond facilities: the NTP and the STP. The NTP has been essentially filled to its design capacity and is currently inactive. Tailings discharge from the wash plant is currently directed to the STP.

The NTP is located on the west side of the Fording River across from the wash plant. The STP facility is located south of the wash plant, on the east side of the Fording River. Figure 1 shows a location and plan view of the NTP and STP facilities.

2.2 System Description

At the NTP, the earth fill dams provide storage for settled tailings and contain a small pond which receives runoff from the local tailings surface area and a small surrounding catchment area. This facility is not in active use.

At the STP, the earth fill dams provide the following:

- impoundment of the tailings slurry
- storage of settled tailings
- temporary storage of runoff, excess slurry water, and water from pit dewatering or sediment ponds (when viable based on freeboard)
- reservoir of water for the wash plant

2.2.1 Tailings Description

The raw coal delivered to the breaker at FRO contains high-ash material in the form of carbonaceous mineral rock. To meet product specifications, this high-ash rock is separated from the raw coal at the wash plant. The high-ash waste consists of a coarse fraction and a fine fraction. The coarse fraction, referred to as coarse rejects (CR), consists of sand and gravel-sized fragments of washed, crushed rock ranging in size from approximately 1 to 100 mm. The fine fraction of the waste, comprising rock fragments smaller than approximately 1.0 mm, includes "coarse-fine" rejects (0.75 to 1.0 mm) and the flotation tailings (less than 0.75 mm). Since 2005, the coarse-fine rejects and the majority of the flotation tailings have been separated at the wash plant. The coarse-fine rejects mixed with the CR to produce combined coarse and fine rejects (CCFR) which are hauled by trucks to a designated CCFR spoil. The flotation tailings from the wash plant are transferred in slurry and are hydraulically deposited from an end of pipe discharge into the south tailings pond.

2.2.2 Tailings Impoundments

In the past, tailings have been discharged to the two ponds alternately. The tailings stream has never been discharged to the two ponds concurrently. The NTP is essentially full, and tailings have not been discharged to the NTP since 2006.



Tailings are periodically dredged from the STP to restore available tailings storage capacity. Previously, the dredged tailings were pumped to either the NTP, 2 Pit, or 3 Pit South (Golder 2016c). Dredging discharge into the NTP and 2 Pit was discontinued due to capacity constraints in 2008. No dredged tailings have been sent to 3 Pit South since 6 October 2015. Starting in 2016, tailings have been dredged seasonally from the STP to the Turnbull Tailings Storage Facility (TSF) and dredging to the Turnbull TSF is planned to continue annually.

2.3 Overview of Design, Construction, and Previous Operation

A summary of the NTP and STP design, dam construction, and operations is presented in the following subsections. Additional details of construction history are presented in the operation, maintenance, and surveillance (OMS) manual (FRO 2015b).

2.3.1 North Tailings Pond

A plan view of the NTP facility is shown in Figure 2 and 3. The NTP was developed on a segment of the Fording River flood plain and has a surface area of approximately 32 ha. The Fording River was diverted into a new constructed channel (McElhanney 1969) to allow for construction of the NTP against the west side of the Fording River flood plain (Golder Brawner 1969). Along the eastern and southeastern sides of the pond, confinement for water and the stored tailings is provided by a zoned earth fill dam that has a maximum height of approximately

24 m. A confining dam is not required along the west side of the pond because the natural topography to the west of the NTP is higher than the pond level.

The dam extending along the eastern side of the NTP consists of a zoned earth fill dam. Figure 4 presents a typical section through this dam. The crest of the dam was raised in stages, as the tailings storage requirements increased progressively during the early years of operation at FRO. Stage 1 of the dam was constructed entirely of compacted glacial till soil, complete with a compacted glacial till cut-off that extends through the Fording River flood plain gravels and is joined to in situ glacial till soils that underlie the flood plain gravels.

During subsequent stages of construction, the glacial till dam was extended upward in the form of an upstream inclined zone. Structural support for this inclined till zone is provided by CR. As shown in Figure 4, the in situ fluvial sands and gravels of the Fording River flood plain extend beneath the cross-section of the dam. These fluvial sediments have a high hydraulic conductivity and are expected to serve as an underdrain that promotes downward seepage from the dam.

The original design for the NTP was completed by Golder (Golder Brawner 1969, 1970). Construction of the NTP was initiated in 1971 (Golder Brawner 1971), and the facility was put into service in March 1972. The NTP was raised four times between 1973 and 1979 (Golder Brawner 1973, 1974a,b, 1975a,b; Golder 1979) when the NTP reached its current elevation and full tailings capacity. Between 1979 and 1991, the NTP was inactive and the facility was dewatered and excavated via scrapers to recover additional tailings storage capacity (FCL 1981). The NTP was put back into active use and refilled with tailings between 1993 and 1997, after which the facility was again inactive. From 2001 to 2002, the NTP was dredged and the tailings were sent to 2 Pit and 3 Pit. Dredged tailings from the STP were used to fill the excavated areas of the NTP seasonally between 2004 and 2006. No tailings have been sent to the NTP since 2006.





The current ultimate crest of the dam at the NTP is elev. 1,653.09 m (reported as elev. 1,653.54 m FRO Mine Grid FRO 2015b). The minimum elevation of the NTP is at elev. 1,652.6 m (2017 survey completed by FRO).

Following the flood of June 1995, riprap was placed along the outside toe of the dam, as well as along the opposite (left) side of the river channel. The condition of the riprap placed in 1995 had degraded by the time of the 2006 dam safety review (DSR), and review of the riprap sizing and placement was recommended by Golder. Assessment of the riprap was performed by Kerr Wood Leidal Associates Ltd. (KWL 2007, 2009, 2014b).

Between 19 and 20 June 2013, a significant 48-hour rainfall event occurred which resulted in flooding of the Fording River. High flows along the toes of the NTP triggered major erosion of the CR shell. Golder was retained by FRO to provide geotechnical input for flood repairs of the NTP. KWL was retained to provide recommendations for sizing and placement of the riprap (KWL 2014a,b). The dam shell was rebuilt using CCFR. A total CCFR fill of approximately 22,350 m³ was placed and compacted between 3 July and 8 August 2014 (Golder 2014b).

In 2016, FRO constructed a sediment pond north of the NTP (Liverpool Sediment Ponds), and the outlet channel from this pond is routed through the north end of the NTP tailings deposit and includes a fish barrier weir constructed through the north abutment of NTP dam (AMEC-FW 2017). The sediment pond system has a future option for a polishing pond which would be located on the northern area of the NTP tailings deposit.

Riprap upgrade works began under the direction of KWL in 2016. The 2016 work included placing approximately 2.5 m of riprap along the existing NTP riprap alignment for scour protection and to accommodate the revised Q200 design flow. Construction completed during 2017 included the excavation and placement of approximately 150 m of riprap at the upstream end of the NTP, and the placement of approximately 745 m of riprap over the existing bank protection. Approximately 40 m of buried riprap and 95 m of riprap topping were to be completed in 2017 (KWL 2017c). KWL provided oversight to the gradation and quality of the riprap, which was sourced on site. Two deficiencies following the 2016 works at the NTP dam were noted in KWL (2017a). The deficiencies were addressed in 2017 as described in Section 3.1.4 (KWL 2017c).

The future use of the NTP is under review.

2.3.2 South Tailings Pond

A plan view of the STP facility is shown in Figure 5 and 6. The STP occupies a total area of approximately 67 ha and is located to the south of the wash plant, on the east side of a realigned reach of the Fording River. The STP was developed on the flood plain of the Fording River. The Fording River was diverted to a new alignment outside the footprint of the STP by excavating a new channel through a topographic bench on the west side of the Fording River flood plain. This topographic bench consists of native glacial till soils overlying Fernie Shale. Confinement at the STP is provided by the Main Dam, which extends across the full width of the Fording River flood plain, and by the West Dam, which extends parallel to the east side of the Fording River Diversion Channel. The West Dam is primarily founded on the glacial till bench.

Initial construction of the STP dams was performed between 1977 and 1979. From 1983 to 2013, the STP dams have been raised in six stages:

1) 1983 to 1984 (FCL 1984)





- 2) 1985 to 1990 (FCL 1988, 1989, 1990)
- 3) 1993
- 4) 2008 (Golder 2009)
- 5) 2010 (FRO 2010)
- 6) 2012 to 2013 (Golder 2013, 2014d)

The design crest elevation of 1,637.8 m was specified in the original design report (reported as elev. 1,638.3 m FRO Mine Grid in Golder 1976), and this elevation was reached with construction carried out in 2013. Designs of the north and south abutment sections of the dam are presented in the design update report and design drawings (Golder 2011, 2012c), and the construction summary of the STP raise is reported in the construction record report (Golder 2014c).

The current minimum crest of the dam at the STP has remained at elev. 1,637.8 m (reported as elev. 1,638.3 m FRO Mine Grid in FRO 2015b). The 2017 LiDAR survey provided by FRO confirms the dam crest is on average between elev. 1,638 and 1,639 m.

The dam construction prior to 2008 was wider than design, which created a bench along the length of the facility when the 2008 and later lifts were constructed, as shown in the sections in Figures 7 and 8.

The June 2013 flooding of the Fording River caused high flows along the toe of the STP dam, which eroded the foundation soils and a minor portion of the CR shell. Repairs to the STP toe area were completed in 2013, after the NTP flood repairs as the damage to the STP was considered to be less critical.

Riprap upgrades were completed for the STP in 2016. KWL oversaw the placement of approximately 2.5 m of riprap by FRO and FRO contractors along the existing STP riprap alignment for scour protection and to accommodate the revised Q200 design flow (KWL 2017c). Golder provided on site services to oversee resloping of the till bench, cutting into existing bedrock for key-in of the riprap material, and monitoring seepage conditions and signs of instability (Golder 2017a).

Outstanding recommendations from the reconstruction (Golder 2014c) are as follows:

- Repaired sections of the downstream slope and toe were recommended to be hydroseeded to minimize surface erosion. The seed did not root in the area. Hydroseeding should be incorporated into the tailing management system and records of hydroseeding, including the success rate, should be kept.
- River flood protection (updated from resloping and riprap placement) south of STP Stn. 0+680 (updated from 0+600) should be completed to improve long-term stability of the STP structure.





2.3.2.1 Main Dam

The STP Main Dam, which extends across the Fording River flood plain, has a maximum height of approximately 35 m. Figure 7 presents a typical section of the STP Main Dam, which consists of an upstream low permeability starter dam of compacted glacial till soil that has been raised with an inclined upstream low permeability zone of compacted glacial till soil, supported by a zone of CR or CCFR. The CR or CCFR zone that forms the downstream shell of the Main Dam provides the structural strength of the dam.

As indicated in Figure 7, discontinuous flood plain sands and gravels extend beneath the whole downstream shell of the Main Dam. These flood plain sediments are very pervious and serve as an underdrain for the dam.

2.3.2.2 West Dam

The STP West Dam is founded on the till bench that borders the western edge of the Fording River flood plain. A typical section through the West Dam is presented in Figure 8. The dam consists of an upstream low permeability zone of compacted glacial till soil supported by a zone of CR or CCFR.

2.3.2.3 Railway Embankment

A segment of the railway embankment south of the loading loop traverses an area that impounds tailings in the STP facility. A stability assessment of the embankment was previously carried out by Golder in 1984 (Golder 1984) and updated in 2010 (Golder 2010); the assessment recommended a buttress west of the embankment to maintain stability of the railway embankment with respect to the increase in the pond elevation. Golder also recommended that FRO backfill the area east of the railway embankment to provide a buttress for the railway and improve stability (Golder 2010).

In 2012, the rejects buttress fill west of the railway embankment was raised to maintain the stability of the embankment. A till cut-off was constructed through a section, close to the south abutment, of the rejects buttress (Golder 2012c). In 2014, the rejects buttress fill was further raised by FRO personnel to maintain a trafficable surface.

The Golder (2010) recommendations included grouting the existing culverts that conveyed surface runoff through the railway embankment and installing new culverts at a higher elevation. The corrugated steel culverts passing through the railway embankment were filled with concrete during 2009 and 2010 to prevent the flow of tailings from the STP to the east as the tailings level rose above the elevation of the existing culverts. The unused culverts were properly closed and abandoned, and the area of the railway embankment was backfilled and graded. Surface runoff from the area upslope of the railway embankment, including Blackmore Creek, is now diverted around the backfilled area into STP through twin culverts installed in 2010.

2.3.2.4 Waste Water Cells

Waste water cells were built on the north end of the STP (Figure 6) to store waste water from the truck shop sumps for a period of time that would allow hydrocarbons to be floated and sludge to settle prior to removal from site. These cells are designed to decant to the STP, with minimal surface water and hydrocarbons reaching the STP. Samples are taken monthly to ensure the permitted limit of 15 mg/L of hydrocarbons is not exceeded.





2.3.3 Water Management of North Tailings Pond and South Tailing Pond

Water levels in the NTP and the STP are managed to maintain the required minimum freeboard at each facility.

Floating reclaim pumps are used to recirculate water from the STP to the wash plant. Water demand at the wash plant is greater than the volume of water that is available from recirculation of slurry transport water alone, creating a water deficit in the system which is accommodated through introduction of makeup water to the STP from various locations on site. There are no permanent working pumps at the NTP.

In the event of high pond levels in either the NTP or the STP, the OMS manual indicates the pond levels will be controlled by shutting off all input sources and pumping the excess water to other locations on site (FRO 2015b). A passive system of controlling pond levels (such as a spillway) is not currently available in either the NTP or the STP.

2.3.4 Design Parameters for the North Tailings Pond and South Tailings Pond

The following design parameters apply to the NTP and STP. Typical sections of the dams are shown in Figure 4 for the NTP and in Figures 7 and 8 for the STP.

2.3.4.1 Foundation Materials

The retention dams at the NTP and the STP are founded on Fording River flood plain sands and gravels, dense glacial till soils, or shale bedrock.

A subsurface investigation was completed by FRO to compile in situ density data and subsurface stratigraphy under the NTP and STP dams (FRO 2016).

2.3.4.2 Embankment Fill Materials

The following materials were used in the construction of the dams: till fill and CR and CCFR.

2.3.4.2.1 Till Fill

A wedge of compacted glacial till fill forms the upstream face of the retaining dams. The till fill serves as a low permeability zone to minimize seepage through the dam rather than structural support. The till fill is sourced locally on site.

2.3.4.2.2 Coarse Rejects and Combined Coarse and Fine Rejects

At both the NTP and the STP, the bulk of the fill that provides support for the low permeability zone of the dams consists of CR. The CR is a waste product generated at the wash plant and consists of sand and gravel-sized, well-graded, washed crushed rock.





For the 2010 and 2012 raises of the STP dams, CCFR was used in place of the CR following modifications to the wash plant waste streams. The CCFR is formed by combining the CR with finer material previously sent to the tailings ponds as tailings. The CCFR contains approximately 2% to 10% material finer than 0.075 mm. The engineering properties of the CCFR are similar to those of the CR presented in Table 1.

Property	Value
Average in situ density	1.75 t/m ³
Friction angle	38 to 40.5 degrees
Compressibility	Low

2.3.4.3 Seismicity

The site is located in an area of relatively low seismicity for British Columbia. Golder developed a site-specific seismic hazard model for the FRO site based on historical seismicity and a review of geological and paleoseismological features (Golder 2016a). Golder's model includes four area sources from the 5th Generation Seismic Hazard Model and nine faults and fault segments mapped in northwest Montana. The 5th Generation Seismic Hazard Model was developed by Natural Resources Canada for use in the 2015 National Building Code of Canada.

Probabilistic analysis results from site-specific hazard model are listed in Table 2. All site-specific peak ground acceleration values were evaluated for a soil Site Class C as described in the 2010 National Building Code of Canada (NRCC 2010) as this represents Golder's understanding of the general foundation conditions at the dam locations.

Exceedance Probability	Return Period (years)	Peak Ground Acceleration (g)
40% in 50 years	100	0.020
10% in 50 years	475	0.063
5% in 50 years	1,000	0.097
2% in 50 years	2,475	0.158
1% in 50 years	5,000	0.222
1/2% in 50 years	10,000	0.300

Table 2: Fording River Operations Site Seismic Hazard Values

Notes: For firm ground site class "C," very dense soil and soft rock foundation, as defined by 2010 National Building Code of Canada (NRCC 2010).

Return periods are not exact representations of annual exceedance probabilities; rounding per CDA (2013, 2014) is shown. FRO site coordinates: 50.202°N, -114.876°W.

The HSRC Guidance Document, Section 3.3.1 (MEM 2016) recommends a return period of ½ between the 2,475-year and 10,000-year seismic event or the maximum credible earthquake for Very High consequence structures.





2.4 Dam Consequence Classification

Guidelines for the classification of dams are presented in the HSRC Guidance Document, Section 3.4 (MEM 2016), which references the Canadian Dam Association (CDA) *Dam Safety Guidelines* (CDA 2013). Table 3 presents the dam classification criteria. Consequence categories are based on the incremental losses that a failure of the dam may inflict on downstream or upstream areas, or at the dam location itself. Incremental losses are those over and above losses that might have occurred in the same natural event or condition had the dam not failed. The consequences of a dam failure are ranked as Low, Significant, High, Very High, or Extreme for each of four loss categories. The classification assigned to a dam is the highest rank determined among the four loss categories.

Incremental Losses Dam Class Population at Risk **Environmental and** Infrastructure and Loss of Life **Cultural Values Economics** Low economic losses; area Minimal short term loss. Low None 0 contains limited No long term loss. infrastructure or service. Temporary only The appropriate level No significant loss or (e.g., seasonal of safety required deterioration of fish or Losses to recreational cottage use, depends on the wildlife habitat. or facilities, seasonal passing through on number of people, the Significant Loss of marginal habitat workplaces, and transportation exposure time, the onlv. infrequently used routes, participating nature of their transportation routes. Restoration or compensation in recreation activities, and other in kind highly possible. activities) considerations Permanent-Significant loss or ordinarily located in High economic losses deterioration of important the dam-breach affecting infrastructure, fish or wildlife habitat. High 10 or fewer public transport, and inundation zone Restoration or compensation commercial facilities. (e.g., as permanent in kind highly possible. residents) Permanent-Significant loss or Verv high economic losses deterioration of critical fish or ordinarily located in affecting important wildlife habitat. the dam-breach infrastructure or services Very High 100 or fewer (e.g., highway, industrial inundation zone Restoration or compensation facility, storage facilities for (e.g., as permanent in kind possible but dangerous substances). residents) impractical. Extreme losses affecting Permanent critical infrastructure or Major loss of critical fish or ordinarily located in services wildlife habitat. the dam-breach Extreme More than 100 (e.g., hospital, major inundation zone Restoration or compensation industrial complex, major (e.g., as permanent in kind impossible. storage facilities for residents)

Table 3: Dam Classification

Source: HSRC Guidance Document (MEM 2016), Table 3-3 based on CDA (2013), Table 2-1.



dangerous substances).



2.4.1 Facility Consequence Classification

An inundation study considering both flood-induced (overtopping) and sunny day (piping) failure modes for the NTP and STP dams was performed to understand the potential incremental impacts on downstream receptors (Golder 2014e). The flood-induced (overtopping) inundation assumed a 1-in-2-year flood event (bankfull conditions) in the Fording River (Golder 2014e). A single classification for the dam system is based on the failure scenario that would result in worse consequences: either sunny-day failure or flood-induced failure (CDA 2013).

The rationale applied for assigning the consequence level for each attribute for the NTP and STP facilities is as follows:

- Population at risk (High)—Permanent, as identified by Golder (2014e), some 18 permanent residences are located on the floodplains downstream of the dams within the flood inundation extents.
- Loss of life (Significant to High)—Since people may be present in the inundation zone, it is foreseeable that there is a possibility for loss of life (for STP and NTP permanent downstream residences and for Maxam Yard [site explosive storage facility including Maxam personnel offices]). Quantification of loss of life has been conservatively inferred from population at risk (Golder 2014e; KCB 2014).
- Environmental and cultural (High to Very High)—Presence of critical habitat for Westslope Cutthroat Trout, a species of Special Concern. Restoration is considered to be possible but difficult. The classification is Very High for fair weather failure scenario and High for flood-induced failure scenarios (Teck 2016).
- Infrastructure and economics (High)—Economic losses are anticipated to be high in the event of a failure (Golder 2014e).

Table 4 presents a summary of the current dam consequence classifications for the FRO facilities.

		Population at Risk	Consequences of Failure			
FRO Facility	Dam Class		Loss of Life	Environment and Cultural Values	Infrastructure and Economics	
NTP	Very High	High	Significant to High	High to Very High	High	
STP	Very High	High	Significant to High	High to Very High	High	
NTP and STP river flood-induced components	High	High	Low to Significant	High	Significant	

Table 4: Dam Consequence Classification Results

Note: River flood induced component classification based on dam inundation concurrent with major flood event. Lower design criteria related to "High" classification is for the riprap components of the NTP and STP only and does not change the overall classification of the facility. Refer to Section 2.5.4, CDA 2013.

FRO = Fording River Operations; NTP = North Tailings Pond; STP = South Tailings Pond.



The NTP and STP dams are classified as Very High, while the components for a river flood-induced failure are classified as High. The NTP and STP classification is governed by the consequences of a potential fair weather failure scenario.

2.4.2 River Flood Component Consequence Classification

An incremental inundation assessment was recently completed (Golder 2017g) to assess the consequence of failure of the NTP and STP during 200-year and 500-year Fording River flood events. The assessment concluded that the consequence of a failure occurring coincident with the flood events considered was High.

A risk-informed assessment, which would be supported by a design level flood-induced dam break and inundation assessment, is recommended to determine the appropriate criteria for the flood protection requirements along the downstream toes of the NTP and STP dams.

2.5 Key Personnel

The Engineer of Record for the NTP and STP dams is John Cunning, P.Eng., an employee of Golder.

KWL has historically been responsible for hydraulics-related works and has completed the most recent Fording River hydraulics assessment, including design for erosion protection along the NTP and STP dam toes following the 2013 flood event. Jason Miller of KWL is the Designer of Record for the erosion protection works for both the NTP and STP facilities. Inspection of riprap for both the NTP and STP was completed in 2017 by KWL (KWL 2018).

The NTP and STP Qualified Person is Heather Brickner, P.Eng., a Tailings Engineer and employee of FRO.

Golder, FRO, and KWL are in the process of clarifying and documenting the roles of the Engineers of Records for the geotechnical and hydraulics-related works.

2.6 Quantitative Performance Objectives

Golder is currently reviewing all QPOs and this report includes information that may be updated or changed based on subsequent review by Teck and/or Golder. Specifically, trigger-action-response plans (TARPs), related monitoring plans, and QPOs should be reviewed with consideration of the results from the 2017 TSF risk assessment.

The current QPOs for the piezometers as they are set in the GeoExploer monitoring system are presented in Table 5. These QPOs are currently under review based on the updated stability assessment for the Very High dam class.





Dam	Monitoring Instrument	Warning Water Elevation (m)	
	TH15-05	> 1,646.546 m	
NTP	TH15-06	> 1,643.546 m	
	TH15-07	> 1,640.546 m	
	SP-3	> 1,604.046 m	
STP – Main Dam	VW-3	> 1,627.046 m	
STP – Main Dam	TH15-01 / VW-5	> 1,617.546 m	
	TH15-02 / VW-4	> 1,624.046 m	
	TH15-03 / VW-1 / VW-2	> 1,627.546 m	
STD West Dom	SP-W1	> 1,623.146 m	
STP - West Dam	SP-W3	> 1,623.046 m	
	TH15-04	> 1,603.546 m	

Table 5: GeoExplorer Piezometer Instrumentation Trigger Levels for NTP and STP

Table 6 provides updated QPOs for the prism and GPS units on the NTP and STP dams. These QPOs should be considered draft.

Dam	Monitoring Instrument	Survey Data	Warning	Alarm
NTP	Prisms	SD displacement	> 15 mm	> 20 mm
		3D displacement	> 100 mm	> 150 mm
	GPS	3D displacement	> 100 mm	> 150 mm
		3D velocity with 12 point averaging	> 50 mm/day	> 100 mm/day
STP		3D displacement	> 100 mm	> 150 mm
	GPS	3D velocity with 12 point averaging	> 50 mm/day	> 100 mm/day

Table 6: Draft Prism and GPS Instrumentation Trigger Levels for NTP and STP

SD=slope distance or two dimensional; 3D is three dimensional.

Note: Discuss with engineer of record prior to zeroing displacement data

The warning and alarm triggers shown in Table 7 are currently used by FRO for the NTP and STP pond elevations. The freeboard levels are expected to increase based on the updated consequence classification. The hydraulic assessment is under review by Teck at the time of writing this report. It is anticipated that FRO and the EoR will need to develop an action plan and schedule to address the expected changes in freeboard requirements once the revised hydraulic assessment for the Very High dam class is complete.

Table 7: Freeboard Trigger Levels for NTP and STP

Dam	Survey Data	Survey Data Warning Ala	
NTP	Pond freeboard	> 1,651.4 m	none
STP	Pond freeboard	> 1,636.8 m	> 1,636.9 m





In total, there are seven inclinometers (Table 8): four inclinometers are installed in the STP dam (TH15-01 to TH15-04) and three are installed in the NTP (TH15-05 to TH15-07).

Location	Test Hole	Approximate A-A Axis Azimuth (°)	Hole Depth (m)	Casing Stickup (m)	Start Depth (m)	Reading Intervals (m)
	TH15-01	310	41.00	0.8	40.0	1.0
STP	TH15-02	10	40.00	1.0	40.0	1.0
512	TH15-03	30	30.05	1.1	30.0	1.0
	TH15-04	15	6.00	1.0	6.0	1.0
NTP	TH15-05	235	20.90	0.9	21.0	1.0
	TH15-06	290	29.20	1.0	29.0	1.0
	TH15-07	305	40.80	0.9	41.0	1.0

Table 8: Inclinometer Summary

Summary table provided by email (Roseingrave R. 2017.)

The inclinometers were first read in December 2015, and to date, the readings have not shown any indication of significant deformations. Maximum total downstream displacements (from the initial reading) of 3 mm have been observed in TH15-02, TH15-03, and TH15-07 during the period from December 2015 to July 2017.

Based on the information available, with a monthly data gathering frequency, the following TARP for the inclinometers is recommended:

ltem	Threshold Criteria				
	Acceptable	Warning	Alarm		
Inclinometer QPO	downstream displacement < 5 mm	downstream displacement > 5 mm and < 15 mm	downstream displacement > 15 mm		
Action Required	 Record monitoring results 	 Increase frequency of monitoring of GPS and inclinometers to weekly and record results Inspection of embankment in area of concern; document inspection 	 Suspend activities in area of concern Increase frequency of monitoring of GPS and inclinometers to daily and record results Inspection TSF by EoR or designate Daily inspection of embankment in area of concern; document inspection 		
Personnel Notified	 Engineering of Record receives a copy of the monitor annually with DSI data Tailings Engineer 	 Engineer of Record Tailings Engineer 	 Ministry of Energy, Mines and Petroleum Resources Teck's Tailings Working Group Engineer of Record 		





3.0 OPERATIONS, MAINTENANCE, AND CONSTRUCTION DURING 2017

A TSF risk assessment for both facilities was undertaken in 2017, with reporting of the results currently in progress.

Changes at the NTP and STP since the September 2016 DSI are discussed in the following sections.

3.1 North Tailings Pond

3.1.1 **Operations and Capacity**

The NTP was not operational in 2017 and there was no tailings deposition.

The NTP has an available volume of about 175,000 m³ to the current freeboard elevation (1,651.4 m); about 12,000 m³ of the available volume is occupied by the existing pond. However, the required freeboard is expected to increase based on the change to the NTP consequence classification to Very High.

For planning purposes, the NTP should be considered as having no available tailings capacity.

3.1.2 Inspections

The dam is inspected monthly by FRO geotechnical personnel, and water quality testing is completed quarterly. Dam inspection forms are provided in Appendix D. Water quality testing results are provided in Appendix E.

3.1.3 Liverpool Sediment Pond System

The Liverpool outlet channel and fish barrier at the north abutment area of the NTP facility were completed in late 2016. They are not considered part of the NTP facility, but as part of the Liverpool Sediment Pond system. However the Liverpool Sediment Pond outlet channel was constructed over the NTP tailing beach at the north end of the facility and the fish barrier structure was constructed through the NTP dam's north abutment. The area should continue to be inspected during both of the monthly NTP and Liverpool Sediment Pond. A figure outlining the NTP facility boundary has been prepared and should be included in the OMS manual for each facility. Discussions between Engineers of Record for each facility and FRO should be held to review the boundary.

3.1.4 Future Use and Analyses

The future use of the NTP is under review. Golder completed a flowability assessment for the NTP in 2016 (Golder 2017f) to assess the possibility of revising the NTP from a tailings dam to a mine waste facility or "landform" per Section 10.6.12 of the HSRC (MEM 2017). A prefeasibility design was completed for the facility (Golder 2017d, e).





3.1.5 Riprap Improvements

Riprap upgrade works to accommodate the revised Q200 design flow plus 0.5 m were completed in 2017 at the NTP under the direction of KWL. In 2017, the setback riprap between Sta. 0+165 and 0+205 was constructed to complete the riprap revetment and the riprap elevation between Sta. 0+205 and 0+930 was increased to meet the 2016 design basis, however, there remains areas that are about 0.1 to 0.2 m below the design elevation (KWL 2017c). Riprap has been placed along a section approximately 1,030 m long, from the Fording River Multiplate haul road along the Fording River to approximately Sta. 1+080 (KWL 2017c).

3.2 South Tailings Pond

3.2.1 **Operations and Capacity**

The STP was active over the reporting period and received tailings and site run off.

Bathymetric surveys are completed by FRO twice a year to confirm remaining capacity in the STP. Based on the 11 September 2017 survey, there were 537,900 m³ of tailings deposited since 4 October 2016, with a projected annual deposited volume of 628,000 m³ (FRO 2017b).

The remaining storage in the STP as of September 2017 was approximately 389,000 m³, assuming the water elevation is at the minimum freeboard elevation (1,636.6 m). This volume does not allow for the typical pond volume of 500,000 m³ to support the plant operations (FRO 2017b) which has resulted in tailings solids depositing around the reclaim barge. The available storage is typically occupied by pond water as the STP has been operated near, but below, the freeboard elevation over the reporting period (Section 5.4.1.2).

Based on the change to the consequence classification, the inflow design flood (IDF) will increase which will increase the required freeboard unless the catchment area is decreased.

3.2.2 Inspections

The STP dams are inspected monthly by FRO geotechnical personnel (Appendix D), and water quality testing is completed quarterly (Appendix E).

3.2.3 Dredging

Dredging from the STP to the Turnbull TSF began in 2016. FRO has a plan to transfer up to 1 million dry metric tonnes of tailings each year by dredging between about May and October. An estimated 850,076 dry metric tonnes of tailings were dredged from STP and sent to the Turnbull TSF between 24 May and 8 October 2017. A total of just over 1 million dry metric tonnes have been transfer by dredging operations over two years of operations, which is behind the planned schedule.





3.2.4 Construction and Maintenance

There was a potential tailings water discharge event in late June 2017 when FRO Environmental personnel noticed water seeping back into the STP dam in the area of the north abutment. Water was seeping into an incomplete section of the dam embankment, presumably into granular fill around the FortisBC gas line. No seepage was observed exiting the downstream slope of the north abutment and no soft spots were found on the crest of the north abutment. A sandbag berm was constructed near the STP discharge as a temporary measure to exclude tailings water from the area, and a till berm was later constructed directly behind the sandbag berm (Golder 2017e). Golder does not characterize this event as a "spill" as no tailings water was discharged to the environment, although FRO personnel pre-emptively reported to Emergency Management BC as a precautionary measure. In response the Golder's recommendations, FRO has extended the tailings line out further into the tailings pond and filled in the low area around the previous end of the tailings pipe to reduce the seepage rate reaching the unfinished north abutment. Golder recommends FRO complete the north abutment tie-in as soon as feasible but recognizes the removal of the high pressure gas pipeline is not likely to be completed in 2018. More details are provided under separate cover (Golder 2017c).

The berms along the West Dam of the STP's lower access road were repaired in the area adjacent to the Fording River bank. The ditches along the base of the West Dam were also cleaned out and regraded. This work was completed in August 2017.

Riprap was placed along the upstream slope of STP between Sta. 0+700 and 1+700 from 20 to 29 September 2017 to address DSI recommendation 2015-11.

An emergency riprap stockpile is typically maintained at the south end of the STP with an approximate volume of 4,500 m³. The stockpile was removed to be used in a rock drain project for the Swift expansion. The riprap stockpile was replaced with better quality material (as confirmed by KWL personnel) in late 2017. The removal of the stockpile was completed in consultation with the tailings engineer and replaced within a timely manner to ensure material was in place prior to the 2018 freshet.

3.3 North and South Tailings Pond

A probable maximum flood assessment was completed for the Fording River watershed above the NTP and STP facilities by KWL (2017b). The probable maximum flood peak Fording River flow estimate is 790 m³/s for the Fording River watershed with consideration of planned future mining activities. Additional details are available in the KWL report (2017b).





4.0 REVIEW OF CLIMATE DATA, WATER BALANCE, AND DAM REGISTRY

4.1 Climatic Review

Precipitation data are available from regional stations operated by Environment Canada and local stations operated by FRO.

Regional climate data were compiled from climate monitoring stations operated by Environment Canada. Six stations have historical year-round daily data, with periods of record ranging from 11 to 40 years. Some of the data years are incomplete or missing. Fording River Cominco (Climate Station 1152899) and Sparwood (Climate Station 1157630) are the only stations that are still active and have year-round daily data. The Fording River Cominco station is the closest Environment Canada station with long-term climate data. A third active station, Sparwood CS (Climate Station 1157631), has seasonal daily data.

The Fording River Cominco station is the closest regional station to the FRO site with long-term climate data. As part of the ongoing hydrology baseline analysis conducted for the Turnbull Project, a continuous record of daily precipitation using data from 1970 to 2017 was derived for the Fording River Cominco climate station by infilling data gaps (~9% missing). Missing precipitation data were infilled with data from regional stations, after adjusting for elevation differences using regression equations, in the following order of preference (depending on the availability of data at the station): Elkford, Sparwood, Sparwood CS, and Line Creek Operations Mine Services Area. Data from the local stations at FRO were not used in the development of the regression relationship because of their short data records.

The local climate data are available from three local climate monitoring stations operated by FRO: the Brownie Spoil, waste water treatment plant, and A Spoil stations. The available records include the period of interest for the DSI evaluation (i.e., from 1 September 2016 to 31 August 2017). It is noted that the A Spoil station record has several periods with missing data likely resulting in underestimation of monthly precipitation.

The monthly precipitation data from 1 September 2016 to 31 August 2017 at the Brownie Spoil, waste water treatment plant, and A Spoil climate stations are plotted in Chart 1 along with the derived precipitation for the Fording River Cominco climate station (infilled data) for the same period. The average precipitation for the period 1970 to 2016 from the regional station is also plotted in Chart 1.



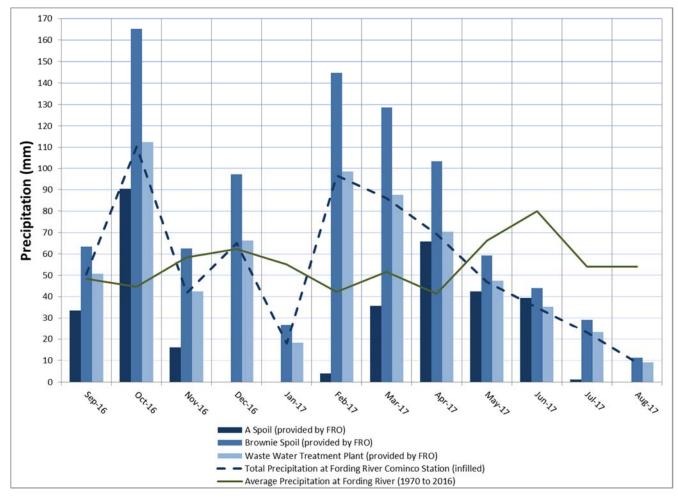


Chart 1: 1 September 2016 to 31 August 2017 Precipitation Data

The monthly precipitation values from the waste water treatment plant local station and the Fording River Cominco regional station are comparable as shown in Chart 1. This is consistent with the fact that both stations are located at similar elevations. The Brownie Spoil station is located at a higher elevation and consistently shows higher precipitation values.

Several storm events occurred in October 2016 in the Elk Valley, resulting in a high monthly precipitation for October as can be observed in Chart 1.

Winter snowfall accumulation typically runs off starting in April to May at FRO, and the higher flow events are expected to occur as a result of combined rainfall and freshet (snowmelt) events.

The total precipitation received between 1 September 2016 and 31 August 2017 is shown in Table 10 and compared with the average precipitation at Fording River for the period of 1970 to 2016.





Table 10: Precipitation from 1 September 2016 to 31 August 2017

Weather Station Location	Total Precipitation (mm)
Brownie Spoil Weather Station	936
Waste Water Treatment Plant Weather Station	662
A Spoil Weather Station	329
Fording River Cominco Station (infilled)	651
Long-Term Average Precipitation at Fording River Cominco (1970 to 2016)	659

The climate data in Table 10 indicate the annual precipitation received at FRO from 1 September 2016 to 31 August 2017 was average when compared to the long-term historical average (based on the available data).

4.2 Water Balance

4.2.1 North Tailings Pond Water Balance

The annual NTP water balance is summarized in Table 11.

Table 11: 2017 North Tailings Pond Water Balance	11 Sor	stombor 2016 to	31 August 2017)
Table 11. 2017 North Tallings Fond Water Balance	(1 Och		31 August 2017)

IN	Annual Volume (Flows ×1,000 m³/ 12 month)	OUT/LOSSES	Annual Volume (Flows ×1,000 m³/ 12 month)	Total Inventory Change (Flows ×1,000 m³/ 12 month)
Surface water	353	Evaporation	41	
Precipitation	29	Seepage loss	349	-
Sum	382	Sum	390	-9

Note: Total inventory change may not exactly equal the sum of inflow minus the sum of outflow due to rounding errors.

The water balance for the NTP was updated using a revised storage-elevation-area curve and catchment area. The revised catchment area is about three times the previous estimate in the water balance model, which has resulted in an increase to the estimated the total surface water runoff inflows to and seepage losses from the NTP.

The water balance model for the NTP estimates a seepage loss of 349,000 m³ from 1 September 2016 through 31 August 2017. The total seepage volume is higher than previous years, which is attributed to the change in catchment area and corresponding increase in surface water inflows to the NTP.





4.2.2 South Tailings Pond Water Balance

Teck identified the need for model improvements for the STP water balance during the 2016 DSI, specifically related to its ability to (i) model water levels in the STP, and (ii) account for movement of solids (i.e., tailings) through deposition and dredging activities. Based on these needs, the STP water balance was updated in 2017 as follows:

- Incorporated dynamic storage elevation and storage area curves for the STP based on historical bathymetric surveys, to improve the ability of the model to predict pond water elevations.
- Incorporated updated dredging rates from the STP to Turnbull TSF.
- Incorporated dredge line inputs to facilitate tracking of solids and water within the STP and from the STP to Turnbull TSF.
- Incorporated a new dashboard to present the tailings solids mass balance, i.e., inflows and outflows within the STP, and accumulated tailings mass in 3 Pit and Turnbull TSF.

The September 2016 to August 2017 STP water balance, based on the updated version of the model, is summarized in Table 12.

IN	Annual Volume (Flows ×1,000 m³/ 12 months)	OUT/LOSSES	Annual Volume (Flows ×1,000 m³/ 12 months)	Total Inventory Change (Flows ×1,000 m³/ 12 months)
Runoff water	400	Retained in Tailings	0	
Process make-up water	3,530	Dredge slurry to Turnbull TSF	901	
Precipitation	110	Evaporation	149	-
Miscellaneous	432	Clarified water return	22,774	
Tailings slurry	22,080	Seepage loss	2,637	
Sum	26,552	Sum	26,461	91

Table 12: 2017 South Tailings Pond Water Balance (1 September 2016 to 31 August 2017)

Note: Total inventory change may not exactly equal the sum of inflow minus the sum of outflow due to rounding errors.

3PS = 3 Pit South; Turnbull TSF = Turnbull Tailings Storage Facility.

The water balance assessment included a comparison of modelled and monitored water levels for calibration. The water balance model indicates a seepage loss estimation of 2,637,000 m³ from 1 September 2016 through 31 August 2017.

The STP mass balance is approximately neutral, based on the volume of deposited tailings and the volume of dredged tailings being nearly equal over the reporting period. As a result, the volume of retained water in the net new tailings is negligible. Note that the dredging continued past the end of the reporting period, and the annual volume of dredged tailings exceeded the estimated volume of deposited tailings by October 2017 (Section3.2).





4.3 Water Quality Monitoring

Teck monitors water quality monitoring in and around the NTP and STP facilities as follows:

- NTP at discharge line
- STP north seep (at culverts)
- STP southwest corner (pond at toe of dam)
- STP west seep (embankment below West Dam)
- STP barge
- STP northwest pond (pond west of wastewater cells)
- STP groundwater wells (northwest of STP)
- STP at discharge line

Water quality monitoring data is submitted to the BC Ministry of Environment for compliance reporting. Water quality testing results are provided in Appendix E for completeness, but assessment of the water quality results are beyond the scope of this DSI.

4.4 Tailings Storage Facility Registry

The tailings storage facility registry for the NTP and STP are included in Appendix F.





5.0 TAILINGS FACILITY DAM SAFETY ASSESSMENT

This section presents the dam safety assessment of the NTP and STP facilities based on the observations and data review for each of the failure modes that are most relevant to this type of dam.

5.1 Method

5.1.1 Site Visit

A site inspection was carried out on 3 to 5 October 2017 by Mr. John Cunning, P.Eng., and M., E.I.T., of Golder, accompanied by Ms. Heather Brickner, P.Eng., of FRO.

The temperature during the visit was between approximately 0°C and 10°C and the weather was clear and sunny.

Appendix A presents a summary of photographs of NTP and STP from the site inspection. The location, direction, and number for each photograph are noted in Figures 2 and 5.

A summary of the observations is included in the inspection reports in Appendix B and C for the NTP and STP, respectively. In general, the NTP and STP were observed to be in good condition at the time of the 2017 annual inspection. Details of the site inspection are discussed in Sections 5.3 and 5.4.

5.1.2 Review of Background Information

FRO provided the following information for this DSI:

- 2017 FRO site LiDAR survey data
- 2017 FRO site air photo
- 2017 tailings pond bathymetric survey data for the STP
- 2016 and 2017 FRO site climate data
- vibrating wire (VW) piezometer and pond water level data
- dam movement data: prism survey on the NTP, GPS monitoring data and slope inclinometers on the NTP and STP
- records of visual inspections
- records of construction completed





5.2 **Review of Operational Documents**

5.2.1 Operation, Maintenance and Surveillance Manual

The most recent update of the operation, maintenance, and surveillance (OMS) manual was completed by FRO in 2015 (FRO 2015b). As required by HSRC Section 10.5.2 (MEM 2017), FRO is currently completing a review and update of the OMS manual for the NTP and STP which is expected to be issued in Q2 2018.

Updates to the OMS manual should include:

- Update all references to consequence classification of structures—change from High to Very High.
- Include design criteria.
- Review the manual using the updated HSRC and Guidance Document (MEM 2017, 2016).
- Review the manual using most recent MAC guidelines
- QPOs to be included for surveillance.
- The dredging section needs to be updated to identify that dredging is currently operating to the Turnbull Tailings Storage Facility.
- Include safe work plans.
- Include incident reporting procedures.
- Include non-compliance reporting procedures.
- Include animal burrow inspection and procedures
- Include NTP pipe inspections
- Include hydroseeding records
- Include Liverpool and NTP boundaries

5.2.2 Emergency Preparedness Plan / Emergency Response Plan

An update to the 2015 version of the emergency response plan (ERP) is in draft for the NTP and STP (SP&P EP.009) (FRO 2017a). The updated ERP now includes the in pit tailings storage facilities on site. This document was updated to meet the guidelines provided by the HSRC (MEM 2016, 2017), the CDA (2013), the Mining Association of Canada (MAC 2011, 2017), and Teck Resources Limited standard guidelines (Teck 2014).

FRO has also developed a *Tailings Impoundment Flood Response Protocol for the Fording River*. This document was issued on 26 September 2017 (FRO 2017c).

The emergency preparedness plan (EP.008.R1) was last updated on 15 December 2015 (FRO 2015a). FRO plans to update the emergency preparedness plan once the ERP is finalized as the document will outline the warnings FRO will issue and the expected actions of local authorities and other responders for dam breach flood emergencies.





The emergency planning documents should continue to be reviewed at least annually, with updates incorporated when required. The ERP should be tested annually. FRO plans to tabletop test the ERP in 2018.

5.2.3 Dam Safety Review

The most recent DSR of the NTP and STP was completed in 2014 (KCB 2014). A DSR is required every five years for all water and tailings storage facilities regardless of dam consequence classification according to HSRC Section 10.5.4 (MEM 2017). The next DSR is required in 2019.

5.3 North Tailings Pond

The record of inspection for FRO's NTP is included in Appendix B. A plan of the NTP with the location of the monitoring points is shown in Figure 3, and a typical section of the NTP retaining dam is shown in Figure 4.

This section presents an assessment of dam safety for the NTP dam based on observations and data review and includes a review of the 2016 recommendations for the facility.

5.3.1 Assessment of Dam Safety Relative to Potential Failure Modes

A summary of the assessment and potential failure modes is presented in Table 13.

Potential Failure Mode	Observations/Data	Comments	
Internal erosion (suffusion and piping)	Filter compatibility is generally met between materials except for the tailings and the foundation flood plain sand and gravel	The potential filter inadequacy between the tailings and foundation is considered a low risk. Portions of the NTP tailings deposit is drained with no driving head for seepage will not impact the stability of the dam, and the stability is not reliant on the tailings Migration of the tailings through the sand and gravel is considered a low risk.	
Overtopping	Within acceptable range based on pond elevations over reporting period	Draft IDF and freeboard assessment complete, in review with Teck. Anticipated changes in freeboard requirements expected to be manageable.	
Instability	No evident instability	Draft seismic stability assessment complete, in review with Teck. No anticipated changes to stability management.	

Table 13: Assessment of Dam Safety Relative to Potential Failure Modes

HSRC = Health, Safety and Reclamation Code.





5.3.1.1 Internal Erosion (Suffusion and Piping)

Internal instability of a dam can be caused by materials migrating out of the dam, leaving voids. This generally happens with materials that do not have filter compatibility; that is, the fines fraction of one material can migrate into or through the voids of the adjacent material under a sufficient hydraulic gradient. Piping is induced by regressive erosion of particles towards an outside environment until a continuous pipe is formed. Suffusion is the migration of soil particles through the soil matrix.

Design Basis

Filter compatibility was reviewed using the internal stability criteria based on grain size distributions of till and CR or CCFR as part of the NTP flood repairs. Tailings grain size distribution testing was completed during an investigation of the existing coal tailings in 2 Pit, 3 Pit, and the NTP (Golder 2012b). The review indicates that the filter compatibility criteria are met between the till and the CR or CCFR, and between tailings and till.

Another filter compatibility assessment was completed by Golder in 2015 in response to a February 2015 MEM order to undertake an assessment to determine if the dams associated with the tailings facilities on site may be at risk due to filter inadequacy (Golder 2015a). The following filter relationships were checked:

- compatibility between the upstream till blanket (base soil) and CR shell (filter)
- compatibility between the till cut-off (base soil) and flood plain sand and gravel foundation (filter)
- internal stability of the CR shell (filter)

The compatibility between the till (base soil) and the CR shell or flood plain sand and gravel (filter) met the filter compatibility criteria for two of the three methods checked. The internal stability of the CR shell was confirmed. Based on the performance of the dam over the last 45 years, piping due to filter incompatible material is not expected to be an issue.

The CR shell, which acts as a filter for the upstream till blanket, was constructed in accordance with the design. While not explicitly stated in the reports (Golder Brawner 1973, 1974b), the Terzaghi method was likely the method used to confirm compatibility during design and construction.

It is noted that there are some gaps in construction quality control records. Where data were available, they indicated that filter compatibility was achieved. The gaps in the quality control records are considered to be low risk to confirming filter compatibility.

The filter compatibility of the underlying flood plain sands and gravels (filter) was assessed against the CR/CCFR, till, and tailings (bases) by Golder based on the 2015 investigation results (FRO 2016). Filter compatibility was checked using the Sherard and Dunnigan 1989 criteria, as recommended by the CDA (2007). The results showed that the filter compatibility is generally met between the sands and gravels and the CR/CCFR, as well as with the sands and gravels and till. However, filter compatibility between the tailings and the flood plain sand and gravel is not generally met. The potential filter inadequacy between the foundation and tailings will not impact the stability of the dam, as the stability is not reliant on the tailings. Migration of the tailings through the sand and gravel is expected to be contained by the till cut off, and therefore a low risk.





Observed Performance

The key observations made during the NTP dam inspection were as follows:

- No significant zones of external seepage were observed that would indicate the possible development of internal piping.
- No zones of subsidence or any sinkholes were observed that would indicate voids due to either suffusion or piping.

5.3.1.2 Overtopping

Design Basis

CDA (2013) provides two calculations for freeboard; the more critical of the two cases sets the minimum freeboard:

- no overtopping by 95% of the waves caused by the most critical wind with a return period of 1 in 1,000 years with the pond at its maximum normal operating elevation
- no overtopping by 95% of the waves caused by the most critical wind with a return period of 1 in 2 years (for Very High consequence structures), with the pond at the maximum level during the passage of IDF

The current minimum crest elevation of the dam at the NTP is 1,652.6 m (2017 survey completed by FRO).

The HSRC Guidance Document (MEM 2016) recommends that the IDF be designed to 2/3 between the 1,000-year flood/storm event and the probable maximum flood for a structure classified as Very High consequence. The HSRC also requires that a facility that stores the IDF use a minimum event duration of 72 hours plus snowmelt. The current minimum freeboard is 1.2 m (elev. 1,651.4 m). An updated IDF and freeboard assessment has been completed in draft and updates to the maximum normal operating water level and freeboard are expected in 2018.

The NTP currently has no inputs of water except direct precipitation and some runoff from a small local catchment area. The water levels are generally maintained with over 2 m of freeboard and pumping and dewatering is not required under normal annual conditions. In the event that critical water levels in the pond are approached, FRO's contingency plan includes installing a diesel pump to transfer excess water to the Shandley Pond. The pump will be set up in the southwest corner of the NTP and connected to the nearby T fixture on the Shandley pipeline. This pump is to be inspected and refuelled daily while needed (FRO 2017a). The pump should be tested annually. The NTP is not equipped with an emergency spillway because a normal minimum freeboard of 1.2 m (as assessed for High consequence) will be maintained with emergency pumping as necessary. A draft IDF and freeboard assessment for Very High consequence classification is complete and in review with Teck. Anticipated changes in freeboard requirements are expected to be manageable, FRO personnel and the EoR will develop an action plan for the updated freeboard requirements once final.

A passive method of controlling water elevation would be a best practice. Golder has produced feasibility level drawings for an emergency spillway on the NTP (Golder 2015b).



An overtopping failure caused by landslide is a possible failure mode for the NTP due to the adjacent CR spoil to the west of the NTP. The CR spoil was resloped in 2015 as per previous Golder recommendations and FRO analyses (Golder 2014a,e; FRO 2014). This work was performed to reduce the hazard of a potential spoil failure to impact the NTP and create wave action that could potentially overtop and breach the NTP dam. Based on stability and run out analyses, failure of the reconfigured CR spoil and subsequent wave generation is considered unlikely.

Instrumentation

Pond elevation data for the NTP were received from FRO, and Chart 2 presents the variation in pond elevation from 1 September 2016 to 31 August 2017 based on this information.

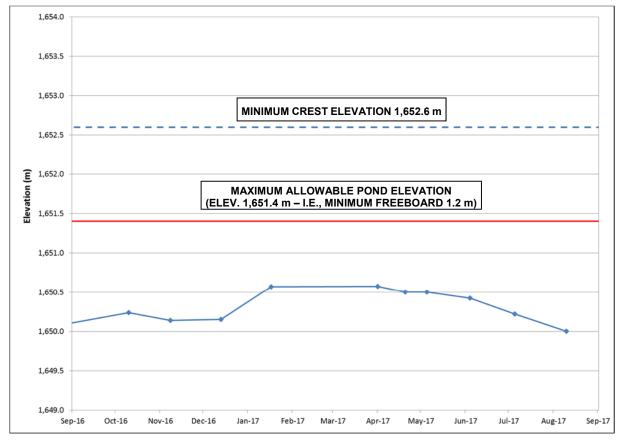


Chart 2: North Tailings Pond Water Elevation from 1 September 2016 to 31 August 2017

Note: Pond elevations reported in Elk Valley Elevation Datum.

The NTP water level is shown to be maintained below the maximum allowable water level. A total of 10 readings were taken during the period between 1 September 2016 and 31 August 2017, with readings missing in September 2016 and February 2017. FRO should survey the pond elevation monthly, and this is reflected in the OMS manual (FRO 2015b).





Observed Performance

The key observations made during the NTP dam inspection were as follows:

- The tailings have filled most of the area upstream of the NTP dam, and there is a small reclaim pond at the southern end. The fetch distance on the surface of the NTP is short, so the potential for generation of significant waves when a pond is present is small.
- Unused and damaged pipelines that extend through the crest of the dam should ideally be removed or grouted to eliminate the hazard of future deformation or settlement of the abandoned pipes creating low points in the dam crest (locations shown in Golder 2017b).
- All pipes should be inspected as part of the monthly NTP inspections until removed or grouted or until the NTP is decommissioned.

5.3.1.3 Instability

The stability of the NTP is monitored with piezometers, inclinometers, prisms, GPS units, and regular visual inspections.

Design Basis

The drained conditions beneath the NTP dam are favourable with respect to structural stability. The downstream slope of sections rebuilt after the June 2013 flood is less steep (1.5 to 1.75H:1V) than the original design (1.3 to 1.4H:1V).

In situ and laboratory results were used by Golder to assess the liquefaction potential of the floodplain foundation soils, per the recommendations from the 2014 DSR (KCB 2014). Results indicated that alluvial soils downstream of the dams are not likely to liquefy, and removes the requirement for any further work related to the DSR recommendation for stability analysis of the liquefied foundation condition. A detailed discussion of the results is provided in Golder (2016d).

A dam stability update for the NTP was completed by Golder (2016e) based on the drilling program conducted by FRO in 2015 (FRO 2016) in accordance with the CDA (2013) guidelines and related technical bulletins (CDA 2007, 2014). The 2,475-year earthquake event was selected (2% exceedance probability in 50 years) for long-term stability analyses under pseudo-static loading conditions as recommended by the CDA (2013), and a peak ground acceleration of 0.158 g was used in the pseudo-static analyses based on the site-specific seismic hazard assessment completed by Golder (2016a). Based on the HSRC, this assessment is out of date since a return period ½ between the 2,475-year and 10,000-year seismic event or the maximum credible earthquake should be used for a facility with a consequence classification of Very High. A draft seismic stability assessment has been completed and is in review with Teck. There are no anticipated changes to stability management based on the Very High consequence seismic stability assessment. The NTP is also susceptible to instability from erosion during flooding of the Fording River. This has been assessed by KWL and riprap was placed on the toe of the dam in late 2016 and 2017 (KWL 2017c) to mitigate against erosion.





Instrumentation Data – Crest Displacement Monitoring

Seven survey prisms (NT1 to NT7) and a GPS monitor (NTP-GPS 01) are installed along the crest of the NTP dam, and one reference prism is located at the wash plant location (NT10), as shown in Figure 3. FRO has improved the prism reading accuracy by installing a back site prism for the NTP (Prism M102).

Prism data were provided by FRO and the GPS data was downloaded from GeoExplorer. The grid system was updated in October 2016, therefore data for the prisms is shown from October 2016 through August 2017 (no historical data). The GPS data were downloaded for the reporting period, 1 September 2016 to 31 August 2017. The survey data are summarized in Appendix G.

Table 14 lists the prism and GPS units in use at the NTP.

Instrument Identification	Instrument	Northing (m)	Easting (m)	Elevation (m)
NT1	Prism	5,562,034.0	651,126.3	1,654.3
NT2	Prism	5,561,884.1	651,130.1	1,653.9
NT3	Prism	5,561,735.3	651,087.4	1,653.8
NT4	Prism	5,561,597.1	651,028.4	1,654.4
NT5	Prism	5,561,462.6	650,957.7	1,654.2
NT6	Prism	5,561,326.9	650,876.2	1,654.1
NT7	Prism	5,561,225.1	650,766.9	1,653.7
NT10	Prism	5,561,586.7	651,257.6	1,655.5
NTP-GPS 01	GPS	5,562,143.7	651,102.6	1,645.5

Table 14: Instrument Monitoring Locations

Note: Northing and Easting reported in FRO UTM, Elevations reported in NAD83. Sensor locations downloaded from GeoExplorer.

FRO = Fording River Operations; NTP = North Tailings Pond.

NT10 is a reference prism.

The total movement (3D displacement) measured for each survey prism relative to the original installation position is shown in Chart 3. The total displacement presented is the vector sum of the horizontal and vertical displacements.





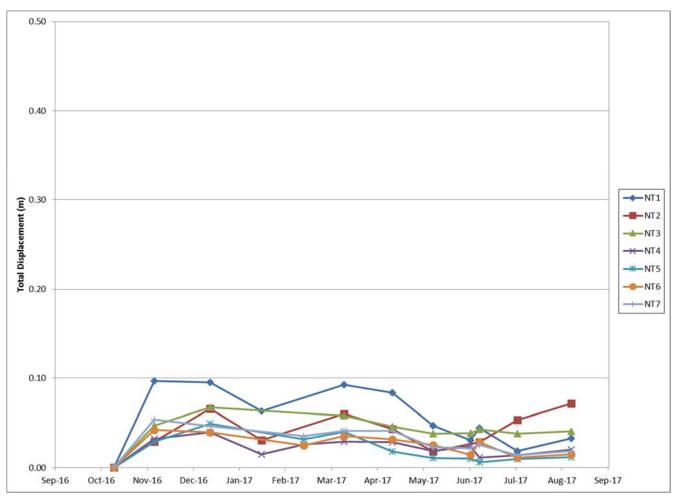


Chart 3: North Tailings Pond Prism Total 3D Displacement from 1 September 2016 to 31 August 2017

Note: Elevations reported in NAD 83.

Displacements have been zeroed at the start of the reporting period to assess annual trends.

Generally, the prism data are surveyed once a month. The survey data indicate little crest displacement during the reporting period. Increase in movement from October to November 2016 could be related to the large storm that was experienced in the Elk Valley in October 2016. As well, the majority of the prisms show a slight increase in movement during the spring freshet (March to April 2017) but they appear to have settled throughout the remainder of the reporting period. Movements are well below the GeoExplorer trigger for 3D movement (200 mm, GeoExplorer warning and the updated QPOs provided by Golder (Section 2.6).

The OMS manual outlines that the prisms should be shot once a month, three times each, to ensure accuracy. FRO followed this practice for the reporting period.

The 3D point velocity for GPS unit NTP-GPS 01 is presented in Chart 4.



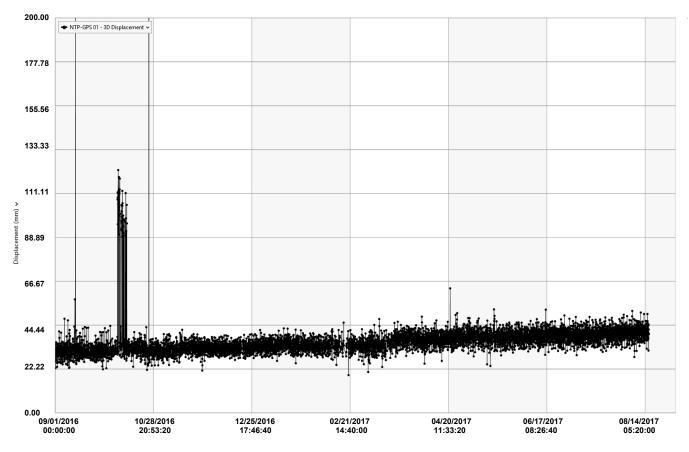


Chart 4: North Tailings Pond 3D Displacement from 1 September 2016 to 31 August 2017

Generally, the GPS device NTP-GPS 01 records on an hourly frequency. The survey data indicate little crest displacement during the reporting period. Minor spikes in NTP-GPS 01 are most likely noise in the system and are not a concern. Movements are well below the GeoExplorer alarm trigger for 3D point velocity (300 mm/day, GeoExplorer alarm) and the updated QPOs provided by Golder (Section 2.6), with the exception of some noise. The warning in GeoExplorer was triggered in September and October 2016. All warnings were investigated by FRO personnel and determined to be erroneous data.

A latent alarm is triggered in GeoExplorer when the measurement age of the GPS unit is greater than a day and the prisms is greater than five days on the NTP. Any offline monitors will be inspected and repaired within one week (FRO 2015b).

Instrumentation Data – Slope Inclinometers

Slope inclinometers were installed at three locations in 2015 along the NTP crest (Figure 3) to monitor horizontal movement in the dam. The A axis is oriented in the upstream to downstream direction (with negative displacements in the downstream direction) and the B axis is oriented along the dam centreline. The location of the inclinometers at the NTP is presented in Table 15.





Inclinometer ID	Northing (m)	Easting (m)	Elevation (m)	A-A Axis Azimuth (°)	Probe Serial No.	Reel Serial No.
TH15-05	5,561,992.0	651,130.8	1,653.6	235		
TH15-06	5,561,641.0	651,047.2	1,653.7	290	DP15600000	DR21300000
TH15-07	5,561,379.7	650,904.4	1,653.4	305		

Table 15: North Tailings Pond Inclinometers

Note: Azimuth is approximate. The upper wheel should face the indicated direction for the first set of readings.

Elevations reported in Elk Valley Elevation Datum.

TH = test hole.

Raw inclinometer data were supplied to Golder by FRO. Readings have been taken approximately quarterly at the NTP inclinometers since December 2015. A total of three readings were taken at each of the three inclinometers between August 2016 and August 2017. Once received, the inclinometer data were interpreted by Golder (Appendix H). Data readings are from 23 January 2016 to 19 July 2017, and include the initial reading from 18 December 2015 as a reference line.

The maximum deflection observed in both direction A and B does not exceed approximately 4 mm for inclinometer TH15-05, which is an acceptable range (Appendix H, Figure H-1).

The maximum deflection observed in both direction A and B reaches a maximum of approximately 6 mm for inclinometer TH15-06, which can likely be attributed to a section of exposed casing above the dam crest which is experiencing movement during data acquisition. Below the first metre of casing, direction A and B appear not to experience more than 4 mm of deflection (Appendix H, Figure H-2).

The maximum deflection observed in both direction A and B does not exceed approximately 6 mm for inclinometer TH15-07 (Appendix H, Figure H-3).

The inclinometer readings do not indicated any significant trends in deformation.

Instrumentation Data – Piezometers

VW piezometers were installed in 2015 at three locations along the NTP crest (Figure 3) to monitor water levels in and below the dam; each location includes sensors at two or three depths. The piezometers located in the NTP are listed in Table 16. Data for the piezometers was provided to Golder by FRO. Chart 5 presents the piezometer readings from 1 September 2016 to 31 August 2017, as well as the NTP pond elevation over the same time period. Readings have been taken at the NTP piezometers since August 2015.





Piezometer ID	Northing (m)	Easting (m)	Elevation (m)	Data Logger Serial No.	Piezometer Serial No.	Piezometer Elevation (m)	
				DT09633	VW33222	1,641.3	
TH15-05	5,561,992.0	651,130.8 1,653.6	651,130.8	1,653.6	DT09636	VW33223	1,638.7
			DT09638	VW33241	1,635.6		
TH15-06	5,561,641.0	E EC1 C11 0	054 047 0	1,653.7	DT09641	VW33240	1,628.5
1010-00		651,047.2	DT09643	DT09643	VW33239	1,626.3	
					VW33231	1,630.0	
TH15-07 5	5,561,379.7	650,904.4	1,653.4	DT094501	VW33230	1,624.0	
					VW33242	1,614.7	

Table 16: North Tailings Pond Piezometer Installation Details

Note: Coordinates reported in UTM and elevations reported in Elk Valley Elevation Datum.

TH = test hole.





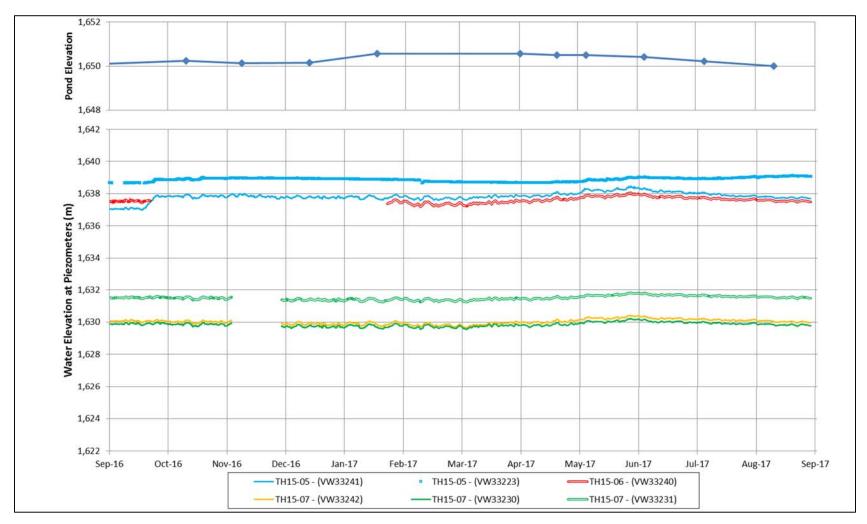


Chart 5: North Tailings Pond Vibrating Wire Piezometers and Pond Elevation from 1 September 2016 to 31 August 2017

Note: Elevations reported in Elk Valley Elevation Datum. TH15-05 (VW33222) and TH15-06 (VW33239) are dry and not presented in Chart 5.





The piezometers' performance was assessed and is summarized in Table 17.

Piezometer ID	Piezometer Serial No.	Piezometer Elevation (m)	Minimum (2016/2017)	Maximum (2016/2017)	Upward Gradient	Exceed Alarm	Comments
	VW33222	1,641.3	n/a	n/a	n/a	No	Negative water level (dry).
TH15-05	VW33223	1,638.7	1,638.6	1,639.1	No	No	Missing data in September 2016, and January 2017.
	VW33241	1,635.6	1,637.0	1,638.4	No	No	Jump of nearly 1 m in September 2016.
TH15-06	VW33240	1,628.5	1,637.1	1,637.9	No	No	Erroneous data from end of September 2016 to January 2017 due to logger malfunction. Data removed from plot and battery replaced.
	VW33239	1,626.3	n/a	n/a	n/a	No	Likely malfunctioning, negative water level, still see freshet trend in piezometer data.
	VW33231	1,630.0	1,631.2	1,631.8	No	No	Missing data in November 2016due to the data logger for TH15-07 being damaged by water.
TH15-07	VW33230	1,624.0	1,629.6	1,630.2	No	No	Missing data in November 2016 due to the data logger for TH15-07 being damaged by water.
	VW33242	1,614.7	1,629.6	1,630.4	Yes	No	Missing data in November 2016 due to the data logger for TH15-07 being damaged by water.

Table	17: N	North	Tailings	Pond	Piezometer	Performance	Summary
IUNIC		101111	runnigo	1 0110	1 10201110101	1 chionnanoc	Cumunary

Note: Elevations reported in Elk Valley Elevation Datum.

Alarms from GeoExplorer TH = test hole; n/a = not applicable.





The phreatic level readings for the time period were generally stable with trends related to spring freshet seen in most sensors in April and May 2017. No warnings were triggered in GeoExplorer for these piezometers.

TH15-05 (VW33241) had a sharp increase of approximately 1 m in head in September 2016. The readings settle out after this increase and show consistent trends with the other piezometers.

The upper VW sensor in TH15-05 (VW33222) was above the phreatic surface and was dry (negative water level readings). The piezometer readings for TH15-06 were reviewed because the lower piezometer (VW33239) was reading a negative water level while the upper piezometer was reading approximately 9 m of water above the piezometer. The sensor is still showing similar trends in water elevation as the other piezometers even though a negative water level is being read. After reviewing FRO installation records and checking the data logger in the field, it was determined that VW33239 is likely malfunctioning.

TH15-05 (V33223) shows the spring freshet trend similar to other sensors in the NTP, but the head does not decrease in the summer months, and continues to trend upward throughout August 2017. Data past August 2017 was reviewed in GeoExplorer and the water elevation reduces through September 2017.

There is an upward gradient in TH15-07 between VW33242 and VW33230. These piezometers are located in differing stratigraphic units, VW33242 is in glacial till and VW33230 is in fluvial material (Golder 2016e). Because the Fording River is close to this area of the NTP toe, there is the possibility that the piezometers may be influenced by the river. The gradient is small and is not a concern at this time. The piezometers should continue to be monitored on a regular basis as outlined in the OMS manual and the gradients should be checked as good practice.

At multiple times over the reporting period missing data was reported. To avoid this in the future, GeoExplorer has been updated to show "No Communication" and "No Frequency" alarms that alert FRO when the piezometers are not reading data. FRO will use these alarms as an indication that the piezometers are malfunctioning and will send someone to check on the instrument in question.

Observed Performance

No evidence of major slope instability was observed during the 2017 DSI. The key observations made during the NTP dam inspection were as follows:

- Minor surficial stepped erosion was noted on the north and central sections of the downstream slope (Appendix A, Photograph 7). This is considered to be cosmetic and will not affect the overall stability of the dam.
- Minor erosion created by surface water runoff was noted near the south end of the tailings pond in the 2015 DSI, but has since been repaired and appears stable (Appendix A, Photograph 3).
- A wet area with previous evidence of ponding was noted below the highest point of the NTP dam in 2016. Most likely due to freezing conditions during the site visit, this wet area was not observed in 2017.





5.3.1.4 River Erosion Protection (KWL)

KWL completed an inspection of the riprap at the toe of NTP in 2017 and the inspection report is provided in Appendix I (KWL 2018).

5.3.2 Review of Previous Deficiencies and Non-conformances

The deficiencies and non-conformances presented in Table 18 were noted in the previous DSI (Golder 2017b). Table 18 provides the current status of the 2016 DSI recommendations for the NTP. Items from the 2016 DSI that are incomplete have been brought forward into the 2017 DSI recommendations (Table 29).

There are a number of recommendations that are in progress and some that are incomplete, however Golder feels the work is being appropriately prioritized based on good communication between the EoR team and the FRO engineer teams.





Table 18: Current Status of 2016 Dam Safety Inspection Recommended Actions for North Tailings Pond Facility

ID Number	Deficiency or Non-conformance	Recommended Action	
2015 05a b	No passive emergency system against overtopping;	Assess the need for spillway after finalizing the closure plan NTP.	Incomplete deadline
2015-05a,b emergency system requires active response		If required, determine a construction schedule.	Incomplete deadline
		Perform risk-informed assessment to determine appropriate flood protection requirements for downstream toe of dam along Fording River and timeline to implement.	Incomplete deadline
2015-06a,b,c	Risk-informed criteria for flood erosion protection along toe of dams not defined	Implement required protection measures for the operational phase according to the as-defined schedule.	Incomplete deadline
		Execute the flood risk mitigation plan until the flood protection requirements defined by the risk- informed assessment are in place.	Ongoing
2015-07a,b	Buried pipes passing through crest locations	Complete review of pipe abandonment timeline as part of feasibility investigation into NTP decommissioning.	Complete - abandonme upstream si
	-07a,b Buried pipes passing through crest locations If required, execute abandonment plan. Assess and revise required internal and external communication carried out near the site TSFs. Sediment pond designer to sign off on reconstruction of NTP dar	If required, execute abandonment plan.	Incomplete deadline
		Assess and revise required internal and external communication for work and construction activities carried out near the site TSFs.	In Progress updated rec
2016-05	North abutment excavated without input or approvals	Sediment pond designer to sign off on reconstruction of NTP dam north abutment.	Complete (
2010-03	m Engineer of Record or Qualified Person	Engineer of Record to review final NTP dam re-construction summary.	Complete - considered Pond Syste
2016-06	No closure plan for NTP	Develop closure plan for NTP based on results of feasibility investigation into NTP decommissioning.	In Progress updated rec
		Fill voids in downstream slope created by burrows.	Complete
2016-7a,b	Additional animal burrows in toe of south abutment	Perform regular inspection for new burrows. Burrows to be marked or identified by GPS during inspection to improve tracking of ongoing efforts.	Ongoing, in
2016-11	Discrepancy within the documentation regarding the minimum elevation of the NTP crest. The lower elevation value has been reported here (i.e., more conservative)	The NTP dam, particularly the dam crest, be surveyed to confirm the minimum dam elevation.	Complete
2015-03	Roles of Geotechnical and Hydraulics Engineers of Record undocumented	Golder, FRO, and KWL to document the roles of the Engineer of Record for the geotechnical and hydraulics related works in the OMS manual.	In Progress updated rec
2016 01c h	Seismic design criteria for stability out of date due to	Complete updated seismic stability assessment and liquefaction based on revised design criteria.	In Progress or exceeds recommend
2016-01a,b dam reclassification from High to Very High		Update QPOs based on revised stability assessment.	In Progress prism QPOs completion
2016-02	IDF and freeboard out of date due to dam reclassification from High to Very High	Update the IDF and freeboard assessment for the NTP and STP.	In Progress updated rec

Updated Status as of March 2018

ete – see Table 29 for updated recommended action and

ete – see Table 29 for updated recommended action and

ete – see Table 29for updated recommended action and

ete-- see Table 29for updated recommended action and

e – Golder provided recommendations for pipe ment in November 2017, pipes are currently capped on a side and are inspected regularly

ete – see Table 29 for updated recommended action and

ess – to be included in OMS manual; see Table 29 for recommended action and deadline

e (AMEC 2017)

te – Liverpool outlet channel and fish barrier should now be ed outside of NTP facility and part of Liverpool Sediment stem.

ess – prefeasibility design completed; see Table 29 for recommended action and deadline

е

, include in OMS manual update

е

ess – to be included in OMS manual; see Table 29 for recommended action and deadline

ess – draft assessment complete, seismic stability meets ds new design criteria; see Table 29 for updated ended action and deadline

ess – pending completion by Golder, updated GPS & Os in this DSI report; updated piezometer QPOs pending on by Golder

ess – draft assessment complete; see Table 29 for recommended action and deadline





ID Number	Deficiency or Non-conformance	Recommended Action	
2016-03	OMS manual requires updating	 Update OMS manual as follows: Update all references to consequence classification of structures—change from High to Very High. Include design criteria. Review the manual using the updated HSRC and Guidance Document (MEM 2017, 2016). QPOs to be included for surveillance. The dredging section needs to be updated to identify that dredging is currently operating to the Turnbull Tailings Storage Facility. Include safe work plans. Include incident reporting procedures. Complete minor updates identified in the 2015 DSI report (Golder 2016b). 	In Progres completion action and
2016-04	EPP & ERP require updating	Reference to the QPOs needs to be included for actions required based on instrumentation warnings and alarms.	In Progres
2016-09	No QPOs set for inclinometers	QPOs and frequency of readings should be set for the inclinometers.	In Progres
2016-10	Warning level QPO for piezometers exceeded. Based on review of data, this is not a failure concern	Update warning level QPOs for piezometers based on review of data all available data (2014 to present).	In Progres

IDF = inflow design flood; FRO = Fording River Operations; KWL = Kerr Wood Leidal Associates Ltd.; NTP = North Tailings Pond; STP = South Tailings Pond; HSRC = Health, Safety and Reclamation Code; DSI = dam safety inspection; TSF = tailings storage facility; OMS = operation, maintenance and surveillance; EPP = Emergency Preparedness Plan; ERP = Emergency Response Plan; QPO = quantitative performance objectives.

Updated Status as of March 2018

ress – under review by FRO, QPO update pending ion by Golder; see Table 29 for updated recommended nd deadline

ess – pending completion by FRO; see Table 29 for recommended action and deadline

ess – draft complete; see Table 29 for updated ended action and deadline

ess – pending completion by Golder, recommendation ted in 2017-06





5.4 South Tailings Pond

The record of inspection for the FRO STP is included in Appendix C. A plan of the STP with the location of the monitoring points is shown in Figure 6, and typical sections of the STP dams are shown in Figures 7 and 8.

This section presents an assessment of dam safety for the STP dam based on observations and data review and includes a review of the 2016 recommendations for the facility.

5.4.1 Assessment of Dam Safety Relative to Potential Failure Modes

A summary of the assessment and potential failure modes is presented in Table 19.

Potential Failure Mode	Observations/Data	Comments
Internal erosion (suffusion and piping)	Filter compatibility is generally met between materials except for the tailings and the flood plain sand and gravel	The potential filter inadequacy between the foundation and tailings will not impact the stability of the dam, as the stability is not reliant on the tailings. Migration of the tailings through the sand and gravel is considered low risk.
	Ongoing seepage monitoring since 2015	Seasonal trends evident in seepage data collected.
Overtopping	Within acceptable range based on pond elevations over reporting period	Draft freeboard assessment complete, in review with Teck. FRO and EoR to develop action plan and schedule to address expected changes in freeboard requirements.
Instability	No evident instability	Draft seismic stability assessment complete, in review with Teck. No anticipated changes to stability management.

Table 19: Assessment of Dam Safety Relative to Potential Failure Modes

HSRC = Health, Safety and Reclamation Code.

5.4.1.1 Internal Erosion (Suffusion and Piping)

Design Basis

Filter compatibility was reviewed using the internal stability (piping) criteria based on grain size distributions of till and CR or CCFR in the design review (Golder 2011). The review indicates that the filter compatibility criteria are met between the till and the CR or CCFR and between tailings and till.

Another filter compatibility assessment was completed by Golder in 2015 in response to the MEM (Golder 2015a). The STP is constructed with an upstream till blanket and a CR (lower portion) or CCFR (upper portion) shell, which also acts as the filter. A till cut-off was constructed through the flood plain sand and gravel foundation of the STP.





The following filter relationships were checked:

- compatibility between the upstream till blanket (base soil) and CR/CCFR shell (filter)
- compatibility between the till cut-off (base soil) and flood plain sand and gravel foundation (filter)
- internal stability of the CR/CCFR shell (filter)

The compatibility between the till (base soil) and the CR/CCFR shell or flood plain sand and gravel (filter) met the filter compatibility criteria for two of the three methods checked. The internal stability of the CR/CCFR shell was confirmed.

The filter compatibility of the underlying flood plain sands and gravels (filter) was assessed against the CR/CCFR, till, and tailings (bases) by Golder in 2016 per the recommendations made by CDA (2007) to use the Sherard and Dunnigan (1989) criteria. The results showed that the filter compatibility is generally met between the sands and gravels and the CR/CCFR, as well as with the sands and gravels and till. However, filter compatibility between the tailings and the flood plain sand and gravel is not generally met. The potential filter inadequacy between the foundation and tailings will not impact the stability of the dam, and the stability is not reliant on the tailings. Migration of the tailings through the sand and gravel is expected to be contained by the till cut off, and therefore a low risk. No tailings have been observed downstream to date.

It is noted that there are some gaps in construction quality control records, particularly for the 1983 to 1984, 1985 to 1990, and 1993 raises; however, the gradation of the CR and CCFR filter/shell material created by the wash plant appears to have remained relatively consistent from the 1970s to present day (Golder 2015a). Where data were available, they indicated that filter compatibility between the local till and the CR/CCFR was achieved. Gaps in the construction quality control records are considered to be very low risk.

Based on the performance of the dam over the last 40 years, piping through the dam due to filter incompatible materials is not expected to be an issue. Continual seepage is evident in the foundation materials below the toe of the STP dam, particularly along the West Dam, and has been reported for many years. Cloudy seepage water can indicate internal erosion, but records of the seepage from the STP indicate clear water. Regular inspections for evidence of increased seepage and piping should continue.

Quantitative monitoring of seepage at the West Dam began in late 2015 in response to a visual observation of increased year-over-year seepage rates.

Instrumentation Data – Seepage Monitoring

In 1979, shortly after the STP was put into operation, it became apparent that at some location beneath the bottom of the STP, the lower gravel stratum has hydraulic connection with the surficial flood plain gravels that extend over the base of the pond. It is understood that the STP water balance showed unexpected losses.

The total seepage losses from the pond are not measured directly. The estimated rate of seepage loss noted in previous water balances for the STP contains uncertainties resulting from inaccuracies in the water balance modeling, such as not accounting for the mass balance.





The 1 September 2016 through 31 August 2017 water balance model indicates a seepage loss estimate of 2,637,000 m³ or a seepage rate of 5 m³/min. This rate is consistent with estimated historic seepage rates at the STP.

Seepage losses from the STP from 1989, 2000, 2003, and 2006 through 2017 are shown in Table 20. A graphical representation of the observed increase in seepage rate for recent years is provided in Chart 6.

Year	Approximate Average Pond Elevation (m)	Historical FRO Reported Seepage (m³/min)	GoldSIM Seepage (m³/min)
1989	1,629.1	7.5	n/a
2000	1,629.7	4.3	n/a
2003	1,629.5	5.5	n/a
2006	1,629.7	0.4	n/a
2007	1,629.0	3.2	n/a
2008	1,629.5	2.8	n/a
2009	1,630.0	2.3	n/a
2010	1,630.1	1.5	n/a
2011	1,631.9	3.4	n/a
2012	1,632.9	3.9	n/a
2013	1,634.5	10.6	n/a
2014	1,635.5	13.1	n/a
2015	1,636.3	n/a	9.9
2016	1,636.3	n/a	10.4
2017	1,636.2	n/a	5.0

Table 20. Fording	n River Operations F	enorted Seenage	Losses from the S	South Tailings Pond
Table 20. TOTUIN	j nivel Operations r	choiled Sechage		outin rainings ronu

Note: Pond elevations reported in Elk Valley Elevation Datum.

FRO = Fording River Operations; n/a = not applicable.



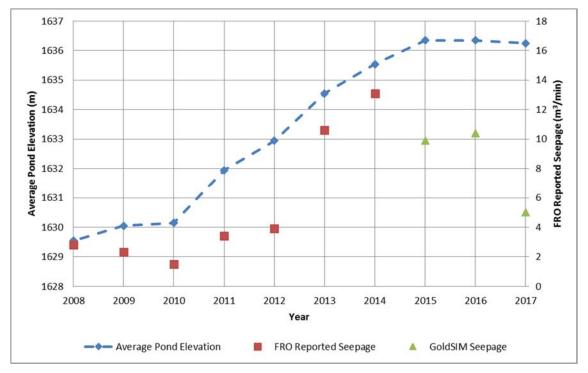


Chart 6: South Tailings Pond Seepage and Pond Elevation, 2008 through 2017

Note: Pond Elevations reported in Elk Valley Elevation Datum.

In response to an increase in the observed seepage below the West Dam, FRO installed two seepage collection pipes within the seepage area in 2015 and has started to collect seepage data. These data should be collected regularly to develop long-term trending of seepage rates in this area. Chart 7 displays the existing seepage and culvert measurement data collected for the STP in 2017.



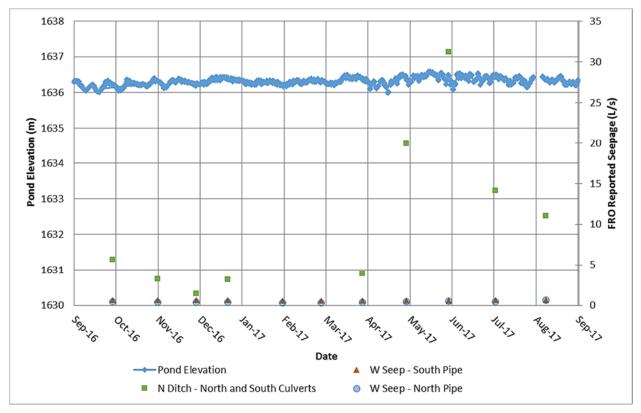


Chart 7: Seepage from West Seep and North Ditch from 29 September 2016 to 11 August 2017

Note: N Ditch North and South culverts include surface flows from Maxam Creek and are not solely seepage measurements. Pond elevations reported in Elk Valley Elevation Datum.

Observed Performance

The key observations made during the STP dam inspection were as follows:

- Seepage continues along the presumed till/bedrock contact below the West Dam (Appendix A, Photograph 39). The seepage has pushed up mats of organics and created a hummocky, broken surface area. This is consistent with previous years. Seepage from the collection pipes at the time of the site visit was approximately 1.5 L/s from each the W Seep North and South pipes. Red staining was noted in some areas of seepage along the bedrock contact (Appendix A, Photograph 39).
- External seepage water was observed to be clear.
- The ditch that captures runoff or seepage at the toe of the West Dam was reinstated (Appendix A, Photograph 37).
- Water flow has been seen in previous years in the ditch along the downstream toe of the north end of the West Dam (Golder 2017b). Due to the cold conditions during the site visit, very little water was observed; however, vegetation growth patterns at the toe of the West Dam indicate some seepage is exiting the dam (Appendix A, Photograph 51). Some of the water is presumed to be surface flow as well.





- No zones of subsidence or any sinkholes were observed that would indicate voids due to either suffusion or piping.
- A pond is present at the north end of the West Dam near Stn. -0+050 (Appendix A, Photograph 33). This pond is assumed to be due to surface water from the Maxam Yard.
- An old river channel is present past the downstream toe of the Main Dam (Appendix A, Photograph 34). This is consistent with previous years.

5.4.1.2 Overtopping

Design Basis

The calculated freeboard and maximum allowable pond levels for the STP were previously calculated as per the CDA design guidelines for High consequence classification (Section 5.4.1.2). The HSRC Guidance Document (MEM 2016) recommends that the IDF be designed to 2/3 between the 1,000-year flood/storm event and the probable maximum flood for a structure classified as Very High consequence. The HSRC also requires that a facility that stores the IDF use a minimum event duration of 72 hours plus snowmelt. The current minimum freeboard for High consequence classification is 1.2 m (elev. 1,636.6 m). A draft IDF and freeboard assessment for Very High consequence classification is complete and in review with Teck. FRO personnel and the EoR will develop an action plan and schedule to address expected changes in the minimum freeboard requirements. The technical bulletin *Application of Dam Safety Guidelines to Mining Dams* (CDA 2014) recommends examination of the condition where the high water level (IDF) occurs at a similar time as a high wind event for calculation of the minimum freeboard.





Instrumentation Data

Pond elevation data for 1 September 2016 to 31 August 2017 at the STP were received from FRO (Chart 8).

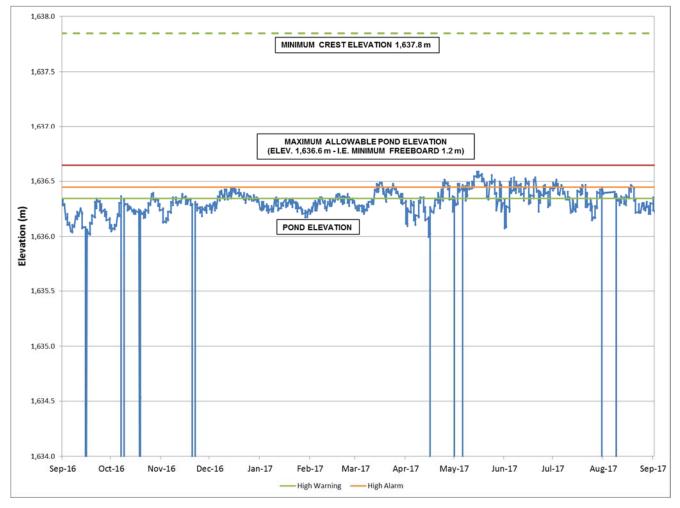


Chart 8: South Tailings Pond Water Elevation from September 2016 to August 2017

Note: Pond elevations reported in Elk Valley Elevation Datum.

Rapid decrease in pond elevation due to plant shut down and not actual pond elevation change

The STP water level is shown to have been maintained below the maximum allowable water elevation from 1 September 2016 to 31 August 2017. In February 2015, a meter was installed at the pond that takes continuous elevation readings which are then fed into the wash plant monitoring system and are thus accessible in real time to anyone with access to that system. The high alarm was triggered several times throughout the reporting period, particularly in November and December 2016, and from March to August 2017. The alarms were investigated by FRO staff and determined that the high readings in winter were due to ice build-up and the increasing water elevation in the spring was due to spring freshet.





Observed Performance

The key observations made during the STP dam inspection were as follows:

- The pond was clear and free of major debris.
- Water from the seepage return wells was being pumped back to the STP. The return line from Kilmarnock Pond was being pumped into the pond at the time of inspection (Appendix A, Photographs 27 and 28).

In an emergency, the following measures could be initiated to control the rise of the water surface in the STP:

- Most inputs to the pond could be turned off. Runoff from the plant site could be redirected to the North Loop Pond. The tailings discharge stream from the plant could be shut down.
- The water reclaim system could be operated to remove water from the pond.
- Water could be pumped from the STP or NLP to Shandley Pit, the NTP, or to the Turnbull TSF, subject to approval of the Ministry of Environment.

The STP is not equipped with an overflow emergency spillway, as the High consequence classification design storm volume can be stored within the facility provided adequate freeboard is maintained. This practice may need to be reassessed as part of the Very High freeboard assessment.

An emergency spillway is considered to be best practice as it allows excess water to exit the facility passively (i.e., without any active intervention). Golder has prepared a conceptual design for an emergency spillway. Spillway location options along with the spillway design criteria and preliminary details are presented in Golder (2012a).

5.4.1.3 Instability

Design Basis

A dam stability update for the STP was completed by Golder (2016e) based on the drilling program conducted by FRO in 2015 (FRO 2016) in accordance with the CDA (2013) guidelines and related technical bulletins (CDA 2007, 2014). The 2,475-year earthquake event was selected (2% exceedance probability in 50 years) for long-term stability analyses under pseudo-static loading conditions as recommended by the CDA (2013), and a peak ground acceleration of 0.158 g was used in the pseudo-static analyses based on the site-specific seismic hazard assessment completed by Golder (2016a). Based on the HSRC, this assessment is out of date since a return period ½ between the 2,475-year and 10,000-year seismic event or the maximum credible earthquake should be used for a facility with a consequence classification of Very High. A draft seismic stability assessment has been completed and is in review with Teck. There are no anticipated changes to stability management based on the Very High consequence seismic stability assessment.





The STP West Dam is also susceptible to instability from erosion during flooding of the Fording River. This has been assessed by KWL, and riprap was placed on the toe of the dam in late 2016 to prevent erosion (KWL 2017c). The south section of the West Dam from the pipe bridge southward does not have any erosion protection but consists partially of bedrock, which provides erosion protection.

Instrumentation Data- Dam Displacement Monitoring

There are 11 operational GPS units on the STP West and Main Dams. Hourly readings from 1 September 2016 to 31 August 2017 were downloaded from GeoExplorer. The initial readings of the GPS units were used for locations of the GPS monitors and are shown in Figure 6.

A summary of the GPS units in use for the 2017 DSI reporting period is presented in Table 21.

GPS Identification	Reading Start Date	Northing (m)	Easting (m)	Location Description
STP-GPS 01	December 2013	5,560,728.9	651,109.0	West Dam – crest
STP-GPS 02	August 2016	5,560,621.6	651,163.7	West Dam – crest above flood construction
STP-GPS 03	April 2016	5,560,537.4	651,186.9	West Dam – flood construction toe
STP-GPS 04	February 2017	5,560,540.1	651,239.9	West Dam – crest above flood construction
STP-GPS 05	October 2014	5,560,441.9	651,355.6	West Dam – crest above flood construction
STP-GPS 06	April 2016	5,560,349.1	651,369.2	West Dam – flood construction toe
STP-GPS 07	December 2013	5,560,259.9	651,525.9	West Dam – crest
STP-GPS 08	August 2015	5,560,152.6	651,659.4	West Dam – crest
STP-GPS 09	April 2016	5,560,081.3	651,844.4	Main Dam – crest
STP-GPS 10	April 2016	5,560,022.7	652,029.4	Main Dam – toe
STP-GPS 11	April 2016	5,560,089.4	652,051.2	Main Dam – crest

Table 21: GPS Monitoring Locations on South Tailings Pond

Note: Northings and Eastings reported in UTM.

Table 22 summarizes the GPS units at STP, whether they have been relocated at any point in time, and any other comments provided by FRO. No GPS units were reset during the reporting period.





GPS Monitor ID	Colour in Chart 9	Relocated	Comments				
STP-GPS 01		No	21 April 2017 erroneous data 17 May 2017 erroneous data, deleted				
STP-GPS 02		No	No activity				
STP-GPS 03		No	No activity				
STP-GPS 04		No	First readings, replaced STP349				
STP349		n/a	Readings discontinued, replaced by STP-GPS 04				
STP-GPS 05		No	No activity				
STP-GPS 06		Yes	 16 September 2016 GPS was moved for rip rap construction 9 November 2016 GPS was moved after rip rap construction 7 February 2017 dead battery 5 March 2017 erroneous data observed 10 April 2017 erroneous data observed 1 April 2017 erroneous data observed, deleted 				
STP-GPS 07		No	16 April 2017 erroneous data, deleted 5 May 2017 erroneous data, deleted				
STP-GPS 08		No	23 May 2017 erroneous data, deleted				
STP-GPS 09		No	23 April 2017 erroneous data, deleted				
STP-GPS 10		No	 11 November 2016 erroneous data observed 14 November 2016 erroneous data observed 13 March 2017 erroneous data observed 14 April 2017 erroneous data observed, deleted 18 April 2017 erroneous data observed, deleted 14 June 2017 erroneous data observed, deleted 3 July 2017 erroneous data observed, deleted 				
STP-GPS 11		No	No activity				

Table 22: South Tailings Pond GPS Unit Activities (Based on FRO Records)

Note: GPS considered "relocated" when unit moved more than 1 m in any direction.

n/a = not applicable.

The 3D displacement for each GPS monitor were downloaded from GeoExplorer and are presented in Chart 9. The cumulative horizontal displacement, cumulative vertical displacement, cumulative relative displacement, and 3D point velocity for the GPS monitors were downloaded from GeoExplorer and are presented in Appendix G.





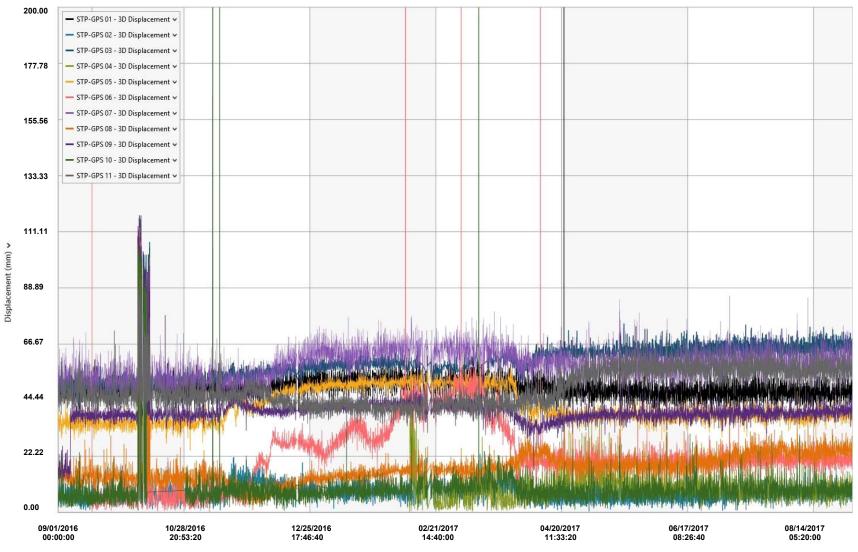


Chart 9: South Tailings Pond GPS Monitors 3D Displacement





The GPS units record on an hourly frequency. The survey data indicate little crest displacement during the reporting period. Spikes in the data, particularly in October 2016, are most likely noise in the system and are not a concern. The overall trend of the data is consistent with little increase over the reporting period. Movements are below the GeoExplorer trigger for 3D displacement (100 mm, updated QPO warning).

Any offline monitors will be inspected and repaired within one week (FRO 2015b).

It is noted that STP-GPS 06 was relocated due to rip rap construction. As best practice, relocating the monitoring points should be avoided as it does not allow long-term trend monitoring of displacement.

Instrumentation Data – Slope Inclinometers

Slope inclinometers were installed at four locations in 2015 along the STP crest (Figure 6) to monitor horizontal movement in the dam in addition to the GPS data. The A axis is oriented in the upstream to downstream direction (with negative displacements in the downstream direction) and the B axis is oriented along the dam centreline. The location of the inclinometers on the STP is presented in Table 23.

Inclinometer ID	Northing (m)	Easting (m)	Elevation (m)	A-A Axis Azimuth (°)	Probe Serial No.	Reel Serial No.
TH15-01	5,560,086.2	652,037.3	1,638.2	310		
TH15-02	5,560,093.0	651,786.4	1,638.3	10		DR21300000
TH15-03	5,560,550.6	651,227.5	1,638.7	30	DP15600000	
TH15-04	5,559,997.8	652,003.4	1,604.6	15		

Table 23: South Tailings Pond Inclinometers

Note: Azimuth is approximate. The upper wheel should face the indicated direction for the first set of readings.

Northings and Eastings reported in UTM and elevations reported in Elk Valley Elevation Datum.

TH = test hole.

Inclinometer data were supplied to Golder by FRO. Readings have been taken approximately quarterly at the STP inclinometers since December 2015. A total of three readings were taken at inclinometers TH15-02, TH15-03, and TH15-04 between August 2016 and August 2017. A total of four readings were taken at inclinometer TH15-01 between August 2016 and August 2017. Results from 5 March 2017 showed very large deflections in TH15-01; therefore, FRO took a second set of readings on this date. The second set of results trends within the expected range.

Once the inclinometer data were received, they were interpreted by Golder (Appendix H). Data readings are from 10 January 2017 to 6 July 2017, including the initial reading from 18 December 2015 as a reference line.

The maximum deflection observed in direction A does not exceed approximately 4 mm for inclinometer TH15-01. Direction B appears to deflect approximately 4 to 3 mm to a depth of about 20 m below the top of inclinometer casing (Appendix H, Figure H-4).





The maximum deflection observed in direction A does not exceed approximately 4 mm for inclinometer TH15-02. Direction B appears to deflect approximately 5 mm from an elevation of about 1,615 to 1,605 m (Appendix H, Figure H-5).

The maximum deflection observed in direction A does not exceed approximately 4 mm for inclinometer TH15-03. Direction B appears to deflect approximately 5 mm to a depth of about 14 m below the top of inclinometer casing (Appendix H, Figure H-6).

TH15-04 shows a deflection of approximately 4 mm at the top of the casing, which is likely due to movement of the casing exposed above the dam crest (Appendix H, Figure H-7).

The inclinometer readings do not indicated any significant trends in deformation.

Instrumentation Data – Piezometers on Main Dam

A summary of the VW piezometer locations and sensor depths on the Main Dam is shown in Table 24.

Piezometer ID	Northing (m)	Easting (m)	Top of Well Elevation (m)	Data Logger Serial	Piezometer Serial No.	Piezometer Elevation (m)
VW-4	5 560 100 6	651 759 7	1,639.2	DT08079	VW27921	1,617.2
V VV-4	5,560,100.6	651,758.7	1,039.2	DT08082	VW27920	1,615.0
VW-5	5,560,106.2	652,102.4	1,639.2	DT08073	VW27929	1,615.5
v vv-5				DT08075	VW27930	1,610.4
	5,560,086.2	652,037.3	1,638.2	DT04498	VW33227	1,611.1
TH15-01					VW33229	1,604.8
					VW33244	1,600.9
					VW33238	1,612.2
TH15-02	5,560,093.0	651,786.4	1,638.3	DT04499	VW33233	1,605.5
					VW33243	1,601.6
TH15-04	5,559,997.8	652,003.4	1,604.6	DT09637	VW33224	1,599.6
SP-3	5,560,032.4	652,043.8	1,610.4	DT08083	VW27931	1,600.6
SP-5	5,560,057.5	652,163.7	1,605.0	DT08074	VW27918	1,595.9

 Table 24: South Tailings Pond Main Dam Piezometer Installation Details

Note: Northings and Eastings reported in UTM and elevations reported in Elk Valley Elevation Datum.

TH = test hole; VW = vibrating wire; SP = retrofitted standpipe.

The VW piezometers and standpipes locations are shown in plan in Figure 6. Chart 10 presents the piezometer readings for 1 September 2016 to 31 August 2017, as well as the pond elevation over the same time period. Data for the piezometers was provided by FRO and supplemented with data downloaded from GeoExplorer to fill in any gaps as necessary.





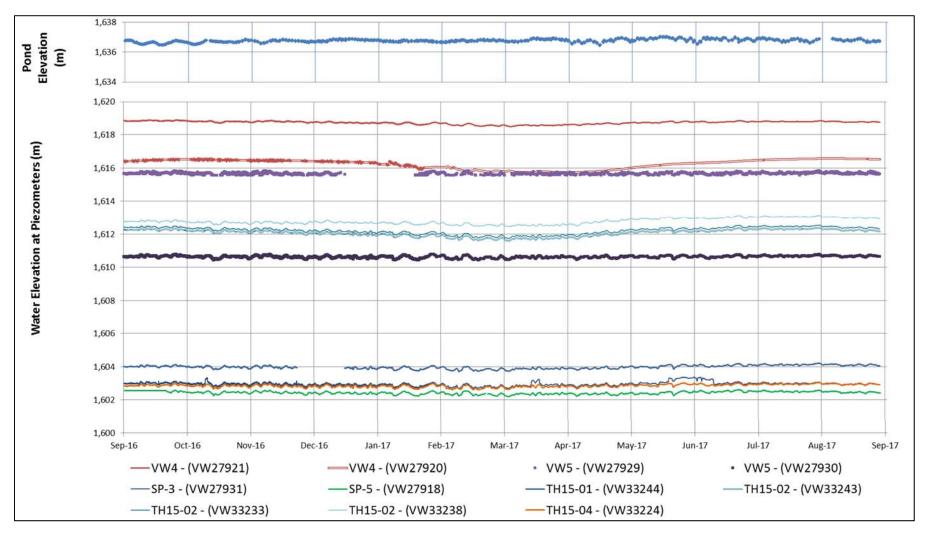


Chart 10: Main Dam Vibrating Wire Piezometer and Standpipe Water Elevations and South Tailings Pond Elevation

Note: Elevations reported in Elk Valley Elevation Datum.





The Main Dam piezometers' performance was assessed and is summarized in Table 25.

Piezometer ID	Piezometer Serial No.	GeoExplorer Sensor No.	Piezometer Elevation (m)	Minimum (2016/2017)	Maximum (2016/2017)	Upward Gradient	Exceed Alarm	Comments
	VW27921	2	1,617.2	1,618.3	1,618.7	n/a	No	No concerns.
VW-4	VW27920	1	1,615.0	1,615.6	1,616.5	No	No	Trending upwards from April to June 2017.
VW-5	VW27929	2	1,615.5	1,615.5	1,615.9	No	No	Missing data from December 2016 to January 2017.
	VW27930	1	1,610.4	1,610.4	1,610.9	n/a	No	No concerns.
	VW33227	1	1,611.1	n/a	n/a	n/a	No	Negative water level (dry).
	VW33229	2	1,604.9	n/a	n/a	n/a	No	Negative water level (dry). Missing data from November to December 2016 due to dead battery (subsequently replaced).
	VW33244	3	1,600.9	1,603.7	1,604.2	n/a	No	Missing data from November to December 2016.
	VW33238	3	1,612.2	1,612.9	1,613.6	n/a	No	Trending upwards from April to June 2017.
TH15-02	VW33233	2	1,605.5	1,611.7	1,612.5	No	No	Trending upwards from April to June 2017.
VW33243		1	1,601.5	1,611.5	1,612.3	No	No	Trending upwards from April to June 2017.
TH15-04	VW33224	-	1,599.5	1,602.5	1,603.0	n/a	No	No concerns.
SP-3	VW27931	-	1,600.6	1,602.8	1,603.5	n/a	No	Small increase of 0.2 m in water level from May to June 2017.
SP-5	VW27918	-	1,595.9	1,602.1	1,602.6	n/a	No	No concerns.

Table 25: South Tailings Pond Main Dam Piezometer Performance Summary

Note: Elevations reported in Elk Valley Elevation Datum.

Alarms from GeoExplorer.

TH = test hole; VW = vibrating wire; SP = retrofitted standpipe; n/a = not applicable.





The phreatic level readings for the time period were generally stable very little to no reaction to spring freshet. No warnings were triggered in GeoExplorer for these piezometers.

VW-4 (VW27920) and TH15-02 sensors all show slight increases in water elevation from April to about June 2017. This trend may be attributed to spring freshet. The trend does not seem to persist, and the piezometer water elevations seem to flatten after June.

At multiple times over the reporting period missing data was reported. GeoExplorer has been updated to show "No Communication" and "No Frequency" alarms that alert FRO when the piezometers are not reading data. FRO will use these alarms as an indication that the piezometers are malfunctioning and will send someone to check on the instrument in question.

Instrumentation Data – Piezometers on West Dam

A summary of the VW piezometer locations and sensor depths on the West Dam is shown in Table 26.

No data were available from GeoExplorer for VW-3 (VW27917); therefore, it is not reported on. FRO personnel noted that the output from the cable is very low and the piezometer is likely not functional.

Piezometer ID	Northing (m)	Easting (m)	Elevation (m)	Data Logger Serial	Piezometer Serial No.	Piezometer Elevation (m)
VW-1	5,560,710.9	651,118.1	1,640.0	DT08070	VW27922	1,620.4
V VV- I				DT08078	VW27923	1,606.4
VW-2	5,560,494.1	651,310.0	1,639.3	DT08076	VW27926	1,616.9
				DT08077	VW27928	1,610.5
VW-3	5,560,278.9	651,509.5	1,638.9	DT08071	VW27925	1,622.3
				DT08072	VW27924	1,611.4
TH15-03	5,560,550.6	651,227.5	1,638.7	DT04500	VW33225	1,618.2
					VW33228	1,614.2
					VW33226	1,612.1
SP-W1	5,560,273.7	651,497.3	1,633.9	DT08081	VW27927	1,613.4
SP-W3	5,560,255.0	651,481.4	1,624.5	DT08080	VW27919	1,615.0

Table 26: South Tailings Pond West Dam Piezometers Installation Details

Note: Northings and Eastings reported in UTM and elevations reported in Elk Valley Elevation Datum.

TH = test hole; VW = vibrating wire; SP = retrofitted standpipe.

The VW piezometers and standpipes are presented in plan in Figure 6. Chart 11 presents the piezometer readings for 1 September 2016 to 31 August 2017, as well as the pond elevation over the same time period. Data for the piezometers was provided by FRO personnel and supplemented with data downloaded from GeoExplorer to fill in any gaps as necessary.





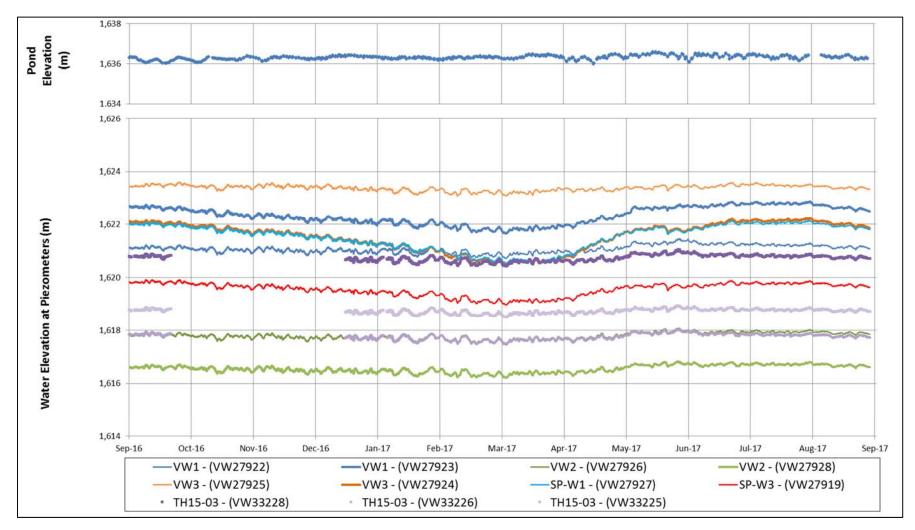


Chart 11: West Dam Vibrating Wire Piezometer and Standpipe Water Elevations and South Tailings Pond Elevation

Note: Elevations reported in Elk Valley Elevation Datum.





The West Dam piezometers' performance was assessed and is summarized in Table 27.

Piezometer ID	Piezometer Serial No.	Piezometer Elevation (m)	Minimum (2016/2017)	Maximum (2016/2017)	Upward Gradient	Exceed Alarm	Comments
	VW27922	1,620.4	1,620.7	1,621.5	n/a	No	No concerns.
VW-1	VW27923	1,606.4	1,621.6	1,622.9	Yes	No	Larger freshet trend as compared to other piezometers, still within range of expected water elevations for piezometer.
VW-2	VW27926	1,616.9	1,617.6	1,618.2	n/a	No	No concerns.
V VV-Z	VW27928	1,610.5	1,616.2	1,616.8	No	No	No concerns.
	VW27925	1,622.3	1,623.2	1,623.7	n/a	No	No concerns.
VW-3	VW27924	1,611.4	1,620.5	1,622.3	No	No	Larger freshet trend as compared to other piezometers, still within range of expected water elevations for piezometer
	VW33225	1,618.2	1,618.5	1,618.9	n/a	No	Missing data from September to December 2016, except for a few sporadic readings. TH15-03 data logger damaged by water and needs to be replaced.
TH15-03	VW33228	1,614.2	1,620.4	1,621.0	Yes	No	Missing data from September to December 2016 except for a few sporadic readings. TH15-03 data logger damaged by water and needs to be replaced.
	VW33226	1,612.2	1,617.5	1,618.1	No	No	Missing data from September to December 2016 except for a few sporadic readings. TH15-03 data logger damaged by water and needs to be replaced.
SP-W1	VW27927	1,613.4	1,620.5	1,622.1	n/a	No	Larger freshet trend as compared to other piezometers, still within range of expected water elevations for piezometer.
SP-W3	VW27919	1,615.0	1,618.9	1,619.9	n/a	No	Larger freshet trend as compared to other piezometers, still within range of expected water elevations for piezometer

Table 27: South Tailings Pond West Dam Piezometer Performance	e Summary
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Note: Elevations reported in Elk Valley Elevation Datum.

Alarms from GeoExplorer

TH = test hole; VW = vibrating wire; SP = retrofitted standpipe; n/a = not applicable.





The phreatic level readings for the time period were generally stable with trends related to spring freshet seen in most sensors in April and May 2017. No warnings were triggered in GeoExplorer for these piezometers.

VW-1 (VW27923), VW-3 (VW27924), SP-W1 (VW27927), and SP-W3 (VW27919) all show stronger trends related to spring freshet as compared to the other piezometers in the West Dam. VW-1 (VW27923) and VW-3 (VW27924) are both installed in bedrock so the trend could be related to the water table within the unit. SP-W1 (VW27927) and SP-W3 (VW27919) installation geologic units are unknown. After the increase in water elevation in the piezometers during spring freshet, the water elevations tend to stay higher for these piezometers but it appears the water elevations trend down due to freezing conditions from October onwards. These trends are considered consistent with previous years.

Upward gradients are noted in TH15-03 and VW-1. In TH15-03, both piezometers are within the foundation fluvial sand and gravel unit. In VW-1 the lower piezometer is in bedrock and the upper piezometer is in the fluvial sand and gravel. The fluvial unit is not confined in both borehole location, and the piezometer readings are likely influenced by groundwater flow through the previous river channel.

Teck note that the data logger for TH15-03 has been damaged by water and a replacement is required.

At multiple times over the reporting period missing data was reported. GeoExplorer has been updated to show "No Communication" and "No Frequency" alarms that alert FRO when the piezometers are not reading data. FRO will use these alarms as an indication that the piezometers are malfunctioning and will send someone to check on the instrument in question.

Observed Performance

The key observations made during the STP dam inspection related to assessment of instability were as follows:

- No significant evidence of slope instability on the constructed dam (i.e., significant sloughing, cracking, crest subsidence) was observed during the 2017 DSI.
- The upstream slope was regraded and riprap protection was installed on the upstream side of the Main Dam (Appendix A, Photographs 29 and 36). It was noted that tailings were pushed into the pond to place the riprap and there is a buildup of water between the tailings and riprap. The tailings should be regraded with an excavator so that water will preferentially flow into the pond.
- Minor erosion has been noted on the downstream slope over the years, generally in the CCFR material. FRO has repaired previous erosion channels present on the STP by placing breaker rock over geotextile on the eroded areas, creating armoured channels. Current and future erosion should continue to be repaired in a similar or equivalent manner as part of ongoing maintenance.
- The downstream slope has sections steeper than design, but the overall embankment has been constructed wider than the design. The over-steepened areas are prone to increased erosion, but are not an overall stability concern.

5.4.1.4 River Erosion Protection (KWL)

KWL completed an inspection of the riprap at the toe of STP in 2017 and the inspection report is provided in Appendix I.



5.4.2 Review of Previous Deficiencies and Non-conformances

The following deficiencies and non-conformances for the STP were raised in the previous DSI (Golder 2017b). Table 28 provides the current status of the 2016 DSI recommendations for the STP. Items from the 2016 DSI that are incomplete have been brought forward into the 2017 DSI recommendations (Table 29).

There are a number of recommendations that are in progress and some that are incomplete, however Golder feels the work is being appropriately prioritized based on good communication between the EoR team and the FRO engineer teams.





ID Number	Deficiency or Non-conformance	Recommended Action	Г
2013-16	No passive emergency system against overtopping; emergency system requires active response	Assess the best combination of active and passive emergency systems during various stages of the pond life cycle. If the assessment determines that passive systems are warranted, then develop a construction schedule for the selected system(s).	Inc act
2015-11	Over-steepened relative to design and susceptible to erosion from wave action	Regrade upstream slope to design (1.75H:1V) and place riprap on regraded slope.	Co
		Perform risk-informed assessment to determine appropriate flood protection requirements for downstream toe of dam along Fording River and timeline to implement.	i In Fo
2015-12a,b,c	Riprap erosion protection along downstream toe north of STP Stn. 0+680, no riprap south of STP Stn. 0+680; risk-informed protection requirements not yet defined	Implement required protection measures for the operational phase according to the as-defined schedule.	Inc act
		Execute flood risk mitigation plan until flood protection requirements defined by the risk-informed assessment are in place.	Or
2016-08	Ditch adjacent road on West Dam bench filled in by haul truck traffic	Reinstate ditch to functional condition.	Co
2016-12	Hydroseeding in the repaired sections of the STP downstream slope did not root. No records available	Hydroseeding should be incorporated into the tailing management system and records of hydroseeding, including the success rate, should be kept.	Inc up
2016-13	The GoldSIM water balance model is not accurately accounting for the change in available volume in the STP due to dredging (increase in available volume) and tailings depositions (decrease in available volume)	The change in available volume be included in future water balances to improve the STP water balance.	Co
2015-03	Roles of Geotechnical and Hydraulics Engineers of Record undocumented	Golder, FRO, and KWL to document the roles of the Engineer of Record for the geotechnical and hydraulics related works in the OMS manual.	In for
		Complete updated seismic stability assessment and liquefaction based on revised design criteria.	In me up
2016-01a,b	Seismic design criteria for stability out of date due to dam reclassification from High to Very High	Update QPOs based on revised stability assessment.	In & p pe rec
2016-02	IDF and freeboard out of date due to dam reclassification from High to Very High	Update the IDF and freeboard assessment for the NTP and STP.	ln up
2016-03	OMS manual requires updating	 Update OMS manual as follows: Update all references to consequence classification of structures—change from High to Very High. Include design criteria. Review the manual using the updated HSRC and Guidance Document (MEM 2017, 2016). QPOs to be included for surveillance. The dredging section needs to be updated to identify that dredging is currently operating to the Turnbull Tailings Storage Facility. Include safe work plans. Include incident reporting procedures. Complete minor updates identified in the 2015 DSI report (Golder 2016b). 	In coi rec

Updated Status as of March 2018

Incomplete – see Table 29 for updated recommended action and deadline

Complete

In **Progress** – completed drilling program for widening of Fording River channel

Incomplete – see Table 29 for updated recommended action and deadline

Ongoing

Complete

Incomplete – recommendation moved to OMS manual updates (2016-03)

Complete

In Progress – to be included in OMS manual; see Table 29 for updated recommended action and deadline

In Progress – draft assessment complete, seismic stability meets or exceeds new design criteria; see Table 29 for updated recommended action and deadline

In **Progress** – pending completion by Golder, updated GPS & prism QPOs in this DSI report; updated piezometer QPOs pending completion by Golder; see Table 29 for updated recommended action and deadline

In Progress – draft assessment complete; see Table 29 for updated recommended action and deadline

In **Progress** – under review by FRO, QPO update pending completion by Golder; see Table 29 for updated recommended action and deadline





ID Number	Deficiency or Non-conformance	Recommended Action	
2016-04	EPP & ERP require updating	Reference to the QPOs needs to be included for actions required based on instrumentation warnings and alarms.	In I for
2016-09	No QPOs set for inclinometers	QPOs and frequency of readings should be set for the inclinometers.	In rec
2016-10	Warning level QPO for piezometers exceeded. Based on review of data, this is not a failure concern	Update warning level QPOs for piezometers based on review of data all available data (2014 to present).	In rec

IDF = inflow design flood; FRO = Fording River Operations; KWL = Kerr Wood Leidal Associates Ltd.; NTP = North Tailings Pond; STP = South Tailings Pond; HSRC = Health, Safety and Reclamation Code; DSI = dam safety inspection; TSF = tailings storage facility; OMS = operation, maintenance and surveillance; EPP = Emergency Preparedness Plan; ERP = Emergency Response Plan; QPO = quantitative performance objectives.

Updated Status as of March 2018

n Progress – pending completion by FRO; see Table 29 or updated recommended action and deadline

n Progress – draft complete; see Table 29 for updated ecommended action and deadline

n Progress – pending completion by Golder, ecommendation incorporated in 2017-06





6.0 SUMMARY AND RECOMMENDATIONS

6.1 Summary of Activities

Activities completed for the NTP during the reporting period were:

- Monthly inspections by FRO geotechnical personnel
- Quarterly water quality testing
- Completion of Liverpool Sediment Pond system
- Prefeasibility design of NTP revision from tailings dam to mine waste facility or "landform"
- Completion of riprap upgrade works to Q200 design flow plus 0.5 m
- Completion of a probable maximum flood assessment for the Fording River watershed

Activities completed for the STP during the reporting period were:

- Two bathymetric surveys
- Monthly inspections by FRO geotechnical personnel
- Quarterly water quality testing
- Dredging of an estimated 850,076 dry metric tonnes of tailings to the Turnbull TSF
- Potential tailings water discharge event and related response including:
 - Inspection by Golder personnel
 - Construction of a till berm to exclude pond water near north abutment
 - Extended tailings line further into the pond
 - Filled in low area around previous end of tailings line
- Cleaned out ditches, regraded, and repaired berms along the lower access road of the West Dam
- Placed riprap along the upstream slope between Sta. 0+700 and 1+700
- Replaced the riprap emergency stockpile
- Completion of a probable maximum flood assessment for the Fording River watershed (same report as noted for NTP)





6.2 Summary of Climate and Water Balance

The climate data indicates the annual precipitation received at FRO from 1 September 2016 to 31 August 2017 was average as compared to the long-term historical average.

The NTP has a negative to neutral water balance. The pond was generally the same size throughout the year and no pumping was necessary.

The STP has a positive to neutral water balance. The pond capacity is reduced due to tailings deposition. The water levels are closely monitored and managed since the levels are near, but below, the maximum freeboard level.

6.3 Summary of Performance and Changes

The STP and NTP facilities were observed to be in good condition at the time of the 2017 DSI field inspection. No significant changes in instrumentation and visual monitoring records, dam stability, and surface water control were noted.

6.4 **Consequence Classification**

Both of the dams are classified as Very High consequence, following the dam consequence classification guidelines from HSRC Guidance Document Section 3.4 (MEM 2016).

An incremental inundation assessment was completed to assess the consequence of failure of the NTP and STP during a major flood event of the Fording River (Golder 2017g). The assessment concluded that the consequence of a failure occurring coincident with a major river flood event was High.

6.5 Current Deficiencies and Non-conformances

Table 29 summarizes the recommended actions for both the STP and NTP facilities.





Table 29: 2017 Dam Safety Inspection Recommended Actions for the North and South Tailings Pond Facilities

Structure	ID Number	Deficiency or Non-conformance	Applicable Guideline or OMS Reference	Recommended Action	Priority Level	Recommended Timing for the Action
	2015 05a b	No passive emergency system against	2/2	Assess the need for spillway after finalizing the closure plan NTP.	4	Q3 2018
	2015-05a,b	overtopping; emergency system requires active response	n/a	If required, determine a construction schedule.	4	Q4 2018
				Perform risk-informed assessment to determine appropriate flood protection requirements for downstream toe of dam along the Fording River and the timeline to implement.	2	Q3 2018
	2015-06a,b,c	Risk-informed criteria for flood erosion protection along toe of dams not defined	CDA 2013 §6.2	Implement required protection measures for the operational phase according to the as- defined schedule.	2	2019
				Execute the flood risk mitigation plan until the flood protection requirements defined by the risk-informed assessment are in place.	1	Ongoing
NTP	2015-07a,b,c	Buried pipes passing through crest locations	n/a	Inspect steel pipes as part of regular dam inspections until NTP closure plans are finalized. Include inspections in OMS manual update.	3	Q2 2018 (tentatively complete since December 2017, pending final OMS revision)
				Execute abandonment plan for PVC pipes.	3	Q4 2018
	2016-05a	North abutment excavated without input or approvals from Engineer of Record or Qualified Person Assess and revise required internal and external or activities carried out near the site TSFs		Assess and revise required internal and external communication for work and construction activities carried out near the site TSFs	1	Q2 2018 (tentatively complete since March 2018, pending final OMS revision)
	2016-06	No closure plan for NTP	n/a	Develop closure plan for NTP based on results of feasibility investigation into NTP decommissioning.	4	Q4 2018
	2013-16	No passive emergency system against overtopping; emergency system requires active response	n/a	Assess the best combination of active and passive emergency systems during various stages of the pond life cycle. If the assessment determines that passive systems are warranted, then develop a construction schedule for the selected system(s).	4	Q4 2018
STP	2015-12a,b,c	Riprap erosion protection along downstream	HSRC §10.1.8	Perform risk-informed assessment to determine appropriate flood protection requirements for downstream toe of dam along Fording River and timeline to implement.	2	Q3 2018
		toe north of STP Stn. 0+680, no riprap south of STP Stn. 0+680; risk-informed protection		Implement required protection measures for the operational phase according to the as- defined schedule.	2	2019 or as determined by schedule
		requirements not yet defined		Execute flood risk mitigation plan until flood protection requirements defined by the risk- informed assessment are in place.	1	Ongoing
OTD	2017-01	North abutment construction deficiencies	HSRC §10.5.1(3)	Address construction deficiency, finish dam construction.	2	2019
STP	2017-02	Tailings that were excavated along upstream slope to place the riprap zone impede water flow towards main pond	n/a	The tailings should be regraded with an excavator so that water will preferentially flow into the pond.	4	Q3 2018
	2017-03	Inspection frequency inadequate for active, Very High consequence facility	HSRC §10.1.12	Increase geotechnical inspections to weekly from April to October and twice per month from November to March for STP	3	Q2 2018 (in progress, pending final OMS revision)
	2017-04	Planned dredging of Tailings to Turnbull TSF is behind schedule and the result is a very high level of tailings in STP which is causing operational issues (e.g., disposition line backing up and reclaimed process water with too much sediment)	n/a	Dredging to Turnbull TSF should be started as soon as possible with an increased annual dredging target.	2	Q2 2018
	2015-03	Roles of Geotechnical and Hydraulics Engineers of Record undocumented	HSRC §10.1.5	Golder, FRO, and KWL to document the roles of the Engineer of Record for the geotechnical and hydraulics related works in the OMS manual.	4	Q2 2018
NTP/STP	2016-01	Seismic design criteria for stability out of date due to dam reclassification from High to Very High	HSRC §10.1.4 & 8	Complete updated seismic stability assessment and liquefaction based on revised design criteria. Check effects of upward gradient noted in STP piezometers.	2	Q2 2018 (draft in review)
	2016-02	IDF and freeboard out of date due to dam reclassification from High to Very High	HSRC §10.1.4 & 8	Update the IDF and freeboard assessment for the NTP and STP.	2	Q2 2018 (draft in review)





ID Number	Deficiency or Non-conformance	Applicable Guideline or OMS Reference	Recommended Action	Priority Level	Recommended Timing for the Act
2016-03	OMS manual requires updating	HSRC §10.5.2(4)	 Update OMS manual as follows: Update all references to consequence classification of structures—change from High to Very High. Include design criteria. Review the manual using the updated HSRC and Guidance Document (MEM 2017, 2016). Review the manual using most recent MAC guidelines QPOs to be included for surveillance. The dredging section needs to be updated to identify that dredging is currently operating to the Turnbull Tailings Storage Facility. Include safe work plans. Include incident reporting procedures. Include animal burrow inspection and procedures Include NTP pipe inspections Include hydroseeding records Include Liverpool and NTP boundaries 	4	Q2 2018 (draft in progress)
2016-04	EPP & ERP require updating	HSRC §10.4.2(1)	Reference to the QPOs needs to be included for actions required based on instrumentation warnings and alarms.	4	Q3 2018
2016-09	No QPOs set for inclinometers	HSRC §10.1.13	QPOs and frequency of readings should be set for the inclinometers.	3	Q2 2018 (draft in this document)
2017-05	Potential overtopping hazard due to tailings liquefaction and redistribution during seismic event needs to be assessed	n/a	Complete liquefaction and overtopping assessment for tailings within facility.	2	Q4 2018
2017-06 (supersedes 2016-01b and 2016-10)	Trigger-action-response plans (TARPs) and related QPOs not strongly tied to risk assessment results	HSRC §10.5.2	TARPs with related monitoring plans and QPOs should be reviewed with consideration of the results from the 2017 TSF risk assessment	3	Q3 2018

Priority Level	Description
1	A high probability or actual dam safety issue considered immediately dangerous to life, health or the environment, or a significant risk of regulatory enforcement.
2	If not corrected could likely result in dam safety issues leading to injury, environmental impact or significant regulatory enforcement; or, a repetitive deficiency that demonstrates a
3	Single occurrences of deficiencies or non-conformances that alone would not be expected to result in dam safety issues.
4	Best Management Practice – Further improvements are necessary to meet industry best practices or reduce potential risks.

Source: HSRC Guidance Document, Section 4.2 (MEM 2016).

s a systematic breakdown of procedures.





7.0 OPPORTUNITIES FOR IMPROVEMENT

Some opportunities for improvement were identified during the DSI:

- The downstream slopes of the STP West Dam could be revegetated to reduce erosion and reduce the required maintenance.
- The STP West and Main dams could be resloped to remove the bench. This could reduce the amount of erosion on the dam and could increase the overall factor of safety against instability.

8.0 CLOSURE

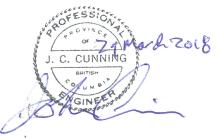
The reader is referred to the Study Limitations, which follows the text and forms an integral part of this report.

We trust the above meets your present requirements. If you have any questions or further requirements, please contact the undersigned.

GOLDER ASSOCIATES LTD.

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STUDY LIMITATIONS

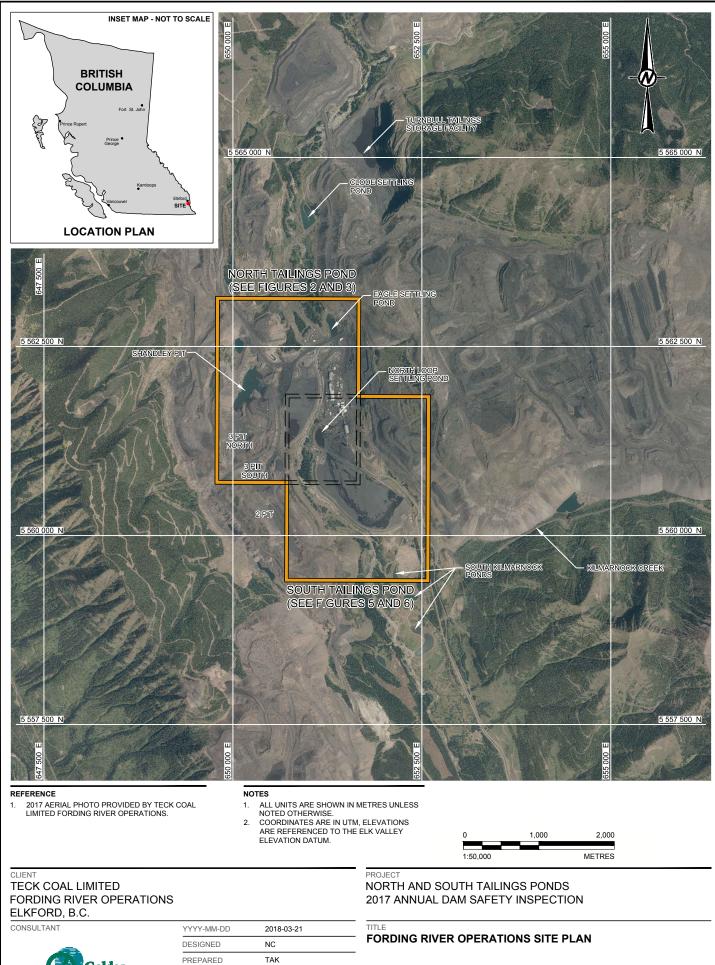
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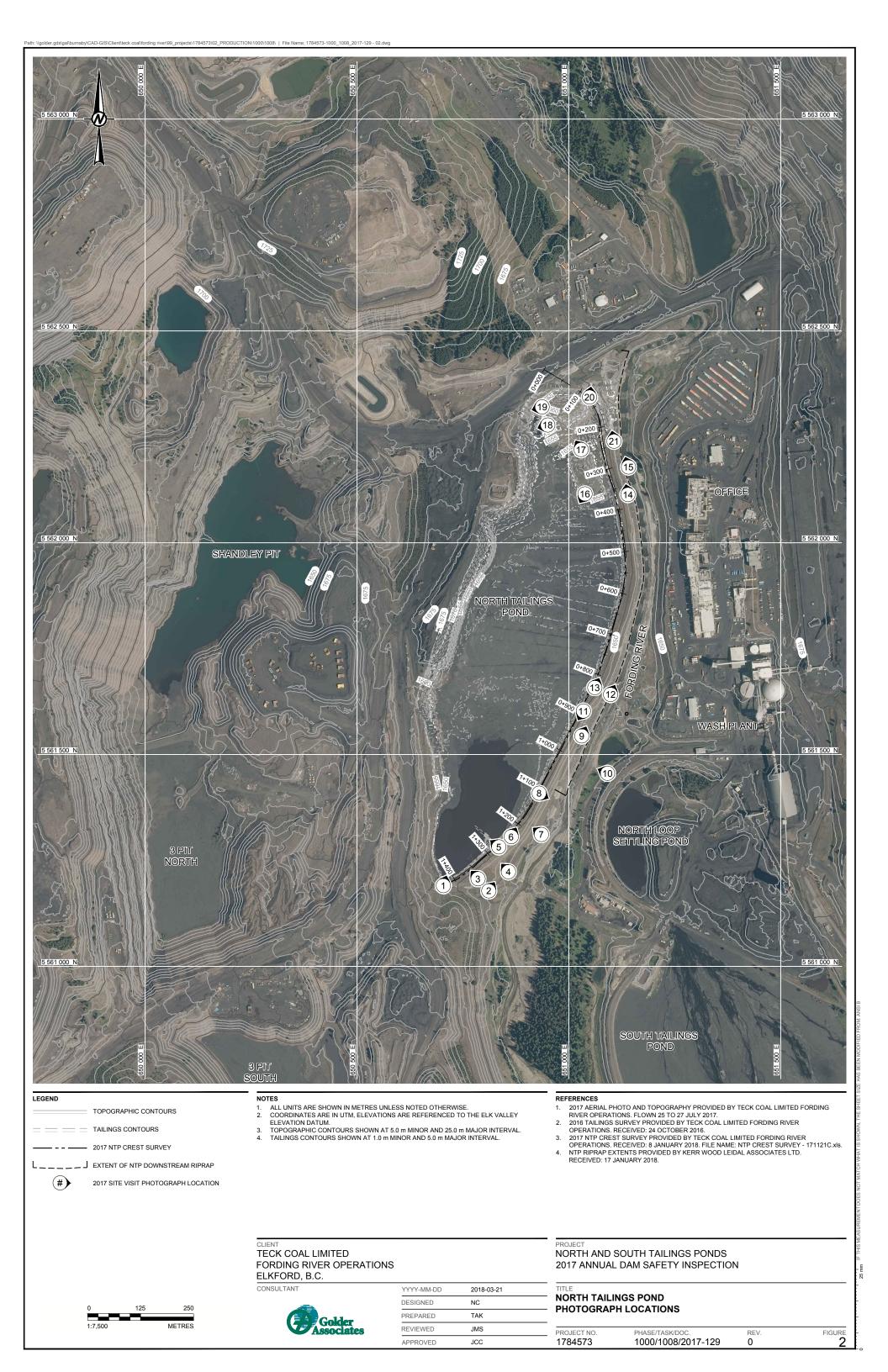
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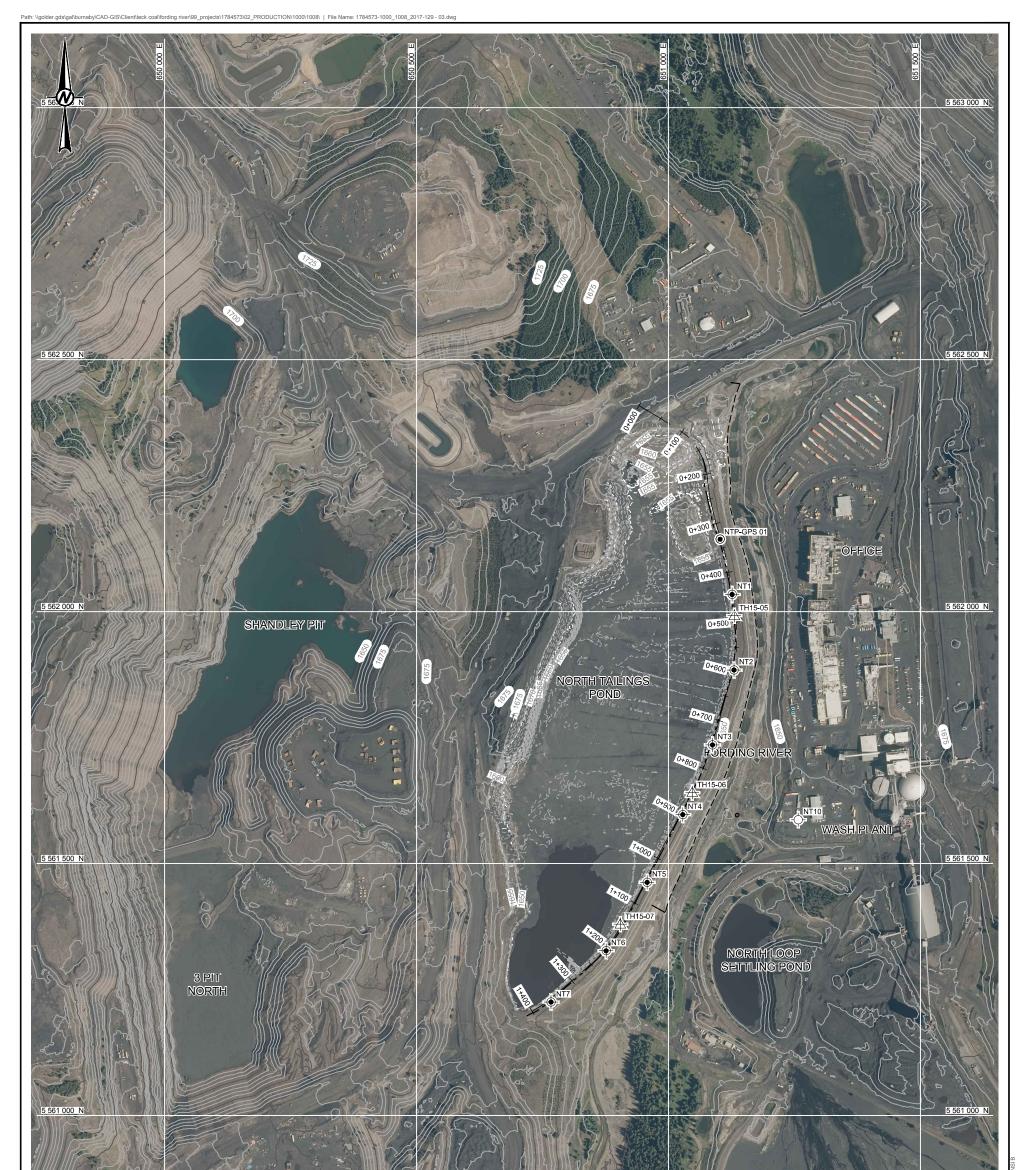
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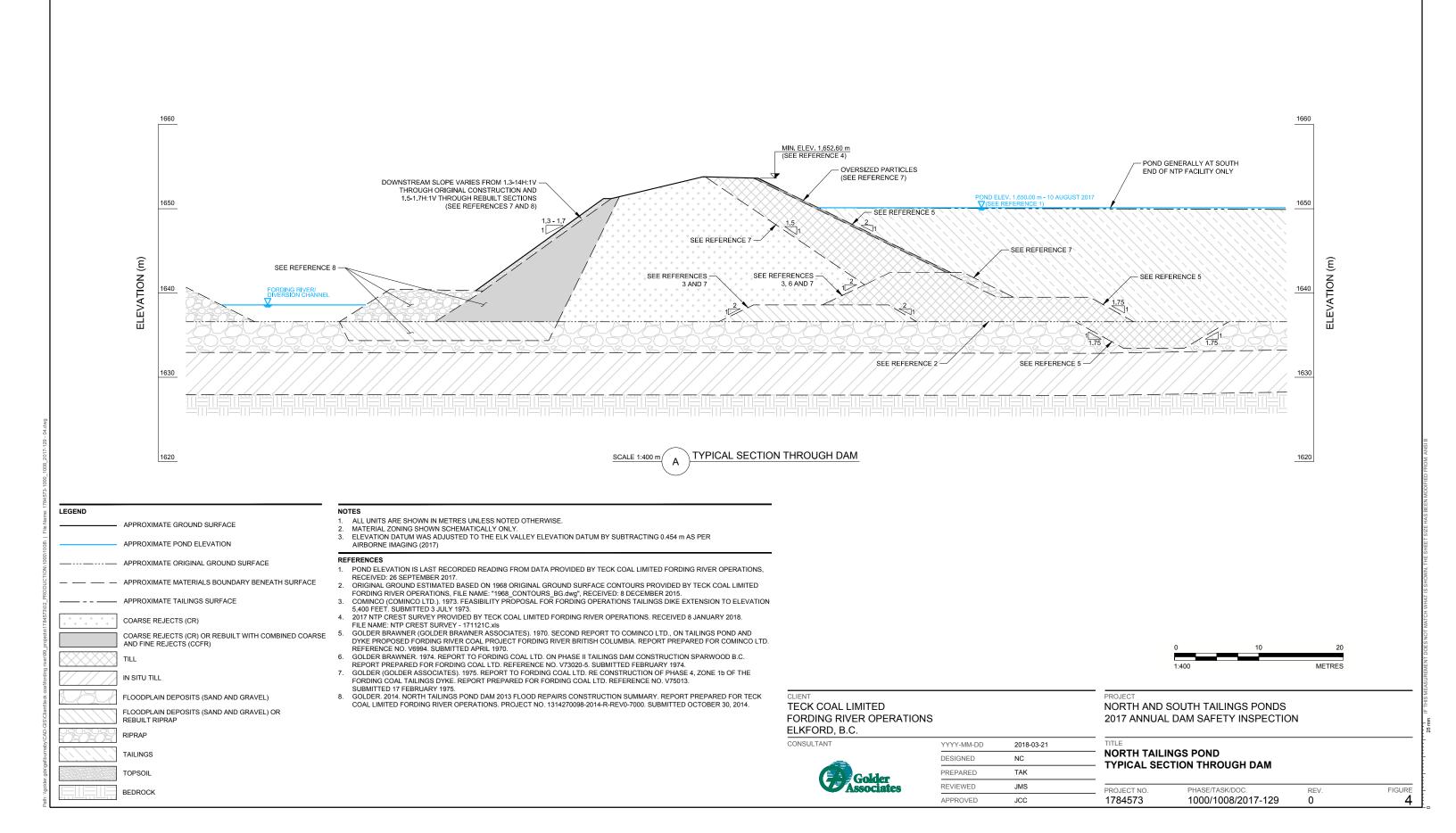
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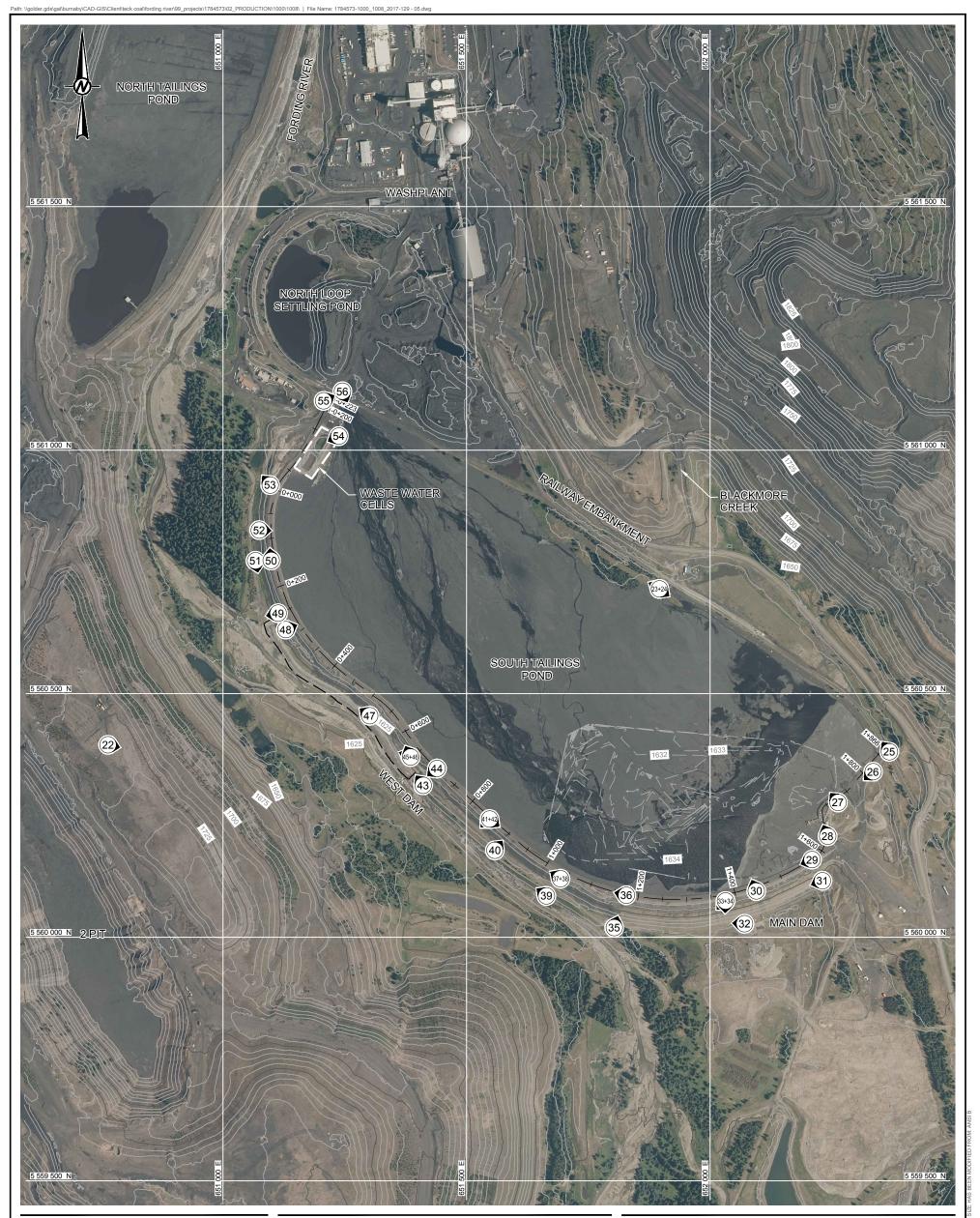
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- TOPOGRAPHIC CONTOURS
- TAILINGS CONTOURS

1:7,500

L EXTENT OF STP DOWNSTREAM RIPRAP

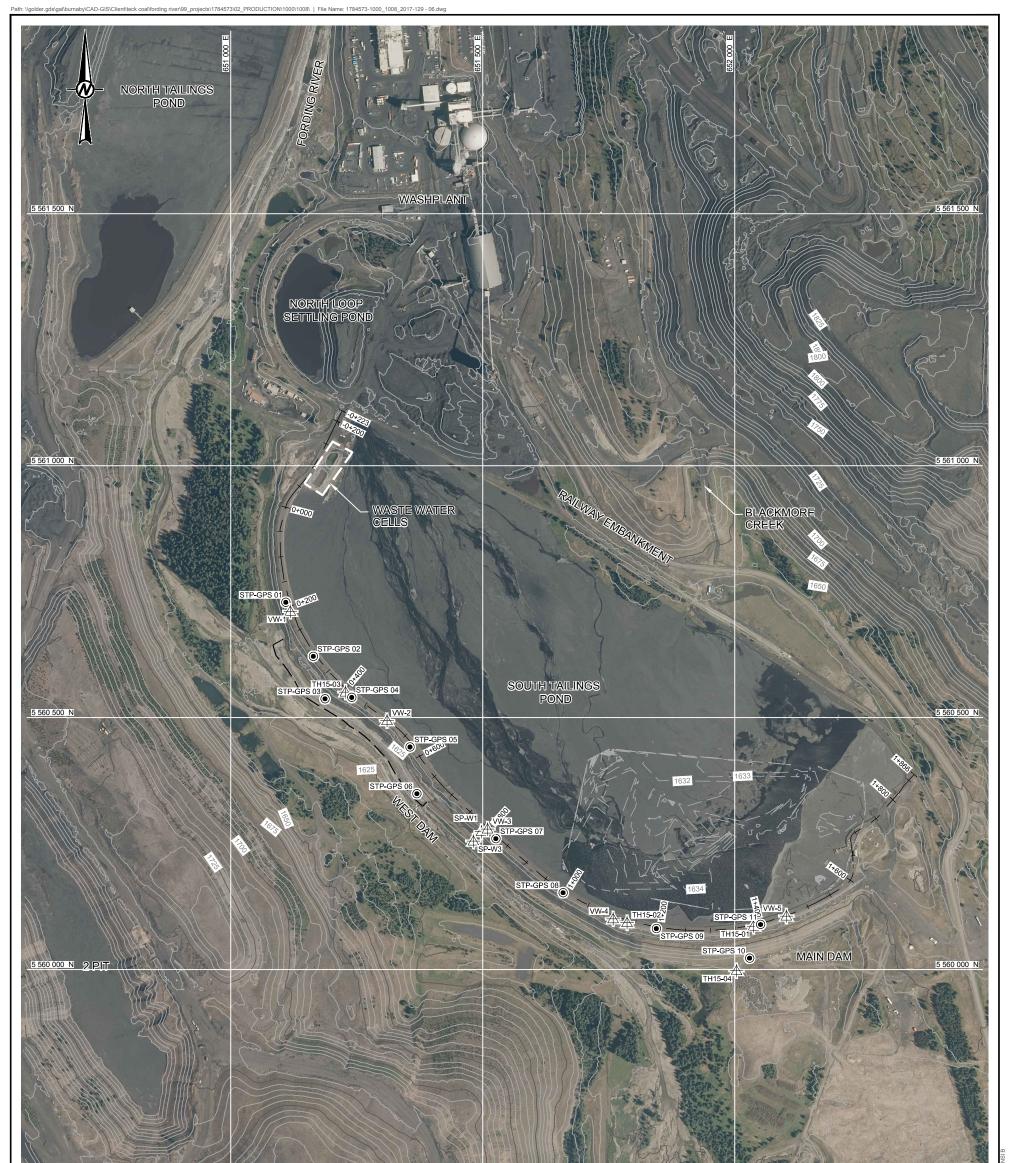
2017 SITE VISIT PHOTOGRAPH LOCATION

- NOTES
 ALL UNITS ARE SHOWN IN METRES UNLESS NOTED OTHERWISE.
 COORDINATES ARE IN UTM, ELEVATIONS ARE REFERENCED TO THE ELK VALLEY ELEVATION DATUM.
 TOPOGRAPHIC CONTOURS SHOWN AT 5.0 m MINOR AND 25.0 m MAJOR INTERVAL.
 TAILINGS CONTOURS SHOWN AT 1.0 m MINOR AND 5.0 m MAJOR INTERVAL.

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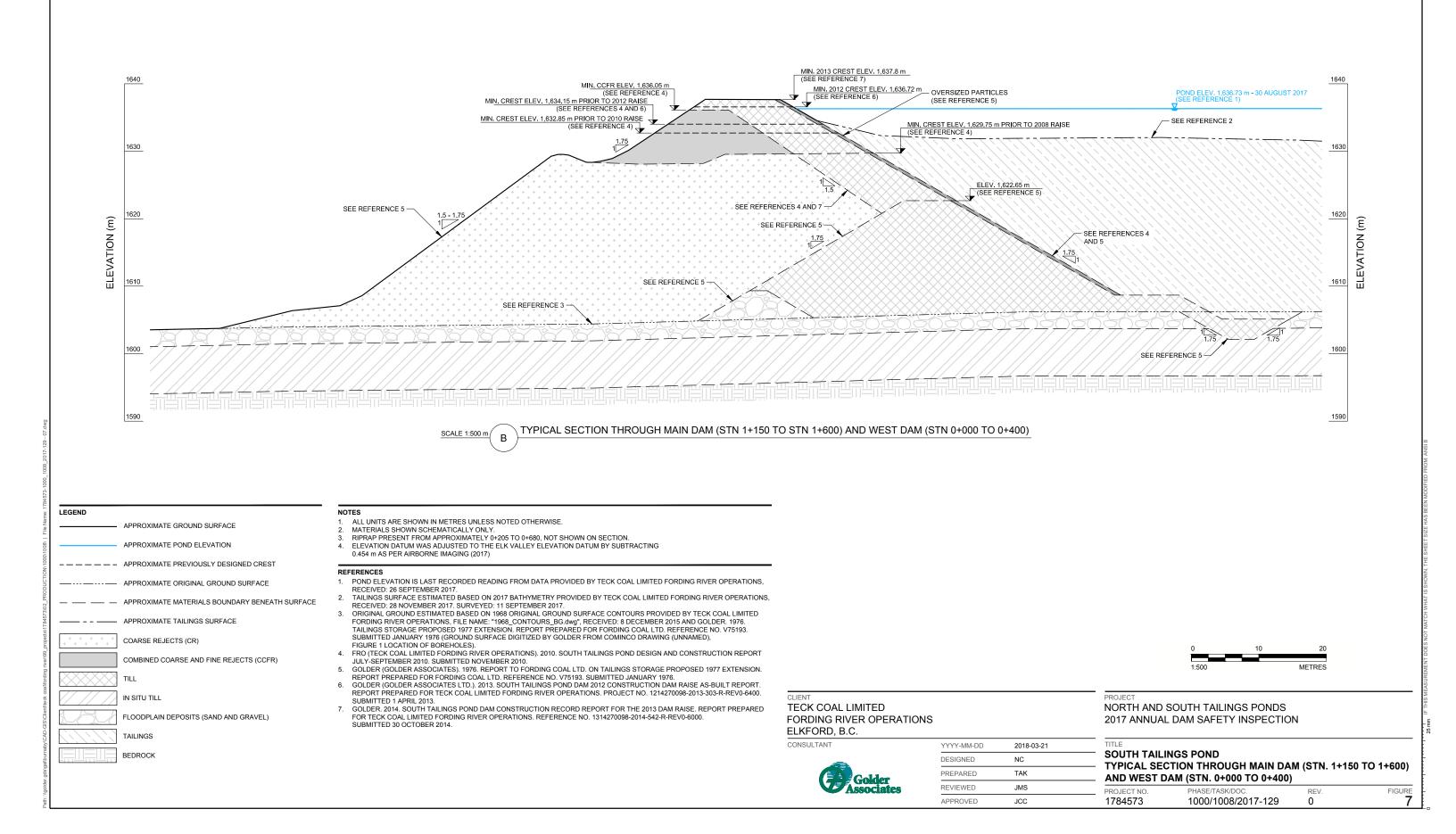
- REFERENCES
 2017 AERIAL PHOTO AND TOPOGRAPHY PROVIDED BY TECK COAL LIMITED FORDING RIVER OPERATIONS. FLOWN 25 TO 27 JULY 2017.
 2017 BATHYMETRY DATA PROVIDED BY TECK COAL LIMITED FORDING RIVER OPERATIONS. FILE NAME: Combined Surface (good).msr RECEIVED: 13 DECEMBER 2017.
 STP RIPRAP EXTENTS PROVIDED BY KERR WOOD LEIDAL ASSOCIATES LTD. RECEIVED: 17 JANUARY 2018

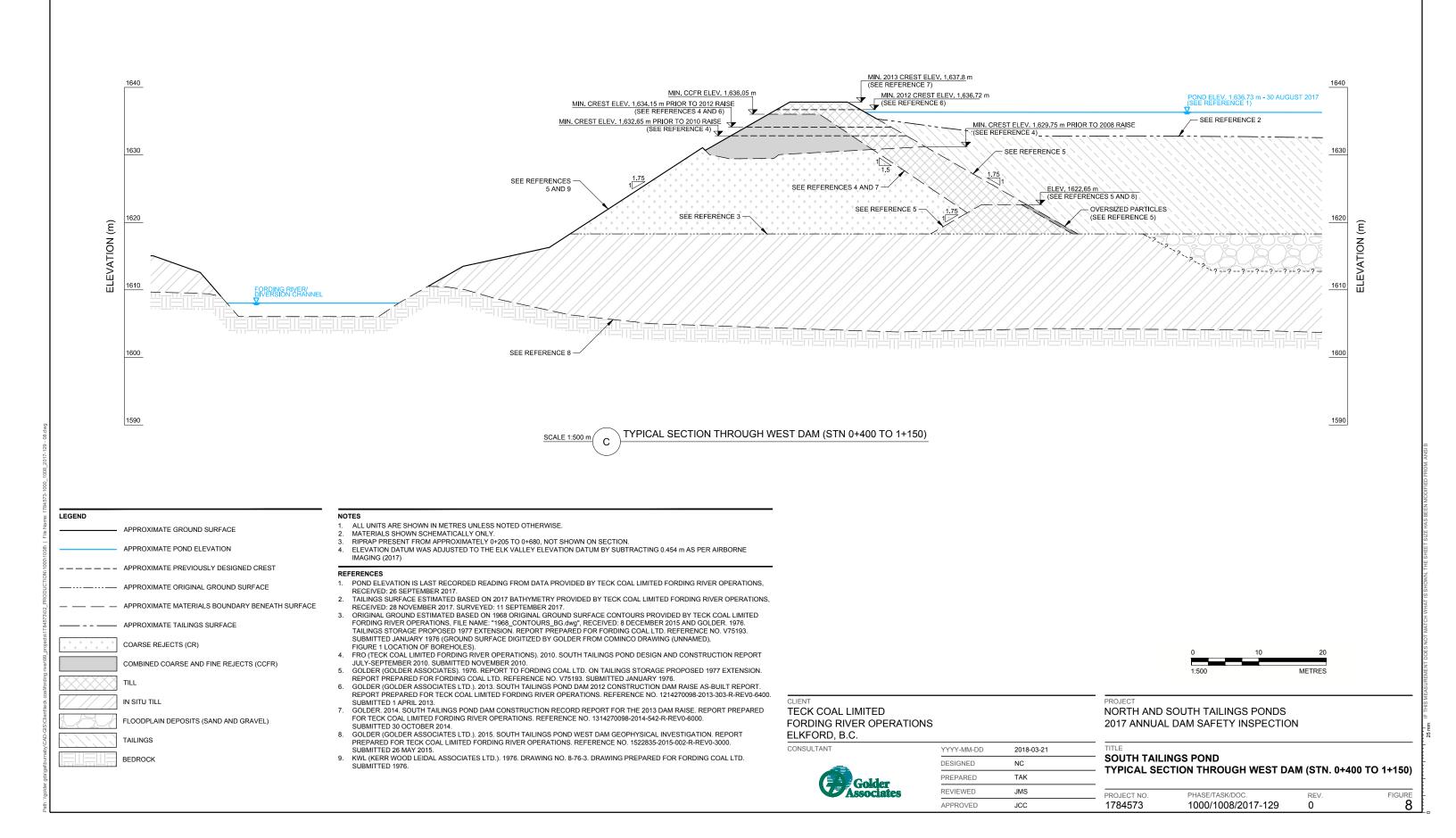
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\oplus	VIBRATING WIRE PIEZOMETER AND INCLINOM	IETER LOCATION				 SURVEY DATA FROM GEOEXPLORER, ACCESSED 13 DECEMBER 2017. LOCATIONS OF 2014 VIBRATING WIRE PIEZOMETERS BASED ON SURVEY DATA FROM GEOEXPLORER, ACCESSED 13 DECEMBER 2017. LOCATIONS OF 2015 VIBRATING WIRE PIEZOMETERS AND INCLINOMETERS BASED ON SURVEY DATA FROM GEOEXPLORER, ACCESSED 13 DECEMBER 2017. 				
♠	STANDPIPE PIEZOMETER LOCATION									
*	RETROFIT STANDPIPE WITH VIBRATING WIRE	PIEZOMETER LOCATION				 SORVET DATA FROM GEOEXPLORER, ACCESSED TO DECEMBER 2017. 2017 BATHYMETRY DATA PROVIDED BY TECK COAL LIMITED FORDING RIVER OPERATIONS. FILE NAME: Combined Surface (good).msr RECEIVED: 13 DECEMBER 2017. 				
+	SEEPAGE RETURN WELL LOCATION	CLIENT								
		TECK COAL LIMITED				PROJECT NORTH AND SOUTH TAILINGS PONDS 2017 ANNUAL DAM SAFETY INSPECTION				
		FORDING RIVER OPERATIO								
		ELKFORD, B.C.								
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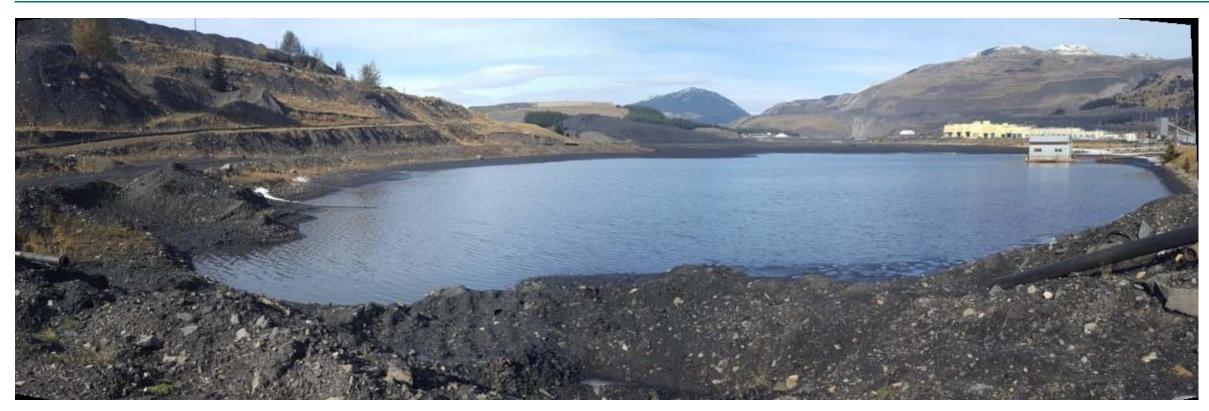




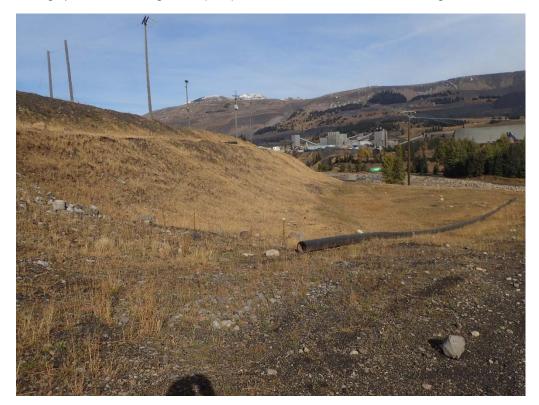








Photograph 1: North Tailings Pond (NTP) overview from south abutment, looking north. 4 October 2017.



Photograph 2: NTP dam downstream slope overview from south abutment, looking northeast. 4 October 2017.



Photograph 3: NTP dam downstream slope at south abutment with filled erosion channel, looking northwest. 4 October 2017.







Photograph 4: NTP downstream toe, animal burrows near south abutment, looking northwest. 4 October 2017.



Photograph 5: NTP barge on small pond, looking northwest. 4 October 2017.



Photograph 6: NTP dam upstream slope with small trees and vegetation growing, and unused reclaim pipes,



Photograph 7: NTP dam downstream slope with stepped erosion, looking northwest. 4 October 2017.

looking northeast. 4 October 2017.







Photograph 8: NTP dam downstream slope and toe with riprap staging area, looking east. 4 October 2017.

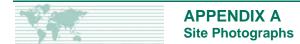


Photograph 9: NTP dam downstream slope with unused pipe along surface of slope, looking northeast. 4 October 2017.



Photograph 10: NTP dam downstream slope and riprap overview from North Loop Pond, looking northwest. 3 October 2017.







Photograph 11 : NTP crest, looking southwest. 4 October 2017.





Photograph 13 : NTP unused dual reclaim pipes on upstream slope and crossing under crest, looking north. 4 October 2017. Photograph 14 : NTP crest, looking north. 4 October 2017.



Photograph 12 : NTP toe, riprap along Fording River, looking northeast. 4 October 2017.







Photograph 15: NTP downstream slope and riprap along Fording River, looking north. 4 October 2017.



Photograph 16: NTP, tailings surface with silt fencing for dust control, looking southwest. 5 October 2017.



Photograph 17: NTP near north abutment, dozed till material and unused steel tailings pipe, looking north. 5 October 2017.







Photograph 18: NTP, surface runoff from haul road ditch reporting to tailings beach, looking west. 5 October 2017.



Photograph 19: Liverpool Pond outlet channel north of NTP, looking west. 5 October 2017.



Photograph 20: Liverpool Pond outlet north of NTP, fish control structure, looking north. 5 October 2017.







Photograph 21: NTP dam toe, downstream riprap recently raised, looking north. 4 October 2017.



Photograph 22: South Tailings Pond (STP) overview from west side of Fording River, looking southeast. 5 October 2017.







Photograph 23: STP buttress adjacent to railway line, twin culvert outlets from Blackmore Creek,

and dredge pipeline to the Turnbull South Tailings Storage Facility, looking southeast. 4 October 2017.



Photograph 24: Natural ground adjacent to railway line and emergency tailings dredge bypass, looking northwest. 4 October 2017.



Photograph 25: STP south abutment till blanket area and gas line crossing location, looking northeast. 4 October 2017.



Photograph 26: STP south abutment till blanket area and upstream slope, looking southwest. 4 October 2017.







Photograph 27: STP south abutment till blanket area, makeup water lines from Kilmarnock Ponds with riprap protection on blanket, looking northwest. 4 October 2017.



Photograph 28: STP Main Dam, barge at south end of pond and reclaim from seepage return wells, looking north.



Photograph 29: STP Main Dam, new upstream constructed riprap 1.4H:1V, looking west. 4 October 2017.

4 October 2017.







Photograph 30: STP, dredge in tailings pond, looking north. 4 October 2017.





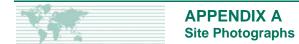
Photograph 32: STP Main Dam, downstream slope and toe at seepage return well, looking west. 4 October 2017.



Photograph 33: STP Main Dam, riprap emergency stockpile removed, and Fording River floodplain, looking south. 4 October 2017. The riprap stockpile was replaced late 2017.

Photograph 31: STP Main Dam, downstream slope at mid-slope bench (approximately elev. 1,630 m), looking west. 4 October 2017.









Photograph 34: STP Main Dam, persistent ponding downstream of toe in old river channel, looking southwest.

Photograph 35: STP corner of West and Main dams, looking northeast. Note vegetation not taking root in CCFR. 4 October 2017.





Photograph 36: STP West Dam, upstream slope with new riprap, looking northwest.

Note tailings were pushed into pond to place riprap. 4 October 2017.



Photograph 37: STP West Dam, access road used for riprap haulage, ditch re-established along bench, looking northwest.

4 October 2017.







Photograph 38: STP West Dam, downstream slope, looking northwest. 4 October 2017.

Photograph 39: STP West Dam, major seepage point at the downstream slope toe (Stn. 1+000). Seepage collection pipes monitor flow. Note discolouration. The flow rate at the time of inspection was approximately 3 L/sec from both pipes. Looking northwest. 4 October 2017.





Photograph 40: STP West Dam, seepage along the downstream toe, looking northeast. 4 October 2017.









Photograph 41: STP West Dam crest overview, looking southeast. Note crest regraded. 4 October 2017.



Photograph 42: STP West Dam, downstream slope and Fording River, looking southwest. 4 October 2017.



Photograph 43: STP West Dam, seepage at riprap at downstream toe, looking northwest. 4 October 2017.



Photograph 44: STP unused dredge line to 2P-3P Tailings Storage Area on pipe bridge, looking southwest. 4 October 2017.







Photograph 45: STP West Dam crest and tailings pond overview, looking northwest. 4 October 2017.







Photograph 47: STP West Dam, riprap placed at toe of dam for erosion protection from Fording River, looking northwest.



Photograph 48: STP West Dam, erosion gully and infilled depression, looking northeast. 4 October 2017.

4 October 2017.







Photograph 49: STP West Dam, twin culverts beneath access road at toe, looking east. 4 October 2017.

Photograph 50: STP upstream slope and crest, looking north. 4 October 2017.





Photograph 51: STP West Dam downstream slope with vegetation and ditch at downstream toe, looking south.

Photograph 52: STP West Dam, erosion gully, looking east. 4 October 2017.

4 October 2017.











Photograph 53: STP West Dam, pond at downstream toe near Stn.-0+050, looking northwest. 4 October 2017.



Photograph 54: STP north abutment, waste water cells, looking southwest. 4 October 2017.





Photograph 55: STP north abutment, recently constructed upstream till berm, looking northeast. 4 October 2017.

Photograph 56: STP north abutment, outlet of tailings pipe and water return pipe, looking southeast. 3 October 2017.

https://golderassociates.sharepoint.com/sites/16299g/deliverables/issued/2017-129-r-rev0-1000_ntp-stp_dsi_2017/appendix a - site photographs/appendix a - site photographs.docx





APPENDIX B

North Tailings Pond Inspection Report





Client:	Teck Coal Limited, Fording River Operations	By:	John Cunning, P.Eng., , E.I.T.
Project:	FRO Dam Safety Inspection	Date:	4 to 5 October 2017
Location:	North Tailings Pond	Reviewed:	John Cunning, P.Eng.

GENERAL INFORMATION

Dam Type: Zoned Earth Fill					
Weather Conditions:	Clear and Sunny	Temp:	0 to 10°C		

Inspection Item	Observations/Data	Photo	Comments & Other Data	
1.0 DAM CREST		6, 11, 13, 14		
1.1 Crest Elevation	Elev. 1,652.6 m (minimum)		2017 NTP crest survey provided by FRO confirms minimum elevation (Elk Valley Elevation Datum).	
1.2 Reservoir Level/ Freeboard	Elev. 1,650.0 m (10 August 2017) 2.8 m freeboard	1, 5	From 2017 pond elevation survey data provided by FRO (Elk Valley Elevation Datum).	
1.3 Distance to Tailings Pond (if applicable)	O m (south end) Approx. Stn. 1+200 to 1+400; Full beach Approx. Stn 0+000 to	1, 5	Usually no beach at south end	
	1+200			
1.4 Surface Cracking	None			
1.5 Unexpected Settlement	None			
1.6 Lateral Movement	None			
1.7 Other Unusual Conditions	Yes	9, 13, 17	 pipe crossings under the crest: dual steel pipes steel pipe to culvert on downstream face shallow PVC pipes old tailings delivery pipe at former bridge abutment 	





Inspection Item	Observations/Data	Photo	Comments & Other Data	
2.0 UPSTREAM SLOPE		1, 6, 13		
2.1 Slope Angle	1.4 to 1.5H:1V			
2.2 Signs of Erosion	2 Signs of Erosion Minor surficial erosion			
2.3 Signs of Movement (Deformation)	None			
2.4 Cracks	None			
2.5 Face Liner Condition (if applicable)	N/A			
2.6 Other Unusual Conditions	Yes	6, 13		
3.0 DOWNSTREAM SLOPE		2, 3, 7, 8, 9, 10, 15		
3.1 Slope Angle	1.4 to 1.75 H:1 V		Original design of 1.4 H:1 V; rebuilt design of 1.5 to 1.75 H:1 V due to 2013 flood.	
3.2 Signs of Erosion	Minor surficial erosion, not stability concern	7 3	Minor stepped erosion throughout downstream slope. Vertical channel at south abutment appears stable since repairs in 2016.	
3.3 Signs of Movement (Deformation)	None			
3.4 Cracks	None			
3.5 Seepage or Wet Areas	Dry			
3.6 Vegetation Growth	Variable	2, 7, 15	Grasses appropriate on old slopes. Good grass growth on soil cover placed on rebuilt slopes.	
3.7 Other Unusual Conditions	Yes		Vertical culvert in downstream slope	
4.0 DOWNSTREAM TOE AREA		8, 12, 15		
4.1 Seepage from Dam	None			
4.2 Signs of Erosion	No	12, 15	Riprap placed to protect from Fording River erosion, in good condition.	



Inspection Item	Observations/Data	Photo	Comments & Other Data
4.3 Signs of Turbidity in Seepage Water	None		
4.4 Discoloration/Staining	None		
4.5 Outlet Operating Problem (if applicable)	N/A		
4.6 Other Unusual Conditions	None		
5.0 ABUTMENTS		2, 17, 20	
5.1 Seepage at Contact Zone (Abutment/Embankment)	None		
5.2 Signs of Erosion	Minor		
5.3 Excessive Vegetation	None		
5.4 Presence of Rodent Burrows	Yes	4	Animal burrows in toe near south abutment.
5.5 Other Unusual Conditions	Yes	18	Surface runoff from haul road reports to north end of tailings beach.
		20	Liverpool outlet channel and fish barrier at north end of NTP.
6.0 RESERVOIR		16	
6.1 Stability of Slopes	Spoils west of tailings storage facility	16	Spoils resloped in March 2015
6.2 Distance to Nearest Slide (if applicable)	N/A		
6.3 Estimate of Slide Volume (if applicable)	N/A		
6.4 Floating Debris	None		
6.5 Other Unusual Conditions	Yes	5	Barge is crooked from being stuck in tailings, barge not in use.
7.0 EMERGENCY SPILLWAY/			
OUTLET STRUCTURE	None		No spillway or emergency outlet.
7.1 Surface Condition	N/A		
7.2 Signs of Erosion	N/A		



Inspection Item	Observations/Data	Photo	Comments & Other Data
7.3 Signs of Movement (Deformation)	N/A		
7.4 Cracks	N/A		
7.5 Settlement	N/A		
7.6 Presence of Debris or Blockage	N/A		
7.7 Closure Mechanism Operational	N/A		
7.8 Slope Protection	N/A		
7.9 Instability of Side Slopes	N/A		
7.10 Other Unusual Conditions	N/A		
8.0 INSTRUMENTATION			
8.1 Piezometers	Yes		Three piezometers installed in 2015, see Section 5.3.1.3. Locations shown in plan in Figure 3.
8.2 Settlement Cells	None		
8.3 Thermistors	None		
8.4 Settlement Monuments	Yes		Prisms and GPS unit monitor crest movements - see Appendix H. Locations shown in plan in Figure 3.
8.5 Accelerograph	None		
8.6 Inclinometer	Yes		Three inclinometers installed in 2015 – see Appendix I. Locations shown in plan in Figure 3.
8.7 Weirs and Flow Monitors	None		
8.8 Data Logger(s) Yes			On piezometers and GPS, all instrumentation connected to GeoExplorer system.
8.9 Other	None		





Inspection Item	Observations/Data	Photo	Comments & Other Data	
9.0 DOCUMENTATION				
9.1 Operation, Maintenance and Surveillance (OMS) Manual	Yes		SP&P GN.029A.R5, 17 December 2015.	
9.1.1 OMS Manual Exists				
9.1.2 OMS Manual Reflects Current Dam Conditions	No			
9.1.3 Date of Last Revision	OMS manual in draft		Under review by FRO, NTP to be included in NTP-STP OMS manual. Updated revision pending.	
9.2 Emergency Preparedness Plan (ERP)	Yes		NTP included in site tailings facilities ERP (SP&P EP.009)	
9.2.1 ERP Exists	ERP: Internal to Teck EPP: External to Teck		(FRO 2017). EPP SP&P EP.008.R1	
9.2.2 ERP Reflects Current Conditions	ERP reflects current conditions on site, EPP to be updated based on ERP once finalized.			
9.2.3 Date of Last Revision	ERP: September 2017, in draft EPP:			
	15 December 2015			

10. NOTES

Currently, no active deposition of tailings into the NTP. Barge is not being operated and pipes are not connected. Ability to pump water from the pond is part of storm water management; temporary pumps need to be available in the event pumping is necessary. The temporary pipeline that crosses the Fording River from Shandley Pit to the STP would be used in an emergency pumping event.

Upgrades to erosion protection along the downstream toe was completed at the time of the site inspection.

The future use of the facility is currently under review by FRO.

Inspectors	, E.I.T.	Date:	4 October 2017
		-	-

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APPENDIX C

South Tailings Pond Inspection Report





Client:	Teck Coal Limited, Fording River Operations	By:	John Cunning, P.Eng., E.I.T.
Project:	FRO Dam Safety Inspection	Date:	4 to 5 October 2017
Location:	South Tailings Pond	Reviewed:	John Cunning, P.Eng.

GENERAL INFORMATION

 Dam Type: Zoned Earth Fill

 Weather Conditions:
 Clear and Sunny
 Temp:
 0 to 10°C

Inspection Item	Observations/Data	Photo	Comments & Other Data	
1.0 DAM CREST		29, 36, 41, 45, 50		
1.1 Crest Elevation	Elev. 1,637.8 m (minimum)		From FRO survey of last raise in 2013. Confirmed with 2017 LiDAR survey (Elk Valley Elevation Datum).	
1.2 Reservoir Level / Freeboard	Elev.1,636.28 m (30 August 2017) 1.52 m freeboard	26, 28, 29	From 2017 pond elevation survey data provided by FRO (Elk Valley Elevation Datum).	
1.3 Distance To TailingsPond(if applicable)	0 m at south along Main Dam. Variable beach along West Dam	29, 30 45, 50		
1.4 Surface Cracking	None			
1.5 Unexpected Settlement	None			
1.6 Lateral Movement	None			
1.7 Other Unusual Conditions or Structures	No		Crest was regraded in 2017	
2.0 UPSTREAM SLOPE		26, 29, 36, 50		
2.1 Slope Angle	Generally 1.4 H to 1.75 H : 1 V	26, 29	 Till crest graded to drain upstream. Riprap placed along upstream slope of Main Dam in 2017 	
2.2 Signs of Erosion	None			



Inspection Item	Observations/Data	Photo	Comments & Other Data
2.3 Signs of Movement (Deformation)	None		
2.4 Cracks	None		
2.5 Face Liner Condition (if applicable)	N/A		
2.5 Other Unusual Conditions	No		
3.0 DOWNSTREAM SLOPE		31, 32, 38, 42, 46, 47, 48, 51, 52	
3.1 Slope Angle	± 1.5 to 1.75 H : 1 V	31, 32	 Oversteepened with respect to Main Dam design which calls for 1.75 H : 1 V. Not re-graded over whole slope height, local sections of oversteep slope with benches.
3.2 Signs of Erosion	Minor channels Repaired old channels	35, 48, 52	 Old CCFR benches have surface erosion channels. Repaired erosion channels are performing well. Erosion channels in Main and West Dam should continue to be monitored and be filled with coarse rock fill as per previously repaired channels.
3.3 Signs of Movement (Deformation)	None		
3.4 Cracks	None		
3.5 Seepage or Wet Areas	None		
3.6 Vegetation Growth	Good on Main Dam Poor on West Dam	35	West Dam south end vegetation not rooting, improve growth on dam.
3.7 Other Unusual Conditions	None		





Inspection Item	Observations/Data	Photo	Comments & Other Data
4.0 DOWNSTREAM TOE AREA		32, 33, 34, 39, 40, 43, 47, 51	
4.1 Seepage from Dam	Yes on West and Main Dams	53 39, 40 43 34	 West Dam Pond at toe at north end. Persistent seepage from West Dam areas at north end and on bench above Fording River diversion channel. Saturated ground observed in cut slope at toe of West Dam above area of new riprap. Main Dam Pond south of dam toe.
4.2 Signs of Erosion	None	22	
4.3 Signs of Turbidity in Seepage Water	None		
4.4 Discoloration/Staining	Yes (green, red)	39 40	 Green mineral (possible calcite) deposits from seepage water. Some minor areas with red colored staining in seepage from bedrock contact.
4.5 Outlet Operating Problem (if applicable)	N/A		
4.6 Other Unusual Conditions	None		
5.0 ABUTMENTS			
5.1 Seepage at Contact Zone (abutment/embankment)	None		
5.2 Signs of Erosion	None		
5.3 Excessive Vegetation	None		
5.4 Presence of Rodent Burrows	None		



Inspection Item	Observations/Data	Photo	Comments & Other Data
5.5 Other Unusual Conditions	Yes	55	 Gas main pipeline in north abutment area did not allow for abutment section of dam to tie into interim berm built. Till berm constructed near north abutment in 2017.
6.0 RESERVOIR		22, 30	
6.1 Stability of Slopes	Stable	23	 Railway embankment on east side of impoundment has a buttress berm. Small natural ground slope present north of the railway embankment on east side of reservoir (low potential for slide generation).
6.2 Distance to Nearest Slide (if applicable)	Adjacent to impoundment		 Slide from railway embankment would impact tailings beach and/or pond water. Potential for generation of small waves from slide into water. Slide from small slope would impact tailings beach. Little to no potential for wave generation.
6.3 Estimate of Slide Volume (if applicable)	Minor		 Potential slide volume from railway embankment or small slope would be small.
6.4 Floating Debris	None		·
6.5 Other Unusual Conditions	Yes	56 23 30 54	 Tailings beach is nearing elevation of plant discharge pipe outlet. Tailings beach in nearing elevation of Blackmore Creek culverts outlet. Tailings being dredged to Turnbull TSF May to October 2017. Dredge operations active at time of inspection. Waste water cells in operation near the north
7.0 EMERGENCY SPILLWAY/ OUTLET STRUCTURE	None		 abutment. No spillway or emergency outlet.





Inspection Item	Observations/Data	Photo	Comments & Other Data
7.1 Surface Condition	N/A		
7.2 Signs of Erosion	N/A		
7.3 Signs of Movement (Deformation)	N/A		
7.4 Cracks	N/A		
7.5 Settlement	N/A		
7.6 Presence of Debris or Blockage	N/A		
7.7 Closure Mechanism Operational	N/A		
7.8 Slope Protection	N/A		
7.9 Instability of Side Slopes	N/A		
7.10 Other Unusual Conditions	N/A		
8.0 INSTRUMENTATION			
8.1 Piezometers	Yes		 West Dam (see Section 5.4.1.3) 2 standpipes (not read) 2 retrofit standpipes with vibrating wire 4 VW piezometers Main Dam (see Section 5.4.1.3) 1 standpipe (not read) 2 retrofit standpipes with vibrating wire 5 VW piezometers Locations shown in plan in Figure 5
8.2 Settlement Cells	None		
8.3 Thermistors	None		
8.4 Settlement Monuments	Yes		GPS units monitor crest and toe movements - see Appendix H. Locations shown in plan in Figure 5.
8.5 Accelerograph	None		





Inspection Item	Observations/Data	Photo	Comments & Other Data
8.6 Inclinometer	Yes		West Dam - 1 location Main Dam - 3 locations See Appendix I. Locations shown in plan in Figure 5.
8.7 Weirs and Flow Monitors	None		
8.8 Data Logger(s)	Yes		On piezometers and GPS units, all instrumentation connected to GeoExplorer system.
8.9 Other	None		
9.0 DOCUMENTATION			
9.1 Operation, Maintenance and Surveillance (OMS) Manual			SP&P GN.029A.R5, 17 December 2015
9.1.1 OMS Manual Exists	Yes		
9.1.2 OMS Plan Reflects Current Dam Conditions	No		
9.1.3 Date of Last Revision	OMS manual in draft		Under review by FRO, STP to be included in in NTP-STP OMS manual. Updated revision pending.
9.2 Emergency PreparednessPlan (EPP)9.2.1 EPP Exists	Yes ERP: Internal to Teck EPP: External to Teck		STP included in site tailings facilities ERP (SP&P EP.009) (FRO 2017). EPP SP&P EP.008.R1
9.2.2 EPP Reflects Current Conditions	ERP reflects current conditions on site, EPP to be updated based on ERP once finalized.		
9.2.3 Date of Last Revision	ERP: September 2017, in draft		
	EPP: 15 December 2015		





Inspection Item	Observations/Data	Photo	Comments & Other Data
10. NOTES			

North abutment final alignment on hold since 2013 due to gas main pipeline; interim berm in place until gas main relocated or north abutment redesigned.

Erosion channels in CCFR benches should continue to be monitored and to be repaired as required similar to previous repairs.

Continue to monitor seepage including measuring flows where possible along downstream toe of West Dam.

	Inspectors		, E.I.T.	Date:	4 October 2017
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https://golderassociates.sharepoint.com/sites/16299g/deliverables/issued/2017-129-r-rev0-1000_ntp-stp_dsi_2017/appendix c - south tailings pond inspection report/appendix c - stp insp rep.docx

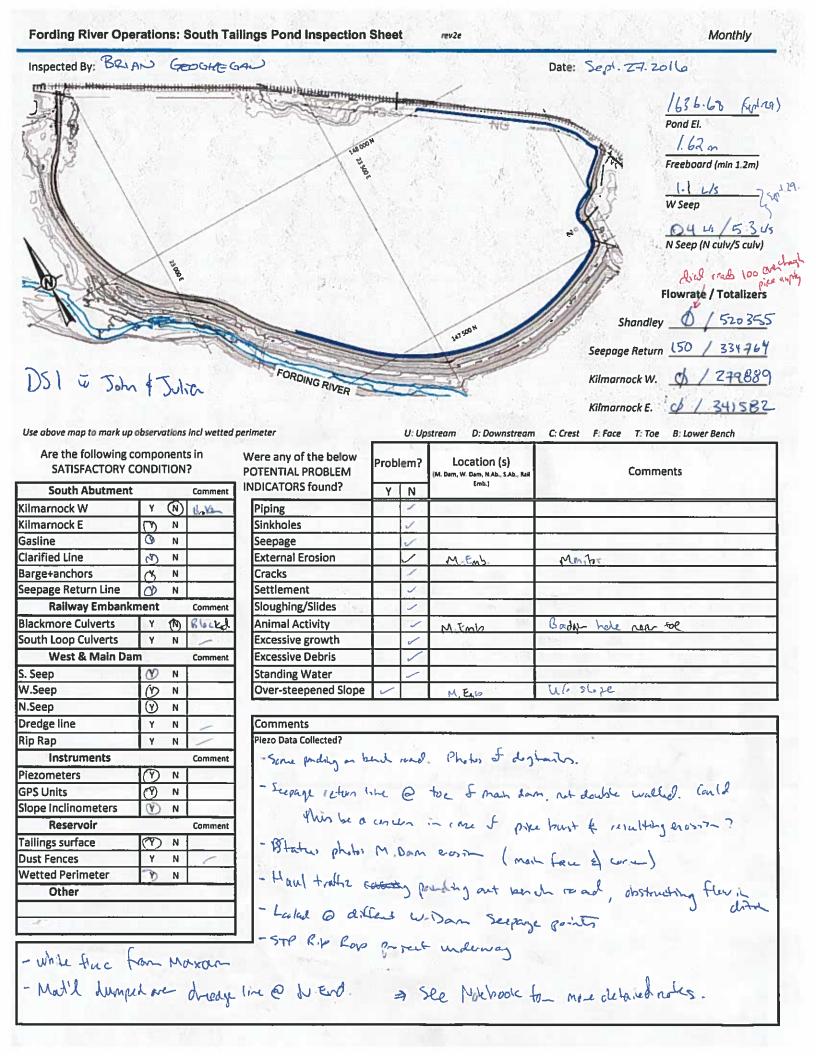




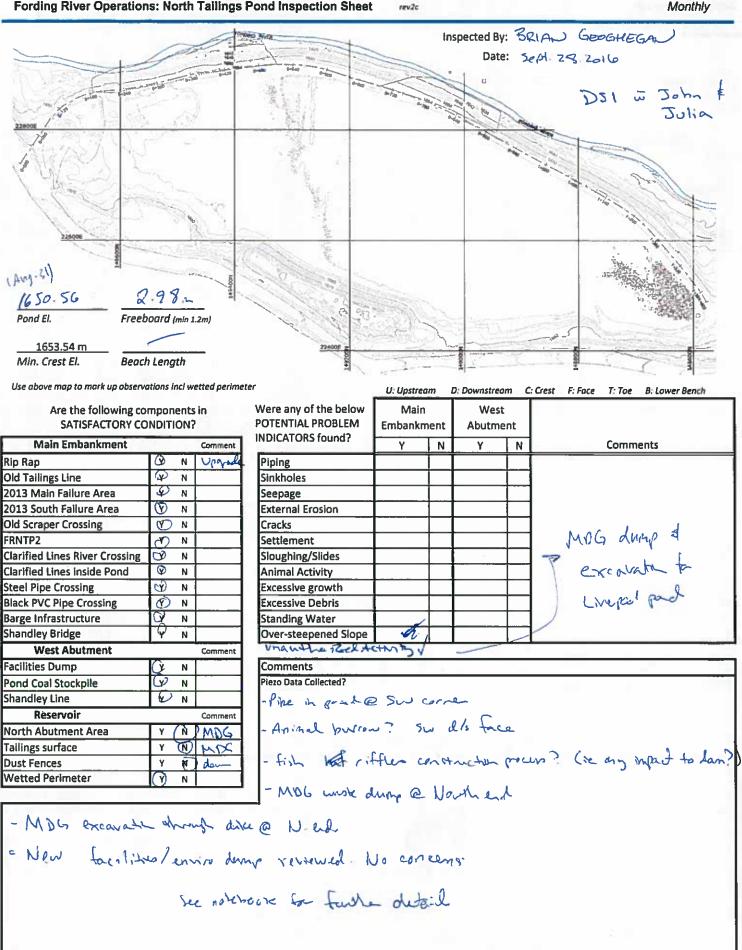
APPENDIX D

Fording River Operations Personnel Tailings Pond Inspections

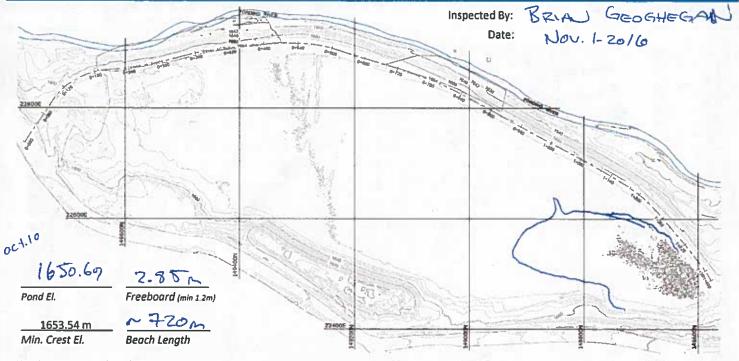




Fording River Operations: North Tailings Pond Inspection Sheet



Fording River Operations: North Tailings Pond Inspection Sheet rev2c



U: Upstream

Use above map to mark up observations incl wetted perimeter

Are the following components in SATISFACTORY CONDITION?

Main Embankment		Comment
Rip Rap	(b) N	Warad
Old Tailings Line	О N	
2013 Main Failure Area	Ý N	
2013 South Failure Area	🕑 N	
Old Scraper Crossing	() N	
FRNTP2	V N	
Clarified Lines River Crossing	X N	
Clarified Lines inside Pond	Y N	
Steel Pipe Crossing	3 N	
Black PVC Pipe Crossing	Q N	the second
Barge Infrastructure	🕑 N	
Shandley Bridge	Y N	
West Abutment	-	Comment
Facilities Dump	(Y) N	New
Pond Coal Stockpile	2 N	
Shandley Line	Y N	
Reservoir		Comment
North Abutment Area	Y -> N	Rowins
Tailings surface	N N	
Dust Fences	Y~N	
Wetted Perimeter	🕐 N	a second and

Were any of the below POTENTIAL PROBLEM	Main Embankment		West Abutment						
NDICATORS found?	Y	N	Y	N	Comments				
Piping	0.0	1		1					
Sinkholes		-							
Seepage		1		/	see noter below.				
External Erosion		K			the notar adam.				
Cracks		1		1					
Settlement		1	1	1					
Sloughing/Slides		1		1					
Animal Activity			5	1					
Excessive growth			=	1					
Excessive Debris		1		1					
Standing Water		1		1	10				
Over-steepened Slope		17		17					

D: Downstream

C: Crest

F: Face

T: Toe

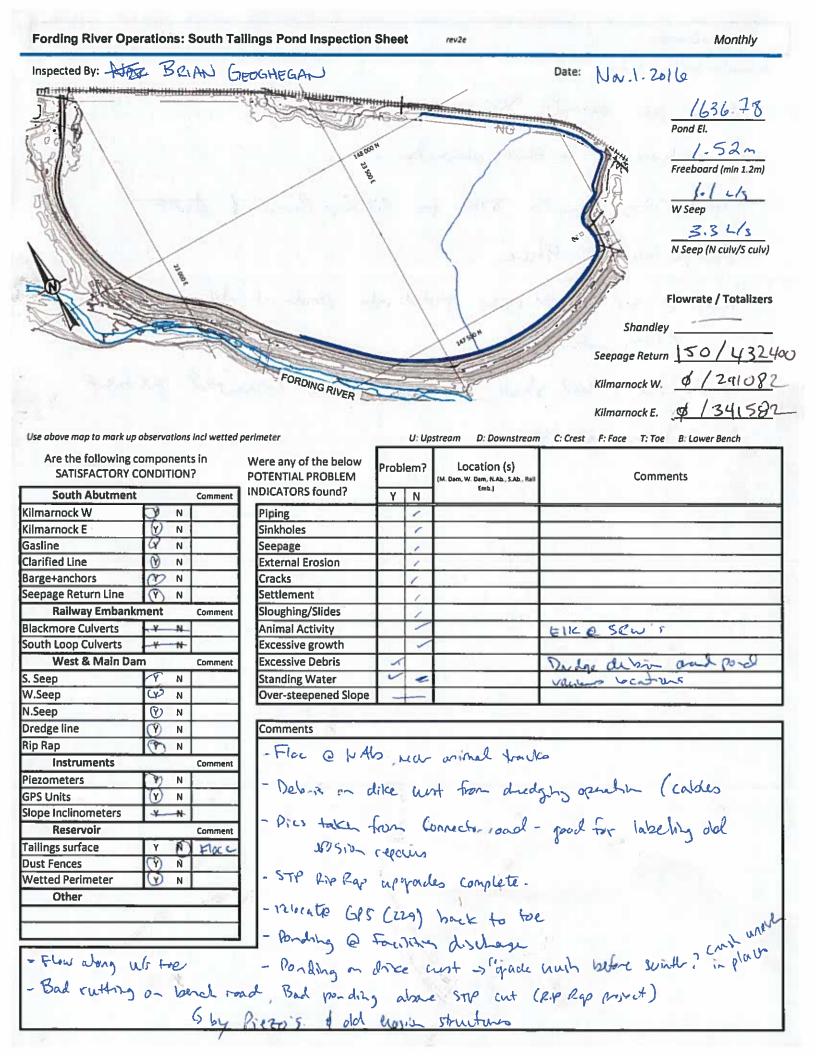
B: Lower Bench

Comments

- WTP TOP Pap regades underway (NTP Junys, & South See)
- Maning MDG stock, and - Spille to TMM (contracted),
pade sure viry don't art steepe the stockpile in the
press - will use have to boil back for edge
& feed a dozer.

- LVP dischast chunch seems complete - New sediment dump in use. Sloppy entrance, no safety burn & dump locarin - Surface and looks good

Stockering hip hap materials @ perhing then the wat Barse (accordian Maxan)



Follow-up Items from previous inspection:

Statum prz., ensine SW corre
- u/s starre @ wetter plante
- stockping benath sons for Riv Rap Port & Awrt
- Badge Lole stru thee
- Road @ toe & M. Earls graded into ponds & old rive chandle
(diving wete?)
- Bend road in bad shape, adjacent did not connected ad bed

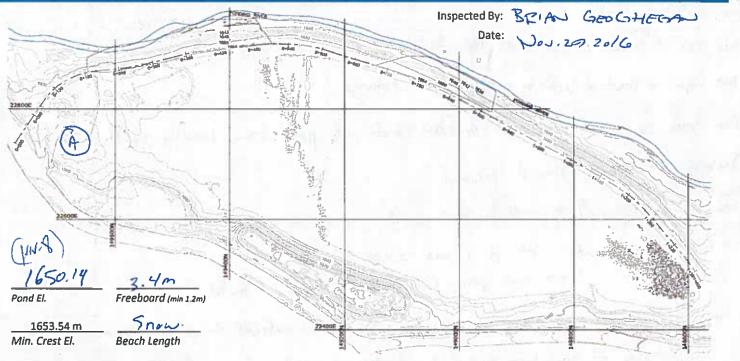
- New bern @ STP cut

which and another that a stranger

the same state the same state of the same state of the same state the same state of the same state of

CONTRACT MEMORY AND A DESCRIPTION

Fording River Operations: North Tailings Pond Inspection Sheet rev2c



Use above map to mark up observations incl wetted perimeter

Are the following components in SATISFACTORY CONDITION?

Main Embankment		Comment
Rip Rap	N (Contra
Old Tailings Line	Y N	chede
2013 Main Failure Area	1 N	
2013 South Failure Area	6 N	
Old Scraper Crossing	(N	
FRNTP2	1 N	
Clarified Lines River Crossing	Ø N	
Clarified Lines Inside Pond	(?) N	
Steel Pipe Crossing	🕐 N	1.000
Black PVC Pipe Crossing	W N	
Barge Infrastructure	1 N	
Shandley Bridge Couls	(?) N	1.1
West Abutment		Comment
Facilities Dump	Y N	check
Pond Coal Stockpile	😗 N	
Shandley Line	D N	1111
Reservoir		Comment
North Abutment Area	Y N	Cinhim
Tailings surface	Y N	Luchim
Dust Fences	Y - N	Contra
Wetted Perimeter	(B N	

Were any of the below POTENTIAL PROBLEM NDICATORS found?	U: Upstream L Main Embankment		D: Downstream C West Abutment		: Crest F: Face T: Toe B: Lower Bench
	Y	N	Y	N	Comments
Piping	1112	1/		1	WE ALLOW A STREET D
Sinkholes	1.	1		1	
Seepage		1		1	And A Destruction of the
External Erosion		1		0	
Cracks		1	J X	1	which we are and the
Settlement		1		1	and the second se
Sloughing/Slides		1	1	1	
Animal Activity	1			1	Coyote
Excessive growth		1	S	(
Excessive Debris		1		1	LI I C SWA
Standing Water	1			1	North extract, from Swi
Over-steepened Slope		1	1000	1	

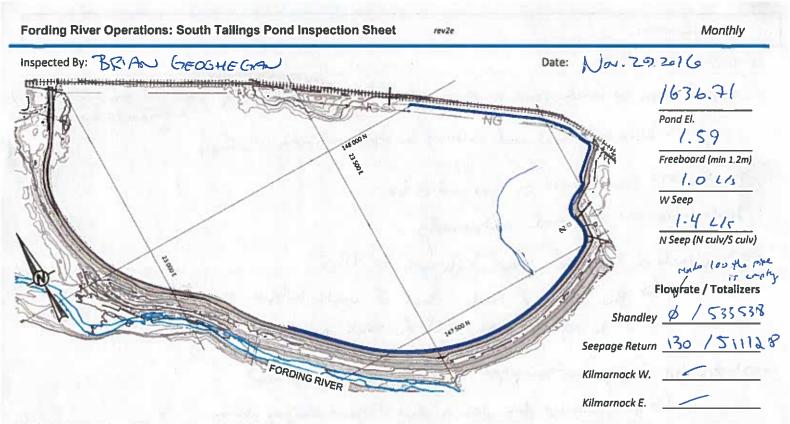
Comments

Swift Construction: burn + ditch along LVP athonnel. to key much from oid MOL shock y le from flowing into channel To reat to Barkhung supp in TSF. Directional is alloch - o) sing away from dive is don't truck integrate for IDF containment) Source innal : and don't automating in the container or is forme to go in tomorrow - connect wat & tre @ N-and where swift out accur

Water from Loe's lake system connecting reporting to NTP (since Sat.) Son Culvet from box structure is to - take LVP convition - How concide plug, but plug was remard on movin & decisin by G.Svord - water flowing it lear takes yeter needed go somewhen, dist of wat it don't to rive wat't some final debails about we (bene around chandlen) ble & tailings reliancement - laciant water from Ld. aparted to box shudwer, wat act 5 Culver, tops in that drea was such that water certial around back to the discharge chancel. It picked up tails of deposited that tails into the LVP channel. No wate through channel now word tails cleaned up Topo dls & s. (which has already been corrected, wate no collections in pand @ W. ed & NTP (xe(A) - A

Dozen tracks at MOG (Hatted) stackput - not work by avoid scouting ditch contraction?

Additional Comments	
Follow-up items from previous inspe	
	t activity at sed dump
	- & activity at NTP Snaw dump
- Area mow course	L. No sign of sufface runals into panel from shadley pyreline road
serving bankers	us drough nighting
- Snow clearing (gra	de) occurred in dike cuit
- Lip Rap priset :	muche bic of recent ymm in ant
	court prace to the mathials, the court build
- cheet old phot	os to make sure doter tracks/ small material dung avoid new.
C Mgside	old tails line stule up, db face & dan)
-coyote, da	k wate from flow and of LL. doop has shurtere
- Sump created Creater	by MDG stockpile push, immuchately wir of dike cust @ N. ed
-sed dump- an	y getech concen /mitigations needed for quarky beaut steep side face?
- Hazard : cimin	g out onto Connects road from NTV? . high speed hand road, und to be maite
- Shandly Flow	meter - needs a buggy while for ID whe snewcornel.
	foldize = 533538
	flow = gd, but divid words 100
1	



Use above map to mark up observations incl wetted perin

Are the following components in SATISFACTORY CONDITION?

South Abutment			Comment
Kilmarnock W	Y	N	Gone
Kilmarnock E	Y	N	Gore
Gasline	Y	Ν	
Clarified Line	Y	Ν	1158
Barge+anchors	y	N	
Seepage Return Line	Y	N	Date of
Railway Embankr	nent		Comment
Blackmore Culverts	Y	N	doutre
South Loop Culverts	*	*	_
West & Main Dar	n		Comment
S. Seep	Y	Ν	
W.Seep	Y	N	
N.Seep	X	N	_
Dredge line	x	Ν	
Rip Rap	Y	Ν	
Instruments	80.0		Comment
Piezometers	0	-14	
GPS Units	0	N	
Slope Inclinometers		-11-	
Reservoir			Comment
Tailings surface	Ø	N	
Dust Fences	+	N	
Wetted Perimeter	\bigcirc	Ν	
Other			
			2.1

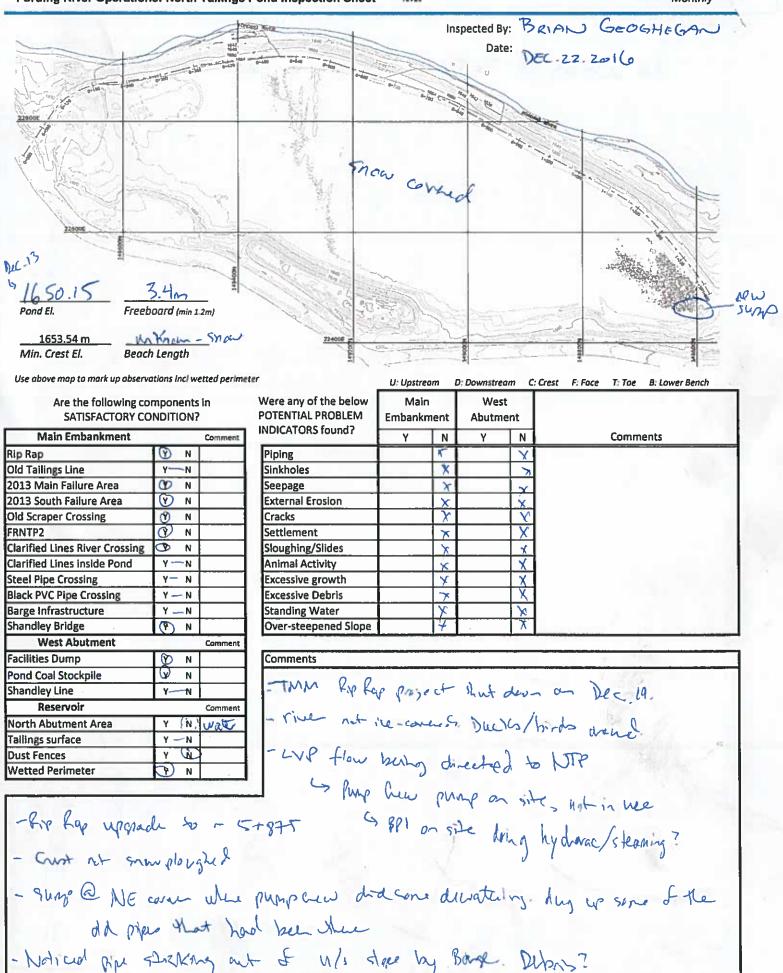
erimeter	-	U: Up	stream D. Downstream	C: Crest F: Face T: Toe B: Lower Bench		
Were any of the below POTENTIAL PROBLEM	Problem?		Location (s) (M. Dam, W. Dam, N.Ab., S.Ab., Rail	Comments		
INDICATORS found?			Emb.)			
Piping		1	and the second second			
Sinkholes		-		1.		
Seepage		1				
External Erosion		1		Service and the service of the servi		
Cracks		1	Sel stre manual			
Settlement		1				
Sloughing/Slides		1				
Animal Activity	1		N	coyete		
Excessive growth		/		AUGAN STATES		
Excessive Debris		1				
Standing Water	-	-	Bud road			
Over-steepened Slope	-	-16 II	uls	not a concern to white		

Dredge line	Y N	Comments
Rip Rap	Y N	-Contrenominal collected sample from N. Abutonet pool yesteday.
Instruments	C	omment
Piezometers	N	- Born from tails brach modification will answe will an
GPS Units	() N	Shuld it which the
Slope Inclinometers	-YN-	mind the WWCs ever decort. Not a which, which is which
Reservoir	G	-brach is snow connect - Brach is snow connect
Tailings surface	Ø N	- A Contract
Dust Fences	TN	- the did water simpling & he hap yelday - should do have about a
Wetted Perimeter	() N	- Env. drd water sampling & he say yeleday - should do have done to sap flow wassurement. Need to conduct to the monther by schedule/tasks - copie With the from wagto by Maxam. (the iten) - possimily to N sug flow road
Other		Nutrick free reactions that the second of the second
		- copie howshy acon the took can after iten - proximity to the sup them norder
		- should rip ig road have a berm? # howling from always rive
		should rip up road have a burn () (
- Need Bun also	ve STP cut	Jup in snew gavered driving surface
		The property of the start the almost got stak on the wey out
-bench nod s	full Mido	wirk Sping said July hanne delivery truck almost got stuk on the way ant wirk Sping said July hanne delivery truck almost got stuk on the way ant
1-0	the day	p at bolting wet by buch provis & and spot is deep - almost stuck in this
	any all	the lat polyny more as which by the states in the

Follow-up items from previous inspection:

-should clean up bunch road as soon as possible in early sports (and mak now world inst) determate ary way Is access along bench road necessary for theod mitigatic shategy - Sw seep still flawing housed on wit read & fore - Terry bomins up drand instruments - Flormeters remared from Kilmanock likes 19 BU Should have control of water below so can have authority to require notification a such activities / items -Obstructed dudy orestlow pipe still needs to be address? () spring cleanup item. prix to start I next dealing season - Part starting to ite up. Bage deriving is effective & wave active caren has ul slare cutting a now wat until 2017 sonts - Maxam froth area now snow-consult not an issue. Pick up in space

Fording River Operations: North Tailings Pond Inspection Sheet rev2c



Follow-up items from previous inspection:

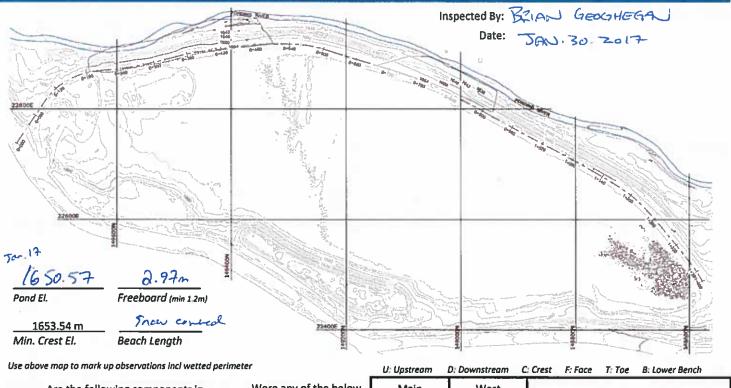
No activity at Sediment Durp a Stor Durp

Inspected By: BRIAN GEOGH	EGAN				Date: Dec. 2	2/2016
	and Bernin Janman	munk	/			
	Charles and the	AGS	and the second s		The statement of the st	1636.85
	Min	/		NG -	and the second s	Pond El.
	made is the is	4			144	1.45m
	2 that		-		1 PAN	Freeboard (min 1.2m)
	1				B.W	LOLIS
	/	-			15.8	W Seep
844 I		1			(XA)	3.2 Lls
ANN I	500				-2° 13	
A MARY A		y				N Seep (N culv/S culv)
A Color					1115	
CO STREET		/		/		Flowrate / Totalizers
Mangel				/	1181H	
MIL STREET			1		st st	nandley 0 / 55424
The second secon	No.		167 550 H		11144	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
1 the	MALL TO				Seepage	Return 180 /5765
	FORDING RIVER	Lo	- to	-	Kilmarn	ock W. <u>Fernand</u>
	SING RIVER	200	24			0
					Kilmarn	ock E. remarcal
ise above map to mark up observations incl wetted	perimeter	0	Upstream D	: Downstream	C: Crest F: Face	T: Toe B: Lower Bench
			opsileoni o	. Domistican		
Are the following components in SATISFACTORY CONDITION?	Were any of the below	Problem		tion (s)		Commonte
	POTENTIAL PROBLEM INDICATORS found?		- 6	m, N.Ab., S.Ab., Rail mb.)		Comments
South Abutment Comment		YN				
Imarnock W Y (1) F/A	Piping			_		
Imarnock E Y N F/m	Sinkholes		<			
asline Y - N	Seepage		×			
arified Line (Y) N	External Erosion		-	0		
arge+anchors 🕑 N eepage Return Line 🛛 (?) N	Cracks Settlement		×	-		
Railway Embankment Comment	Sloughing/Slides		с С			
lackmore Culverts	Animal Activity					
outh Loop Culverts	Excessive growth		K			
West & Main Dam Comment	Excessive Debris				1	
Seep Ø N	Standing Water					
/.Seep V N	Over-steepened Slope		<			
Seep (y) N	Loter steepened slope	1 1				
redge line YN	Comments					
ip Rap (Y) N						
Instruments Comment	· Watt Balence	e 50-	4.0.4	should	be updated	markle
iezometers (Y) N	00000 0000	, 11~	acica		ey e e u	9.
PS Units II N	- Und Suffr	A		- 1.		
ope Inclinometers (Y) N	- Need bette	1000	heniters	systen	e Norup.	
Reservoir Comment	ENL IN	strue				μ. Α
				non har	and not pade	ing similar hast
	(Kisht	1- m	A due	9 C C	7- 1100 6	14.4-2
ailings surface Y N Show		-	Jun	- 10 10	m use on	we
ailings surface Y N Show	the second					
ailings surface Y N Syncor ust Fences Y Q	two scores	to a	He trade	4 nhu	- u liffruit	to get
ailings surface Y N Show	(resent two sursers	to d	We trade	4 nhu	- v difficult	to get.
ailings surface Y N Show	No next au	harry	Q fai.7.	time day	dage or W	VC
ailings surface Y N Show	No nent au	hard	Q fai.).	time day	dage or W	VC
ailings surface Y N Show	No nent au	hard	Q fai.).	time day	dage or W	VC
ailings surface Y N Show	· invisionated it	the ty	e fair. 7. to see in	tin di. Frankl	dage or W	VC

- Deedge stating on N. Ala
- . us slope snow cover
- . S. end at dike wind blow. N. hed has show duties
- locks like animals drink out at N. Segs.

- Pheto of know chand rea NAS (0+100?)
- shadley flavnet neds a buggy whip or it will get hunded in snow.

Fording River Operations: North Tailings Pond Inspection Sheet review

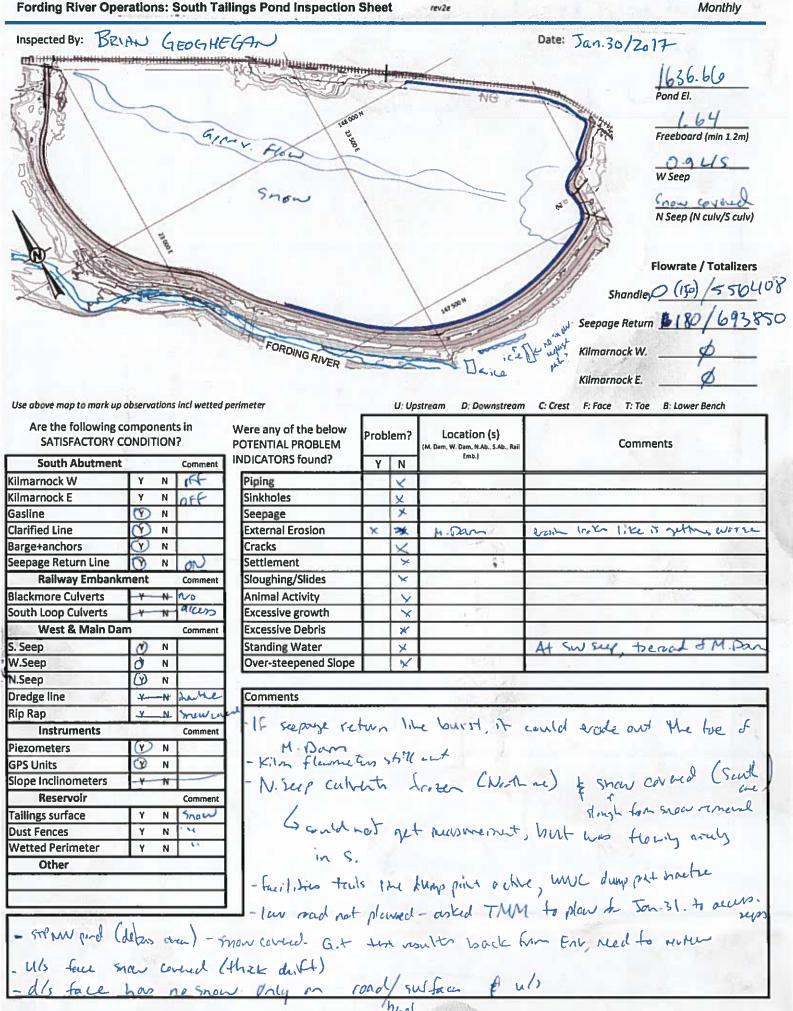


Are	the	follo	wing	compo	onents	i٢
SA	TIS	FACT	TORY	COND	TION?	

Main Embankment		Comment
Rip Rap	Ô N	
Old Tailings Line	V N	
2013 Main Failure Area	🕜 N	
2013 South Failure Area	🕐 N	
Old Scraper Crossing	(*) N	
FRNTP2	·YN	
Clarified Lines River Crossing	🕐 N	
Clarified Lines inside Pond	TYXN	Damaete
Steel Pipe Crossing	Cy N	,
Black PVC Pipe Crossing	9 N	
Barge Infrastructure	O N	
Shandley Bridge	C7 N	
West Abutment		Comment
Facilities Dump	🕑 N	
Pond Coal Stockpile	(Y) N	
Shandley Line	(Y) N	
Reservoir		Comment
North Abutment Area	OV N	
Tailings surface	Ø N	
Dust Fences	Y N	check
Wetted Perimeter	N N	None

Vere any of the below POTENTIAL PROBLEM	Main Embankment		West Abutment		
INDICATORS found?	Y	N	Y	N	Comments
Piping		×		×	32.82
Sinkholes		Y		X	Main Emb. Snow
Seepage		\checkmark		×	conned -
External Erosion		¥		X	Nothing visite tom
Cracks		×		X	Nothing visite tom
Settlement		¥		×	en e i suctore
Sloughing/Slides		×		×	
Animal Activity		*		×	
Excessive growth		×		×	
Excessive Debris		7		*	
Standing Water		×		Y	
Over-steepened Slope		4		Y	

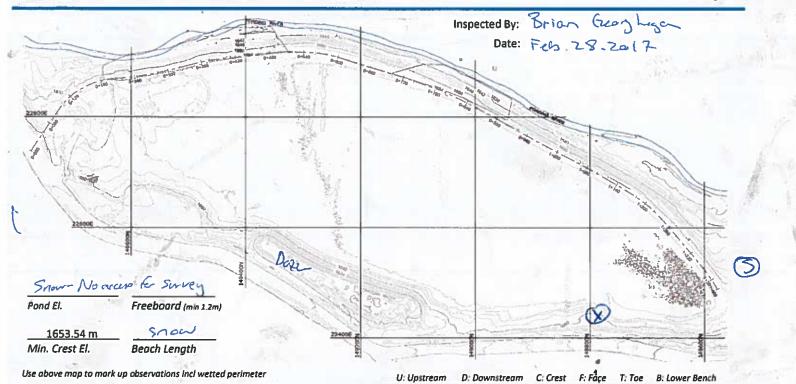
Comments sediment dump- are Env. getting regular scens for volume icioniliation? - Filed where? Rote of deproperty clonified piper during snow removed sung totraining to move removed optimions : cause stadold sates narrow points - EVP dranked to NTV secondary - EIK @ ds for acons for Maxan



- Ice at toe of main dam, alow from a snow-loss section by dd it channel. - 5 sign & supage pathway? - cable stad above Shalley flowmete

Fording River Operations: North Tailings Pond Inspection Sheet

Monthly

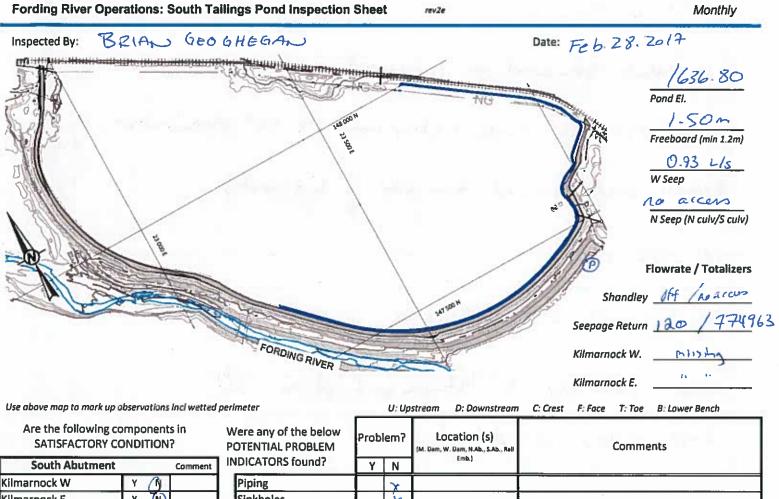


Are the following components in SATISFACTORY CONDITION?

Main Embankment	19 ···	Comment
Rip Rap	Or N	
Old Tailings Line	OP N	
2013 Main Failure Area	🕢 N	
2013 South Failure Area	Q N	
Old Scraper Crossing	🕑 N	
FRNTP2	🖌 N	(
Clarified Lines River Crossing	😥 N	
Clarified Lines inside Pond	1 N	100
Steel Pipe Crossing	K N	2
Black PVC Pipe Crossing	X N	(No LUM
Barge Infrastructure	Q/N	
Shandley Bridge	W N	
West Abutment		Comment
Facilities Dump	O N	
Pond Coal Stockpile	ON N	Dezer
Shandley Line	Y (N)	Bata
Reservoir		Comment
North Abutment Area	Ø N	
Tailings surface	ON N	
Dust Fences	Y - N	
Wetted Perimeter	Y - N	

Were any of the below POTENTIAL PROBLEM	Ma Embani		We: Abutn		
NDICATORS found?	Y	N	Y	N	Comments
Piping		Y	10000	X	
Sinkholes	1	V		X	
Seepage		X		χ	
External Erosion		×		X	and the second se
Cracks		×	6	X	Slan forad
Settlement		×		X	Small
Sloughing/Slides		X		X	significant snow Cereoge
Animal Activity		X		X	Cereage
Excessive growth		X	8	X	
Excessive Debris		X	Ê	X	
Standing Water	10235	×		X	
Over-steepened Slope		×		X	

Comments - Deep puddle at B. Traul/access concern - Break in shandly like @ (D. Too for from NTP to be concern, only snow affected. - No access to dike out from S. side ble no snow remail Couldn't extrave window from snow remarch to Shandley Levents 「金融の Dozer openating on pond coal stockpile Assuming this is part of smith/ Snow remarel for linenew accurs to new powerhus. We authority evident @ Sect. pond. -Lees lake system flowing to rive. Law like Ail a p-d @ hind d NTP. - No accurs to NTP the for survey well reading, but picture taken to wincely



South Abutment		Comment	
Kilmarnock W	Y		P
Kilmarnock E	Y 🕥		S S D O S
Gasline) N		S
Clarified Line	DY N		E
Barge+anchors	(Y) N		C
Seepage Return Line	N N		S
Railway Embankn	nent	Comment	S A E
Blackmore Culverts	¥N		A
South Loop Culverts	¥N		E
West & Main Dan	ו	Comment	
S. Seep	O N		S
W.Seep	(†) N		0
N.Seep	¥ N		
Dredge line	YN		C
Rip Rap	(Y) N		ΙГ
Instruments		Comment	
instruments		comment	
Piezometers	Y N	Comment	
	Y N Y N	Comment	
Piezometers			
Piezometers GPS Units	Y-N	Comment	
Piezometers GPS Units Slope Inclinometers	Y-N		
Piezometers GPS Units Slope Inclinometers Reservoir	Y N Y N		
Piezometers GPS Units Slope Inclinometers Reservoir Tailings surface	Y N Y N		
Piezometers GPS Units Slope Inclinometers Reservoir Tailings surface Dust Fences	Y N Y N		
Piezometers GPS Units Slope Inclinometers Reservoir Tailings surface Dust Fences Wetted Perimeter	Y N Y N		
Piezometers GPS Units Slope Inclinometers Reservoir Tailings surface Dust Fences Wetted Perimeter	Y N Y N		
Piezometers GPS Units Slope Inclinometers Reservoir Tailings surface Dust Fences Wetted Perimeter Other	Y N Y N Y N Y N	Comment	
Piezometers GPS Units Slope Inclinometers Reservoir Tailings surface Dust Fences Wetted Perimeter	Y N Y N Y N Y N	Comment	

ere any of the below DTENTIAL PROBLEM	Problem?		Location (s) (M. Dam, W. Dam, N.Ab., S.Ab., Rell	Comments				
DICATORS found?	Y	N	Emb.)					
Piping		x						
Sinkholes		x						
Seepage	Γ	X						
External Erosion		X		-				
Cracks		K						
Settlement		X						
Sloughing/Slides		X						
Animal Activity	1	X						
Excessive growth		X						
Excessive Debris		X						
Standing Water		X						
Over-steepened Slope	1	X						

C	omments
	should pipeline leak near NTP. Line off, snow removal to line. damage likely from previous snow removal (is sump onew must be) aware
	damage likely from previous mow remained (is hump new must be)
	Greed pumpour frame @ monthly map so can report on this type
	it activity with the out some decovering in field
	esp since water ment when lend be when it environmental
ł	elve-Sodart where env keeps then flowmete vicarde, vinge to ave
venit	ets prist to he fin lay to northly updates
tr	passible starting and - ned survey pretup. See pics.

should compare it doce vations from cumper

N. Leg Calverts snow aveil - no measurement

Slepage return well = aly makeup water to STP at the moment, othe the maxim

100

Nobels dudge remained from site (last week)

als snow creed.

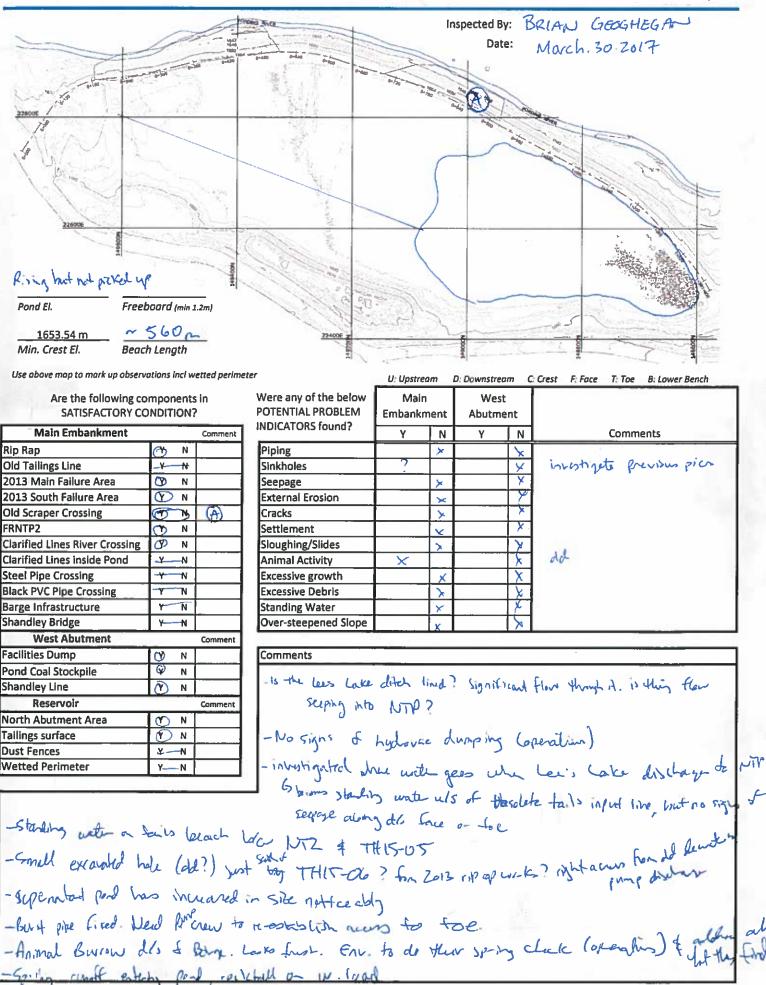
. 1

Bench road clear / dubeable

New datalogge & Antena @f piero @

Good photo: I dls face

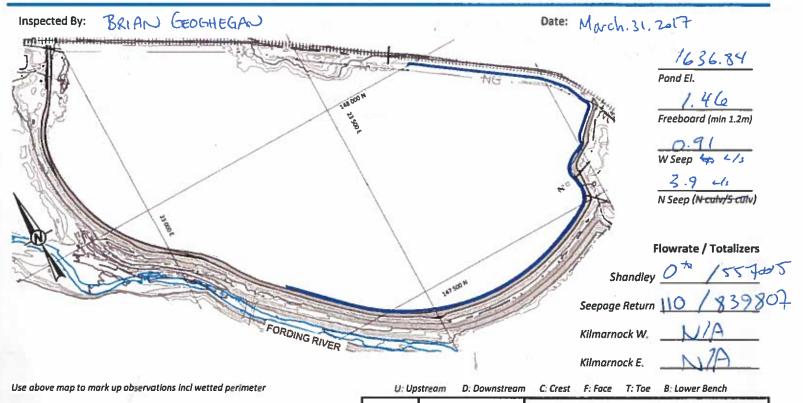
Fording River Operations: North Tailings Pond Inspection Sheet



Follow-up items from previous inspection:

Old Snow drupp (MDG laydown) inactive - drupping along have pead has pranted ag accus to spendim (would need doze he wit in the new accen) No shiped besome by desolute culture avois for MS Pade - - is it Elke Trada?

-Environmental richting & separe thank back & have road by Multiplate (whent NTP)



rev2e

Are the following components in SATISFACTORY CONDITION?

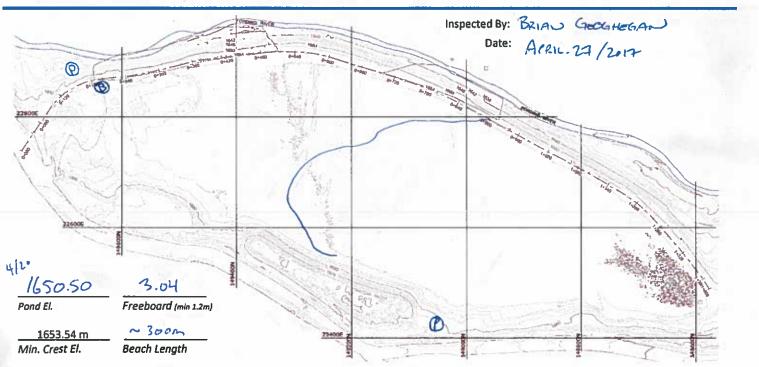
South Abutment			Comment
Kilmarnock W	Y	N	i se
Kilmarnock E	Y	Ν	OFE
Gasline	Y	Ν	
Clarified Line	O	N	
Barge+anchors	P	N	
Seepage Return Line	Θ	N	
Railway Embankn	nent		Comment
Blackmore Culverts	Ø	N	
South Loop Culverts		-N-	
West & Main Dan	n		Comment
S. Seep	đ	Ν	
W.Seep	9	Ν	
N.Seep	Ø	N	
Dredge line	Y	Ν	off
Rip Rap	Ø	Ν	
Instruments			Comment
Piezometers	\mathfrak{G}	N	
GPS Units	Ø	Ν	
Slope Inclinometers	Y	N	
Reservoir			Comment
Tailings surface	ß	N	
	¥-	N	
Dust Fences			
Dust Fences Wetted Perimeter	\odot	Ν	

Were any of the below Location (s) Problem? POTENTIAL PROBLEM Comments (M. Dem, W. Dem, N.Ab., S.Ab., Rell Emb.) **INDICATORS found?** Y Ν Piping × Sinkholes × Seepage ۶ **External Erosion** × Cracks ٢ ۲ Settlement Sloughing/Slides Animal Activity **Excessive growth** $\boldsymbol{\checkmark}$ **Excessive Debris** × ¥ Standing Water X **Over-steepened Slope**

Commente
Comments
reshardley FM - shows 150 wypr ever aboy's reacted flew
This moisid a DE along the Dike this morning
Etails Ben i destructing unich dischare from Traveling tourides Base 6 PS decembre for feedback. No fly identified/needed
6 PS scentre for feedback. No fly identified needed
- Significant Flow through blackmare Cultration (unto bolow S de Flow reconversents there as well? (un operation
"do flow reassurements there as well? (we operation
- Looked at N Dach see it an sign of interved flow from NLP
decart prime principality & earling the of STP it Heavy and age for (NO evidence of earling - light flows.
- Cha up inch and
used (by St. well) - Black - and and and a culvets of /flow
4? put up cress thiss

- Pail Lose draining to site - Miner erogit above N-Ditch - Pieto stadpine height address - M. Dan erost Filling direbut a bit

Fording River Operations: North Tailings Pond Inspection Sheet rev2c



Use above map to mark up observations incl wetted perimeter

Are the following components in SATISFACTORY CONDITION?

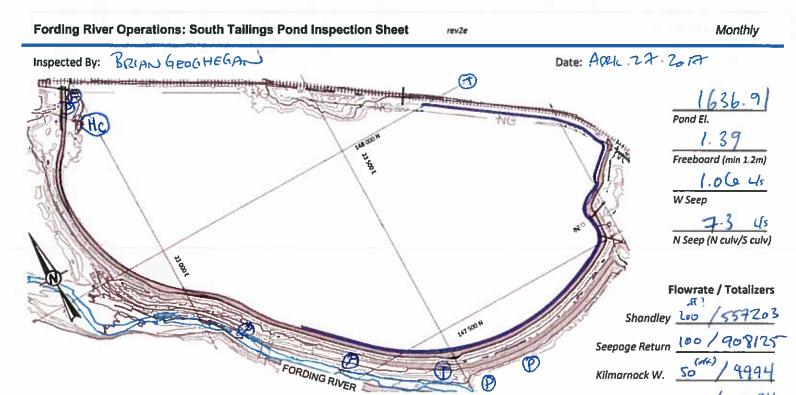
Main Embankment	_	Comment
Rip Rap	D N	
Old Tailings Line	YN	
2013 Main Failure Area	CP N	1
2013 South Failure Area	O N	
Old Scraper Crossing	YN	1
FRNTP2	O N	
Clarified Lines River Crossing	YN	
Clarified Lines inside Pond	N (Y)	
Steel Pipe Crossing	Y N	
Black PVC Pipe Crossing	-Y N	1
Barge Infrastructure	N	
Shandley Bridge	O N	
West Abutment		Comment
Facilities Dump	OS N	
Pond Coal Stockpile	Ø N	2
Shandley Line	N (
Reservoir		Comment
North Abutment Area	C) N	
Tailings surface	O∕ N	
Dust Fences	🕑 N	
Wetted Perimeter	Y N	1

U: Upstream D: Downstream C: Crest F: Face T: Toe B: Lower Bench Were any of the below Main West POTENTIAL PROBLEM Embankment Abutment INDICATORS found? Y. Comments Ν N Y Piping × Sinkholes Ý × Seepage x × X **External Erosion** 4 Cracks × X Settlement 7 × ¥ Sloughing/Slides × × EK **Animal Activity** ¥ **Excessive growth** ۴ 4 ٧ **Excessive Debris** 4 ¥ Standing Water Over-steepened Slope

Comments

- Lees Lake decarry through who channel Ponding weter in some locations (snow mult) (P) above Showed add Solety at top I accure ramp (B) above
- · Took representative. Shots I comple very minar early/slows

fratures.



Use above map to mark up observations Incl wetted perimeter

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Are the following components in SATISFACTORY CONDITION?

South Abutment

Railway Embankment

West & Main Dam

Kilmarnock W

Kilmarnock E

Clarified Line

Barge+anchors

Seepage Return Line

Blackmore Culverts

South Loop Culverts

Gasline

S. Seep

W.Seep

F: Face C: Crest T: Toe U: Upstream D: Downstream B: Lower Bench

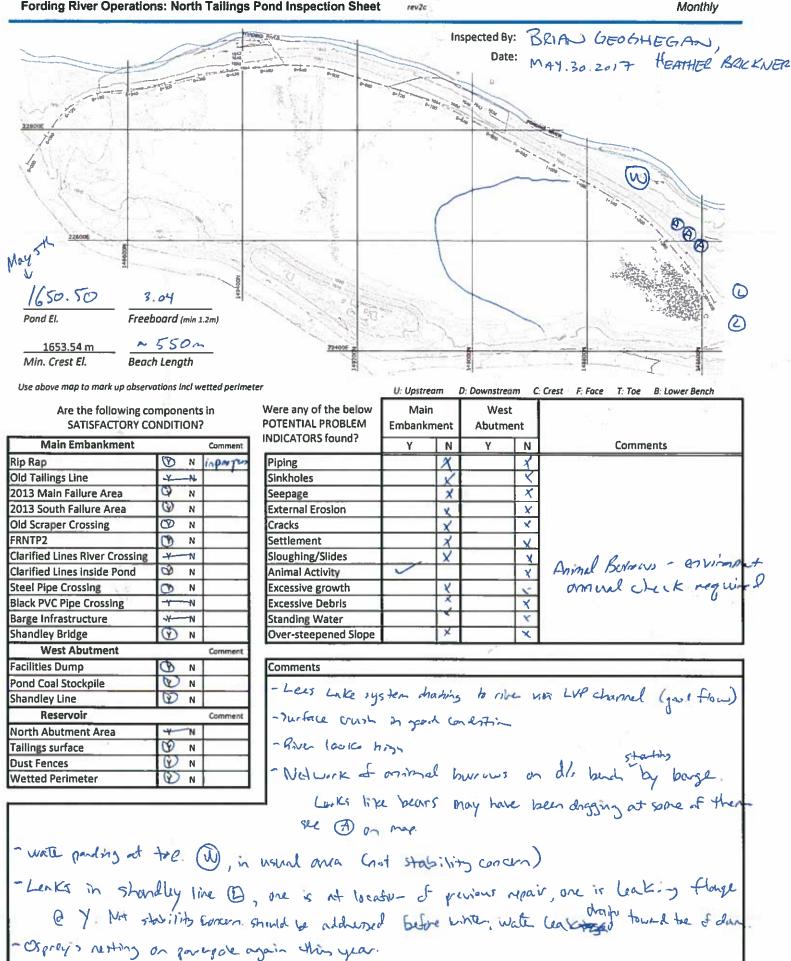
Kilmarnock E.

0

s in ?	Were any of the below POTENTIAL PROBLEM	Problem?		Location (s) (M. Dem, W. Dem, N.Ab., S.Ab., Rell Emb.)	Comments				
Comment	INDICATORS found?	Y	N	CmD.J					
	Piping		X						
	Sinkholes		×						
	Seepage		X						
	External Erosion		Å						
	Cracks		\times						
	Settlement		\times						
Comment	Sloughing/Slides		×						
	Animal Activity		×						
	Excessive growth		+						
Comment	Excessive Debris		4						
	Standing Water	1	7						
	Over-steepened Slope		7						

N.Seep	V) N		
Dredge line	- Y -N		Comments
Rip Rap	Ô N		-Turapue working on booster pumps - (), The put up on dike (that
Instruments		Comment	
Piezometers	Y N		- Flavmeters installed, Kill E andered under PO 478345
GPS Units	🖓 N		
Slope Inclinometers	¥N		- lots of Froth on pond
Reservoir		Comment	- a la to la
Tailings surface	(Ŷ) N		- Ponling wate (P)
Dust Fences	() N		- "Niseep" flowing well, visibly dry chu to maxam - comby up at
Wetted Perimeter	Y N		The set of the set of the country of as
Other			usual spot.
			-Unusual discharge @ facilities weath F
- Hydrocar barr	Visible	in poro	I sy wink's (A). Sweect runoff for lad rate than bakage for WUC
- Bedrspaw Vaci			
- Surface star	the to	reed c	with grading.

Fording River Operations: North Tailings Pond Inspection Sheet



- should invertigate locate survey instrument sature (vertig)

- Lock like new matrial has been placed at sedanat dump

Forung River Op	si duloi	1121.0	Souur Te	allings Pond Inspection	Snee		revze			мопіпіу
Inspected By: Bain	m G	neol	GHE GA	, HEATHER BA	RICK	NE.	2	Date:	MAY.30	1.2017
CONTRACTOR OF STREET	taitteti	in Uh		TEM HER CO		- 01	/			4
	-	_		C.C.	***					1636-82
516		-		- Charles	1000		TVER	TATI DE LA COMPANY		Pond El.
00000					M			1	that have	1.48
Bell 3 \		1		LABON	-				TRA	
Sel ~ \		1		11.40	5				115	Freeboard (min 1.2m)
/ III									L'A	Wseep Wseep
See.				\times				1	2.5	W Seep
N.M.	1		/		1				CAY .	-60 Us
11118 1		/	/		/			Pre-	R.	N Seep (N culv/S culv
1111/1 1	X	-			/			/	1/AC	P to be checked
All and	\leq	100			1	1			115	r to be checked
The second		4					/		1	Flowrate / Totalizer
S W						1	1		r Sha	indley (150) / 5570
		-			-	-	141 500 M	11-		and the second sec
			1 1	March -		-		2	Seepage F	Return 96641
				FORDING RIVER	-		- Ster		Kilmarnoc	kw. \$ /21251
				Sect 2	-				Kilmarnoo	KE. \$ / 3058
Use above map to mark up	observe	ations	incl wetted	perimeter		U: Un	stream D: Downstream	C: Crest		T: Toe B: Lower Bench
Are the following										
SATISFACTORY	-			Were any of the below POTENTIAL PROBLEM	Probl	lem?	Location (s)		G	omments
				INDICATORS found?			(M. Dam, W. Dam, N.Ab., S.Ab., Rail Emb.)			ommenta
South Abutment ilmarnock W	r (B	N	Comment		Υ ·	N				
ilmarnock w ilmarnock E	G	N		Piping Sinkholes	+	X				
iasline	00	N		Seepage	+	$\overline{\mathbf{x}}$				
larified Line	Q	N		External Erosion	+	रि				·····
arge+anchors	Ø,	N		Cracks		1				
eepage Return Line	- G	N		Settlement		$\overline{}$				
Railway Embank	ment		Comment	Sloughing/Slides		$\overline{\mathcal{A}}$				
Blackmore Culverts	Ø	N		Animal Activity		X				
outh Loop Culverts	Y	N		Excessive growth		$\overline{\checkmark}$				147.3
West & Main Da	im		Comment	Excessive Debris		X				
. Seep	3	N		Standing Water		$\overline{\mathbf{x}}$				
V.Seep	Ø	N		Over-steepened Slope		×				
I.Seep	\odot	N								
Dredge line		14	(alet	Comments						
lip Rap	Ð	N					AL C. L	0		
Instruments			Comment	Tailingo discha	re pe	ntre	my submand	K.		
liezometers	C#	N		High Florest		0.0		he are	e it	E STP
PS Units	Ŷ	N		- High Flow in	1100	100	overbasing	nonk	scuth	A DIA
lope Inclinometers	Ø	N		- Mustal - 1		-				(11 C 410 711)
Reservoir			Comment	vigetation clu	mpine	50	the indication to	lybe s	eepage	2 ? (als & STP. 242 by SP-ws
ailings surface	φ	N		Guit	1.0		1			by sp-ws
)ust Fences	Ŷ	N		r come (rach h	n t	prol sloughs.	73 5		
Vetted Perimeter	4	N ⁻		UVV+ (cm				-		1 1. 18
Öther				cuiverts be	unth	ra	ilway hurized	Q boto	the state of a	hedge line vorsily?
	00000	8					· · · · · · · · · · · · · · · · · · ·			
				- Lote & Froll						
- No nondrin y	rate		المحط	& M.Dam, still	(rad)	C L	at a buch .	f w.	Dan	
ion lance of a		C		A COMPANY FILL	8	2	4			
- N Selp Heavy	fl,	w ₅	flow	status much for	the	u	p then would	-		
J					-1	-	•			

Follow-up items from previous inspection:

Teck

Inspected By:	Heather Buckner
Inspection Date:	June 27-28, 2017
Weather & Temperature:	sunny ~23°C

ACTION ITEMS

Record any items of concern noted during the inspection; location of each action item shall be marked on the attached facility maps. If required, additional items can be included in the "Additional Comments" section.

Priority	Description
1	A high probability or actual dam safety issue considered immediately dangerous to life, health or the environment, or a significant regulatory concern.
2	If not corrected, could likely result in dam safety issues leading to injury, environmental impact or significant regulatory action; or, a repetitive deficiency that demonstrates a systematic breakdown of procedures.
3	Single occurrences of deficiencies or non-conformances that alone would not be expected to result in dam safety issues.
4	Best Management Practice as a suggestion for continuous improvement towards industry best practices that could further reduce potential risks. This typically includes ongoing construction items within the appropriate construction cycle.

Location	Item Description & Responsibility	Priority	Target Completion Date
STP tailings discharge	Water from STP flowing into the dam near FortisBC garline location. See Note 4	2	
All	Flowmeter locations is unsure where there are located, map would be useful.	na	Locate by next insp.
STP	Implement Weekly inspections from May-Sept.	4	start week of July 3 zoi7.
STP/ NLP	Request access to daily tailings line inspections (Processing Supervisors)	4	by next monthly insp. Notes

INSPECTION ITEMS

The dam crests, downstream slope, upstream slope, abutments, pipelines, instruments, culverts and other relevant structures shall be inspected. This form does not provide a complete list of potential issues. Details around observations and any other concerns should be noted in the "Additional Comments" section.

Photographs of all structures are required as part of the monthly inspection. The following brief description of areas to inspect. During the inspection, checking "Condition Acceptable" indicates that the relevant items in this section have been reviewed for each structure, and were found to be acceptable.

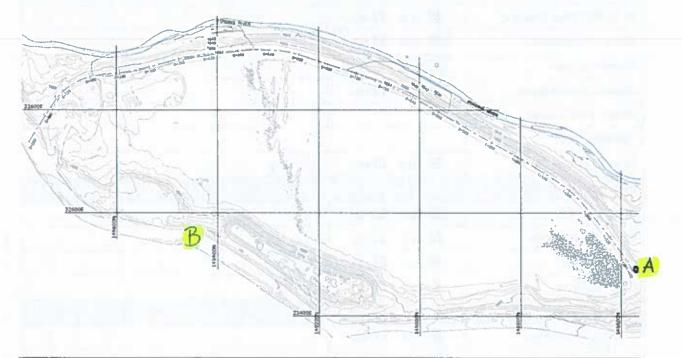
Item Description	Potential Problems
Piping	Uncontrolled internal erosion of the embankment, resulting in rapid failure of the dam
	Any signs of piping should be reported immediately, as this is a serious risk to the structural integrity of the dam
Seepage	Increase in volume of flow
	Turbidity or sediment in the seepage indicates a potential piping issue (internal erosion)
	Seepage boils – check for sediment transport and flow rates. An increase in the number of boils indicates poor drainage conditions through the embankment
Sinkholes	Generally occurring on the dam crest or slopes, can indicate internal settlement of the embankment
External Erosion	Around, overtop or at the toe of the embankment
	Heavy rainfall events typically cause substantial erosion on the downstream
	slopes of earthen/tailings embankments
Cracks	Longitudinal (along the length of the dam)
	Transverse (across the dam)
	Cracks should be marked with paint/stakes along the length and survey pins on
	either side of the crack. These should be surveyed and monitored for
Settlement	Excessive settlement
	Survey pins should be installed in any new settled areas so that settlement can be tracked over time.
Sloughing / Slides	Heavy rainfall events can cause sloughing of material
Animal Activity	Burrowing vectors can potentially damage the dam
	Large animal activity (bears, elk, etc.) can pose a safety concern to those working in the area
Excessive growth	Vegetation can hide potential problems on the crest, slopes and toes of the
	dams. Excessive root infiltration due to vegetation also has the potential of
	damaging the structural integrity of the dam.
General housekeeping	Excessive debris / waste materials
	Dusting concerns from the tailings pond or embankment materials
	Access to the dams (due to snow, poor road conditions, etc.)
Standing water	On the crest or at the toe of the dam. Excessive water can cause increases in
	the saturation of the embankment fills, reducing the soil strength
Over-steepened slope	Steep slopes can lead to sloughing of material and general dam stability issues
Pipeline conditions	Leaks or holes / tears in the line
	Damage to the pipelines from passing equipment / snow removal
	Pipeline leaks on dam crests or slopes will begin to erode the embankment
ORTH TAILINGS PON	material very quickly, with the potential to develop into a dam breach scenario

NORTH TAILINGS POND

Minimum crest elevation = 1653.54 m (mine grid UTM)

NORTH TAILINGS POND

- Minimum crest elevation = 1653.54 m (mine grid UTM)
- Minimum Freeboard = 1.2 m
- Mark any issues on the map below, including tailings wetted perimeter



Structure	Condition Acceptable	Comments
North Tailings Pond – Main	Embankment	
Access	YES NO	2
Downstream Toe	YES NO	
Downstream Slope	YES INO	
Dam Crest	YES INO	
East Abutment	YES INO	
West Abutment	YES NO	
Rip Rap		
Old Tailings Line	YES INO	
2013 Main Failure Area	YES INO	
2013 South Failure Area	YES INO	
Old Scraper Crossing	YES INO	
FRNTP2		

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CI. Lines River Crossing	YES NO	
CI. Lines inside Pond	YES INO	
Steep Pipe Crossing	YES NO	
Black PVC Pipe Crossing	YES D NO	
Barge Infrastructure	YES NO	
Shandley Bridge		
General Housekeeping	YES INO	
Water Level Elevation		
Freeboard (m)	,	
Other Concerns	YES D NO	none
West Abutment Area		
Facilities Dump	YES DINO	
Pond Coal Stockpile	YES NO	
Shandley Line	YES DINO	
Other Concerns	YES 🗖 NO	none
Reservoir Area		
North Abutment Area	YES NO	
Tailings Surface	YES DINO	
Dust Fences	YES INO	
Tailings Wetted Perimeter	YES D NO	
Other Concerns		hone
Additional On-going Monit	oring (add as required	

SOUTH TAILINGS POND

- Minimum crest elevation = 1638.30 masl (mine grid UTM)
- Minimum Freeboard = 1.2 m
- Mark any issues on the map below, including tailings wetted perimeter

Por Ding River

Structure	Condition Acceptable	Comments
South Tailings Pond – Ma	ain Embankment	
Access		
Downstream Toe		
Downstream Slope	YES INO	3
Dam Crest	YES NO	
South Abutment		
North Abutment	YES INO	
South Abutment Area		
Kilmamock W	E YES NO	
Kilmarnock E		
Fortis Gasline		4
Clarified Line		

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Barge & anchors	YES		
Seepage Return Line	VES	D NO	
General Housekeeping	VES	D NO	
Water Level Elevation	1636.	90 m	from Trend.
Freeboard (m)	1.40)m	
Other Concerns	VES	NO	tailings discharge area,
Railway Embankment Area	-		
Blackmore Culverts	VES VES		
South Loop Culverts	VES	D NO	
West & Main Dam Area	VES	D NO	
South Seep	V YES		
West Seep	VES YES		· · · · · · · · · · · · · · · · · · ·
North Seep	VES YES		
Dredge Line	🗹 YES		
Rip Rap	VES		
Instrumentation			
Piezometers	VES YES	🗖 NO	
GPS Units	VES		
Slope Inclinometers	VES		
Reservoir Area			
Tallings Surface	YES		
Duct Fences ha		E -NO ⁻	
Tailings Wetted Perimeter	VES		
Flowrates & Totalizers			
Shandley Flowrate			
Seepage Return Flowrate			
Kilmarnock West			
Kilmarnock East			
Additional On-going Monit	oring (add a	s required)

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Teck

ADDITIONAL COMMENTS:

1) Leak at location "A" on shandley line has been repaired. Access wad on west side (approx. lo cation B) requires repair, lots of evidence of ension along side of wad. 3) Ension gully (approximate location C) was checked, no concerns noted At the failings discharge on June 26, Environment personnel reported that water was flowing pack underreath the embankment hear the FortisBC garline (where it goes beneath the embankment). Water levels in the tailing pond Were lower on June 27, this way not observed. is likely the water way using the gastine ar through the dam. a "condui 15 recommended Check the area but it the agrine some remediative measures wit * next & implemented investigated Checked the Henretta BS TSF and culvert crassing he issuer were noted. Water Playing well Henretta Seemed a

Page 11 of 12 Rev 0 (2017-05-16) * Note 4 (contd). Notified Robin & Andrew Bidwell. Discussed with Julia Steele (Golder). Turns out the STP North Abutment area was not completed as part of 2012 days raise, underlying material is sandy-gravel covered by coarse rejects fill. Julia will be on site for an inspection July 3.

6) Turnbull South Pit TSF + Henretta Culvert: - no issuer noted Will continue to check there areas during the monthly dam inspection.

7) Flow meters - locations of flow meters up not Known, will need to ask (eg. Brian or Erika) where these are located.

8) Talked to Andrea Murland, she said she Would be able to put me on the email dist for claily supervisor inspections (which includes tailings line and discharge).

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Inspected By:	Heathe	r Brickner	_
Inspection Date:	July7	2017	_
Weather & Temperature:	Sunny	1~27°C	_

ACTION ITEMS

Record any items of concern noted during the inspection; location of each action item shall be marked on the attached facility maps. If required, additional items can be included in the "Additional Comments" section.

Priority	Description
1	A high probability or actual dam safety issue considered immediately dangerous to life, health or the environment, or a significant regulatory concern.
2	If not corrected, could likely result in dam safety issues leading to injury, environmental impact or significant regulatory action; or, a repetitive deficiency that demonstrates a systematic breakdown of procedures.
3	Single occurrences of deficiencies or non-conformances that alone would not be expected to result in dam safety issues.
4	Best Management Practice as a suggestion for continuous improvement towards industry best practices that could further reduce potential risks. This typically includes ongoing construction items within the appropriate construction cycle.

Location	Item Description & Responsibility	Priority	Target Completion Date
STP South end, DS slope	Animal burrows at point "A" on map. Continue to monitor, email Environment personnel	4	emailed Environment on July 7/17.

INSPECTION ITEMS

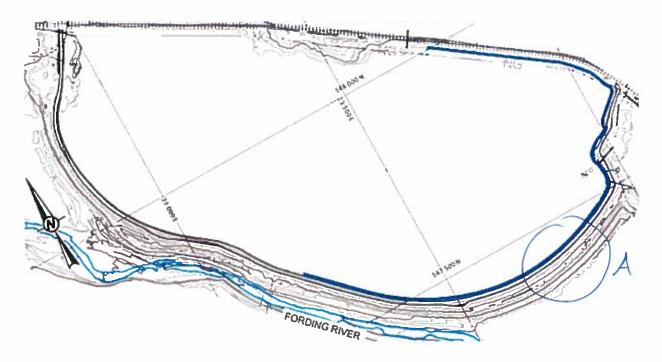
The dam crests, downstream slope, upstream slope, abutments, pipelines, instruments, culverts and other relevant structures shall be inspected. This form does not provide a complete list of potential issues. Details around observations and any other concerns should be noted in the "Additional Comments" section.

Photographs of all structures are required as part of the weekly inspection. The following brief description of areas to inspect. During the inspection, checking "Condition Acceptable" indicates that the relevant items in this section have been reviewed for each structure, and were found to be acceptable.

Item Description	Potential Problems
Piping	Uncontrolled internal erosion of the embankment, resulting in rapid failure of the dam
	Any signs of piping should be reported immediately, as this is a serious risk to the structural integrity of the dam
Seepage	Increase in volume of flow Turbidity or sediment in the seepage indicates a potential piping issue (internal
	erosion)
	Seepage boils – check for sediment transport and flow rates. An increase in the number of boils indicates poor drainage conditions through the embankment
Sinkholes	Generally occurring on the dam crest or slopes, can indicate internal settlement of the embankment
External Erosion	Around, overtop or at the toe of the embankment
	Heavy rainfall events typically cause substantial erosion on the downstream slopes of earthen/tailings embankments
Cracks	Longitudinal (along the length of the dam)
	Transverse (across the dam)
	Cracks should be marked with paint/stakes along the length and survey pins on either side of the crack. These should be surveyed and monitored for
Settlement	Excessive settlement
	Survey pins should be installed in any new settled areas so that settlement can
Olevelie / Olidee	be tracked over time.
Sloughing / Slides	Heavy rainfall events can cause sloughing of material
Animal Activity	Burrowing vectors can potentially damage the dam Large animal activity (bears, elk, etc.) can pose a safety concern to those
	working in the area
Excessive growth	Vegetation can hide potential problems on the crest, slopes and toes of the
	dams. Excessive root infiltration due to vegetation also has the potential of
	damaging the structural integrity of the dam.
General housekeeping	Excessive debris / waste materials
	Dusting concerns from the tailings pond or embankment materials
	Access to the dams (due to snow, poor road conditions, etc.)
Standing water	On the crest or at the toe of the dam. Excessive water can cause increases in
^	the saturation of the embankment fills, reducing the soil strength
Over-steepened slope	Steep slopes can lead to sloughing of material and general dam stability issues
Pipeline conditions	Leaks or holes / tears in the line
	Damage to the pipelines from passing equipment / snow removal
	Pipeline leaks on dam crests or slopes will begin to erode the embankment
	material very quickly, with the potential to develop into a dam breach scenario

SOUTH TAILINGS POND

- Minimum crest elevation = 1638.30 masl (mine grid UTM)
- Minimum Freeboard = 1.2 m
- Mark any issues on the map below, including tailings wetted perimeter



Structure	Condition Acceptable	Comments
South Tailings Pond – Ma	ain Embankment	
Access		
Downstream Toe		
Downstream Slope		see Note 7,8
Dam Crest		
South Abutment		
North Abutment	YES D NO	
South Abutment Area		
Kilmamock W	YES D NO	not running
Kilmamock E		not running
Fortis Gasline	YES D NO	ſ
Clarified Line	VES DINO	

Barge & anchors	V YES		
Seepage Return Line	VES		
General Housekeeping	VES		
Water Level Elevation	1636.5	84m	
Freeboard (m)	1.46	m.	
Other Concerns	C YES		
Railway Embankment Area			
Blackmore Culverts	TYES		
South Loop Culverts	🖬 YES		
West & Main Dam Area	🗹 YES		
South Seep	🗹 YES		
West Seep	🗹 YES		
North Seep	🗳 YES		
Dredge Line	YES		
Rip Rap	VES		
Instrumentation		Starten 1	
Piezometers	VES		
GPS Units	U YES		
Slope Inclinometers	VES YES		
Reservoir Area			
Tailings Surface	⊠ ∕YES		
-Dust Fences	🗖 YES		na
Tailings Wetted Perimeter	YES		
Flowrates & Totalizers		1	
Shandley Flowrate			
Seepage Return Flowrate			ha not running
Kilmarnock West			
Kilmarnock East			
Additional On-going Monito	oring (add a	s required	
Tails discharge			no concerns noted.
Tails discharge area			no concerns noted. More details on pg.5
			10

ADDITIONAL COMMENTS:

Bern just west of STP tailings discharge was filled in with till from near wastewater cells. Tailings water had been very close to flowing onto the wood 2) Sand (For sandbage) stockpiled near tailings discharge, ready to be put in sandbage Sandbag berm 3) North Abutment seep area checked, ho signed of water settling or ension on road surface and maxamsite. Water level in STP tailings discharge is lower than yesterday Hot weather evaporation is helping remove water, plant is also running down Dredal is shill Shandley and -Pit-line into STP are not hunning ots of animal tracks in tailings beach (near north end). See photos. Noticed some animal but rows near "A", close to 3 seepage collection well. Continue to monitor Ehvironment personnel. 8) No changer hoted on ension gully at south end (near "A"). Page 5 of 5 2017-07-03

Inspected By:	Heather Brickner
Inspection Date:	July 14, 2017
Weather & Temperature:	sunny 26°C

ACTION ITEMS

Record any items of concern noted during the inspection; location of each action item shall be marked on the attached facility maps. If required, additional items can be included in the "Additional Comments" section.

Priority	Description	
1	A high probability or actual dam safety issue considered immediately dangerous to life, health or the environment, or a significant regulatory concern.	
2	If not corrected, could likely result in dam safety issues leading to injury, environmental impact or significant regulatory action; or, a repetitive deficiency that demonstrates a systematic breakdown of procedures.	
3	Single occurrences of deficiencies or non-conformances that alone would not be expected to result in dam safety issues.	
4	Best Management Practice as a suggestion for continuous improvement towards industry best practices that could further reduce potential risks. This typically includes ongoing construction items within the appropriate construction cycle.	

Location	Item Description & Responsibility	Priority	Target Completion Date
STP downstream toe (south end)	Continue to monitor animal burrows at STP DS toe (south end). Sparwood Environment personnel to assess.	4	Monitor Weekly Monthly.

INSPECTION ITEMS

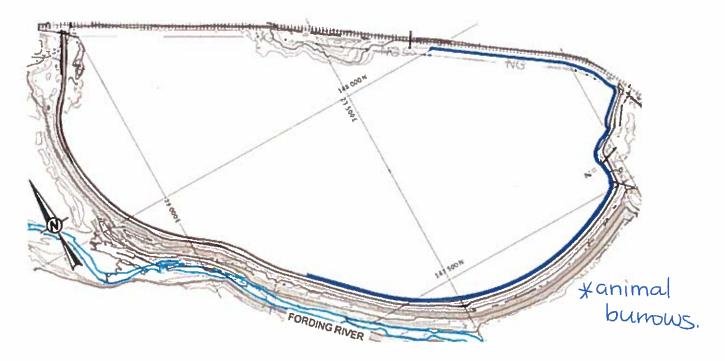
The dam crests, downstream slope, upstream slope, abutments, pipelines, instruments, culverts and other relevant structures shall be inspected. This form does not provide a complete list of potential issues. Details around observations and any other concerns should be noted in the "Additional Comments" section.

Photographs of all structures are required as part of the weekly inspection. The following brief description of areas to inspect. During the inspection, checking "Condition Acceptable" indicates that the relevant items in this section have been reviewed for each structure, and were found to be acceptable.

Item Description	Potential Problems			
Piping	Uncontrolled internal erosion of the embankment, resulting in rapid failure of the dam			
	Any signs of piping should be reported immediately, as this is a serious risk to the structural integrity of the dam			
Seepage	Increase in volume of flow			
	Turbidity or sediment in the seepage indicates a potential piping issue (internal erosion)			
	Seepage boils – check for sediment transport and flow rates. An increase in the number of boils indicates poor drainage conditions through the embankment			
Sinkholes	Generally occurring on the dam crest or slopes, can indicate internal settlement of the embankment			
External Erosion	Around, overtop or at the toe of the embankment			
	Heavy rainfall events typically cause substantial erosion on the downstream			
	slopes of earthen/tailings embankments			
Cracks	Longitudinal (along the length of the dam)			
	Transverse (across the dam)			
	Cracks should be marked with paint/stakes along the length and survey pins on			
	either side of the crack. These should be surveyed and monitored for			
Settlement	Excessive settlement			
	Survey pins should be installed in any new settled areas so that settlement can			
	be tracked over time.			
Sloughing / Slides	Heavy rainfall events can cause sloughing of material			
Animal Activity	Burrowing vectors can potentially damage the dam			
	Large animal activity (bears, elk, etc.) can pose a safety concern to those working in the area			
Excessive growth	Vegetation can hide potential problems on the crest, slopes and toes of the			
-	dams. Excessive root infiltration due to vegetation also has the potential of			
	damaging the structural integrity of the dam.			
General housekeeping	Excessive debris / waste materials			
	Dusting concerns from the tailings pond or embankment materials			
	Access to the dams (due to snow, poor road conditions, etc.)			
Standing water	On the crest or at the toe of the dam. Excessive water can cause increases in			
	the saturation of the embankment fills, reducing the soil strength			
Over-steepened slope	Steep slopes can lead to sloughing of material and general dam stability issues			
Pipeline conditions	Leaks or holes / tears in the line			
	Damage to the pipelines from passing equipment / snow removal			
	Pipeline leaks on dam crests or slopes will begin to erode the embankment			
	material very quickly, with the potential to develop into a dam breach scenario			

SOUTH TAILINGS POND

- Minimum crest elevation = 1638.30 masl (mine grid UTM)
- Minimum Freeboard = 1.2 m
- Mark any issues on the map below, including tailings wetted perimeter



Structure	Condition Acceptable	Comments
South Tailings Pond – Ma	ain Embankment	
Access		
Downstream Toe		₩3
Downstream Slope	YES D NO	
Dam Crest	YES D NO	
South Abutment	VES DINO	
North Abutment	🗹 YES 🗖 NO	4
South Abutment Area		
Kilmamock W	YES D NO	
Kilmamock E	YES D NO	
Fortis Gasline	🗹 YES 🗖 NO	
Clarified Line	YES DINO	

Barge & anchors	🗹 YES		
Seepage Retum Line	🗹 YES		
General Housekeeping	C YES		
Water Level Elevation			
Freeboard (m)			
Other Concerns	VES	NO NO	1,2
Railway Embankment Area			
Blackmore Culverts	VES		
South Loop Culverts	Z YES	NO NO	
West & Main Dam Area	VES		
South Seep	TYES	D NO	
West Seep	VES	D NO	
North Seep	VES		
Dredge Line	VES YES	D NO	
Rip Rap	V YES		
Instrumentation	and the second		
Piezometers	V YES	D NO	
GPS Units	VES	N NO	
Slope Inclinometers	L YES		
Reservoir Area			
Tailings Surface	S YES	D NO	
-Dust Fences	T YES	O NO	ha
Tailings Wetted Perimeter	T YES		
Flowrates & Totalizers			
Shandley Flowrate		/	leak in I-Pitlink, see () (5)
Seepage Return Flowrate			
Kilmarnock West			
Kilmarnock East	/		
Additional On-going Monito	oring (add a	s required	
		V	

ADDITIONAL COMMENTS:

Checked NTP, there was a leak at at clamp line, approx. 0.51 in the Called Makhn 21 -0 was reported WILSON. Shill to appropriate was with ators by rear seeping during inspection Water 1810 to Fording River. m bear tracks at oted 10 3 CKed anima at south bumaus end seepage collection Burrows ot N Itely bigger are than ground Squirrel Will from Sparwood Someone ave nuronment potentially badgers). gnup incoms are an 5 tar side d Pembankment, cn continue to monitor. Wel Si hecker allings discharge area, water level back-floading low again, hot to area of concern. Sandbagt in place (3 high x 3 mide) Permits bern construction are in place for till alting for came and replaced clamp within an be creh reporting. Line from I-Pit was what our of soll within 30 minutes. down

Teck

Inspected By:	
Inspection Date:	July 20th, 2017
Weather & Temperature:	Pourtly cloudy; 23°C

ACTION ITEMS

Record any items of concern noted during the inspection; location of each action item shall be marked on the attached facility maps. If required, additional items can be included in the "Additional Comments" section.

Priority	Description
1	A high probability or actual dam safety issue considered immediately dangerous to life, health or the environment, or a significant regulatory concern.
2	If not corrected, could likely result in dam safety issues leading to injury, environmental impact or significant regulatory action; or, a repetitive deficiency that demonstrates a systematic breakdown of procedures.
3	Single occurrences of deficiencies or non-conformances that alone would not be expected to result in dam safety issues.
4	Best Management Practice as a suggestion for continuous improvement towards industry best practices that could further reduce potential risks. This typically includes ongoing construction items within the appropriate construction cycle.

Location	Item Description & Responsibility	Priority	Target Completion Date
STP downstream toe (south end	Animal burrows still present - under investigation by environment (perhaps badger burrows)	4	monitor Weckiy monthly
Waste water celly	Ama unkept >> responsibility of environment?	4	Ma

INSPECTION ITEMS

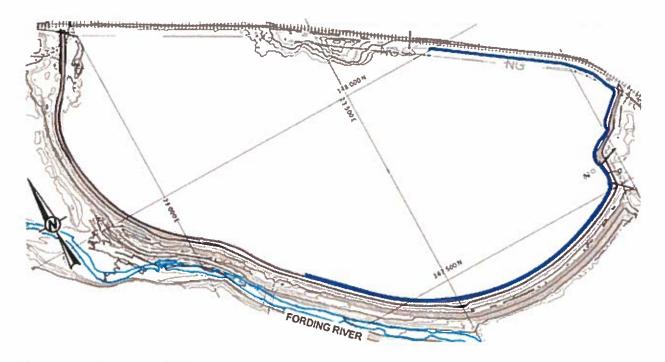
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	Turbidity or sediment in the seepage indicates a potential piping issue (internal erosion)			
	Seepage boils – check for sediment transport and flow rates. An increase in the number of boils indicates poor drainage conditions through the embankment			
Sinkholes	Generally occurring on the dam crest or slopes, can indicate internal settlement of the embankment			
External Erosion	Around, overtop or at the toe of the embankment			
	Heavy rainfall events typically cause substantial erosion on the downstream			
	slopes of earthen/tailings embankments			
Cracks	Longitudinal (along the length of the dam)			
	Transverse (across the dam)			
	Cracks should be marked with paint/stakes along the length and survey pins on			
	either side of the crack. These should be surveyed and monitored for			
Settlement	Excessive settlement			
	Survey pins should be installed in any new settled areas so that settlement can			
	be tracked over time.			
Sloughing / Slides	Heavy rainfall events can cause sloughing of material			
Animal Activity	Burrowing vectors can potentially damage the dam			
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Excessive growth	Vegetation can hide potential problems on the crest, slopes and toes of the			
	dams. Excessive root infiltration due to vegetation also has the potential of			
	damaging the structural integrity of the dam.			
General housekeeping				
	Dusting concerns from the tailings pond or embankment materials			
	Access to the dams (due to snow, poor road conditions, etc.)			
Standing water	On the crest or at the toe of the dam. Excessive water can cause increases in			
	the saturation of the embankment fills, reducing the soil strength			
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	Pipeline leaks on dam crests or slopes will begin to erode the embankment			
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SOUTH TAILINGS POND

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- Minimum Freeboard = 1.2 m
- Mark any issues on the map below, including tailings wetted perimeter



Structure	Condition Acceptable	Comments
South Tailings Pond – Ma	ain Embankment	
Access	🗹 YES 🗖 NO	
Downstream Toe	VES DNO	
Downstream Slope	🗹 YES 🗖 NO	
Dam Crest	VES D NO	
South Abutment	YES D NO	
North Abutment	YES INO	
South Abutment Area		
Kilmamock W		
Kilmamock E		
Fortis Gasline		loady of handed to gastinea
Clarified Line		

Barge & anchors	⊠ ´YES		
Seepage Return Line	🖬 YES		
General Housekeeping	YES		dyke roads need new gravel
Water Level Elevation			
Freeboard (m)			
Other Concerns	D YES	N O	
Railway Embankment Area			
Blackmore Culverts	🗹 YES		
South Loop Culverts	YES		
West & Main Dam Area	🗹 YES		
South Seep	🗹 YES		
West Seep	D TYES		
North Seep	Q YES		
Dredge Line	YES		
Rip Rap	YES		
Instrumentation	814.844		
Piezometers	YES		some piezos have an imal chev man
GPS Units	YES		
Slope Inclinometers	VES		data collected this month
Reservoir Area		200/1 201	
Tailings Surface	YES		
Tailings Wetted Perimeter	VES ·		
Flowrates & Totalizers			
Shandley Flowrate			
Seepage Return Flowrate			Shot taken
Kilmarnock West			Znor men
Kilmarnock East			
Additional On-going Monito	ring (add a	s required	()
	ring (add a	s require	±) ±)
Additional On-going Monito	ring (add a	s require	d)

Teck

. 7

1) dust in tailings pond significant due to windy weather Tailings discharge berm completed by Transcendent. No issues with berm construction Berm built in front of pre-existing sound bags on the road side ? high Im 64 wide. m Mater level was increasing but AIII not near the orres of concern. Hoe to do this of clean-up work (moving pipes and distributing Kilmamock TII edge contractory uping some orklift on any

Inspected By: Heather	Brickner
Inspection Date: July 25	2017
Weather & Temperature:	26°C

ACTION ITEMS

Record any items of concern noted during the inspection; location of each action item shall be marked on the attached facility maps. If required, additional items can be included in the "Additional Comments" section.

Priority	Description	
1	A high probability or actual dam safety issue considered immediately dangerous to life, health or the environment, or a significant regulatory concern.	
2	If not corrected, could likely result in dam safety issues leading to injury, environmental impact or significant regulatory action; or, a repetitive deficiency that demonstrates a systematic breakdown of procedures.	
3	Single occurrences of deficiencies or non-conformances that alone would not be expected to result in dam safety issues.	
4	Best Management Practice as a suggestion for continuous improvement towards industry best practices that could further reduce potential risks. This typically includes ongoing construction items within the appropriate construction cycle.	

Location	Item Description & Responsibility	Priority	Target Completion Date
STP (laver road)	Build up STP berms (lower road) and place fill in wet spot/low spot near nasternater cells	3	Aug. 25, 2017 *
	monitor GBRS 341 Weekly (check GE displacement data) send weekly email	4	Weekly

* Inspection item from MEM, complete within 30 days.

INSPECTION ITEMS

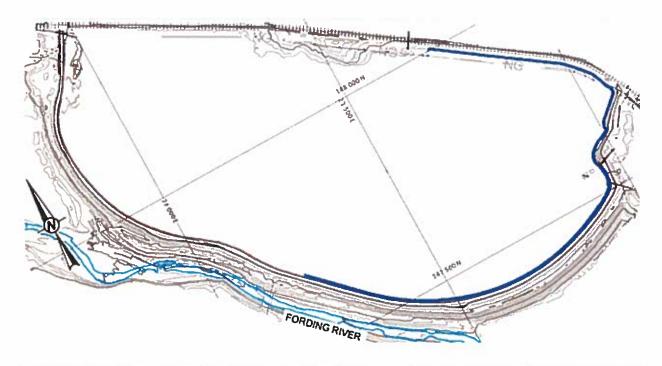
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Item Description	Potential Problems	
Piping	Uncontrolled internal erosion of the embankment, resulting in rapid failure of the dam	
	Any signs of piping should be reported immediately, as this is a serious risk to the structural integrity of the dam	
Seepage	Increase in volume of flow Turbidity or sediment in the seepage indicates a potential piping issue (internal erosion) Seepage boils – check for sediment transport and flow rates. An increase in the	
Sinkholes	number of boils indicates poor drainage conditions through the embankment Generally occurring on the dam crest or slopes, can indicate internal settlement of the embankment	
External Erosion	Around, overtop or at the toe of the embankment Heavy rainfall events typically cause substantial erosion on the downstream slopes of earthen/tailings embankments	
Cracks	Longitudinal (along the length of the dam) Transverse (across the dam) Cracks should be marked with paint/stakes along the length and survey pins on either side of the crack. These should be surveyed and monitored for	
Settlement	Excessive settlement Survey pins should be installed in any new settled areas so that settlement can be tracked over time.	
Sloughing / Slides	Heavy rainfall events can cause sloughing of material	
Animal Activity	Burrowing vectors can potentially damage the dam Large animal activity (bears, elk, etc.) can pose a safety concern to those working in the area	
Excessive growth	Vegetation can hide potential problems on the crest, slopes and toes of the dams. Excessive root infiltration due to vegetation also has the potential of damaging the structural integrity of the dam.	
General housekeeping	Excessive debris / waste materials Dusting concerns from the tailings pond or embankment materials Access to the dams (due to snow, poor road conditions, etc.)	
Standing water	On the crest or at the toe of the dam. Excessive water can cause increases in the saturation of the embankment fills, reducing the soil strength	
Over-steepened slope	Steep slopes can lead to sloughing of material and general dam stability issues	
Pipeline conditions	Leaks or holes / tears in the line Damage to the pipelines from passing equipment / snow removal Pipeline leaks on dam crests or slopes will begin to erode the embankment material very quickly, with the potential to develop into a dam breach scenario	

SOUTH TAILINGS POND

- Minimum crest elevation = 1638.30 masl (mine grid UTM)
- Minimum Freeboard = 1.2 m
- Mark any issues on the map below, including tailings wetted perimeter



Structure	Condition Acceptable	Comments
South Tailings Pond – Main	Embankment	
Access		
Downstream Toe		7
Downstream Slope	M YES D NO	PTD,
Dam Crest	🗹 YES 🗖 NO	
Upstream Slope		6
South Abutment		B
North Abutment	VES D NO	3
STP Tailings Discharge		
Fortis Gasline		
Till Berm / Sandbag Berm		0
South Abutment Area		

Kilmamock E		
	YES 🗖 NO	
Fortis Gasline	YES INO	
Clarified Line	YES D NO	
Barge & anchors	YES NO	
Seepage Return Line		
General Housekeeping	YES INO	24
Nater Level Elevation		
Freeboard (m)	1	
Other Concerns	VES NO	no other concerns
Railway Embankment Area		
Blackmore Culverts		
South Loop Culverts		
West & Main Dam Area		
South Seep		
West Seep		
North Seep		
Dredge Line		
Rip Rap		
nstrumentation		
Piezometers		
GPS Units		8
Slope Inclinometers		
Reservoir Area		
Tailings Surface		5
Failings Wetted Perimeter		dredging on-going.
Additional On-going Monit	oring (add as required	

ADDITIONAL COMMENTS:

Till bermat STP discharge north abutmentarea was constructed July 20. Built directly behind the sandbog berm, temporary only. WL was not encroaching In this area MEM Geotech inspection completed at the same time as this weekly. to issues with wad in this area (no low spots, etc) Downstream slope (Maxam vaid) was checked, no any roughs of excess seepage on surface. 4) Wet spot near the wastewater cells, likely formed after thunderstorm over weekend. Fill material needed to stop water from panding. EIK tracks were again noted in the tailings hear the northwest corner of the bond 6) Upstream slope is oversteepened and susceptible to hurther erosion from wave action. Iten was brought up by the inspectors. This work is planned tor September. Riprap along Fording River looked fine, inspecturs no concerns about it. weekly on-going moniton ha 34

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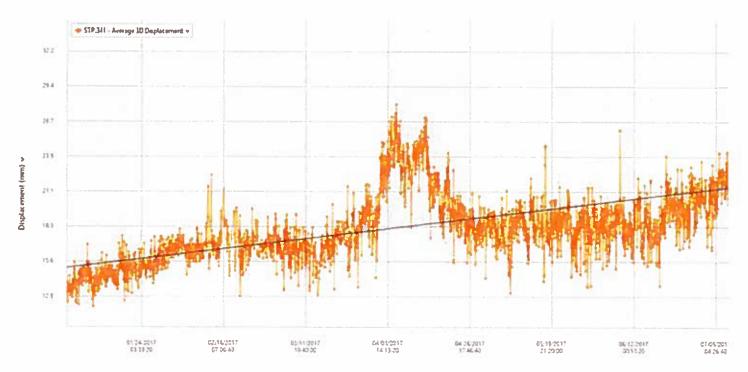
Brickner Heather FRO

From: Sent:	Brickner Heather FRO Friday, July 28, 2017 9:22 AM
То:	Langer Adam FRO (Adam.Langer@teck.com)
Cc:	FRO; FRO Processor
Subject:	GPS Unit 341

Hi Adam,

I have checked the displacement data for GPS Unit 341 on the STP, which was the GPS flagged at the last tailings meeting due to the displacement data showing a slow rise. It looks like this trend is continuing although it may be starting to level off the past couple weeks. The linear trendline is still showing the upward trend we noticed before.

Heather



Heather Brickner, M.Sc., P.Eng. Tailings Engineer Fording River Operations Teck Resources Limited Direct: 250-865-6537 Mobile: 250-425-3852 Email: <u>heather.brickner@teck.com</u>



Inspected By: Heather Brickner		
Inspection Date: July 31, 2017		
Neather & Temperature: <u>Mostly sunny</u> , 24°C		
and the second		

ACTION ITEMS

Record any items of concern noted during the inspection; location of each action item shall be marked on the attached facility maps. If required, additional items can be included in the "Additional Comments" section.

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Location	Item Description & Responsibility	Priority	Target Completion Date
STP	Ponded Water - STP near Woustewater cells	3	Aug 31 2017
NTP	Boulders on access had, Could damage vehicles (Transcendent to look at this next week)	3	Aug 31, 2017
7.4			

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INSPECTION ITEMS

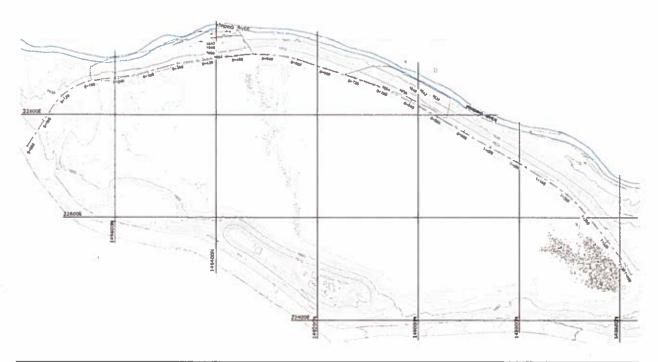
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Item Description	Potential Problems
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	the structural integrity of the dam
Seepage	Increase in volume of flow
	Turbidity or sediment in the seepage indicates a potential piping issue (internal erosion)
	Seepage boils – check for sediment transport and flow rates. An increase in the
	number of boils indicates poor drainage conditions through the embankment
Sinkholes	Generally occurring on the dam crest or slopes, can indicate internal settlement
	of the embankment
External Erosion	Around, overtop or at the toe of the embankment
	Heavy rainfall events typically cause substantial erosion on the downstream slopes of earthen/tailings embankments
Cracks	Longitudinal (along the length of the dam)
Ordono	Transverse (across the dam)
	Cracks should be marked with paint/stakes along the length and survey pins on
	either side of the crack. These should be surveyed and monitored for
Settlement	Excessive settlement
	Survey pins should be installed in any new settled areas so that settlement can
01 11 (011)	be tracked over time.
Sloughing / Slides	Heavy rainfall events can cause sloughing of material
Animal Activity	Burrowing vectors can potentially damage the dam Large animal activity (bears, elk, etc.) can pose a safety concern to those
	working in the area
Excessive growth	Vegetation can hide potential problems on the crest, slopes and toes of the
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	damaging the structural integrity of the dam.
General housekeeping	Excessive debris / waste materials
	Dusting concerns from the tailings pond or embankment materials
	Access to the dams (due to snow, poor road conditions, etc.)
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Over-steepened slope	 the saturation of the embankment fills, reducing the soil strength Steep slopes can lead to sloughing of material and general dam stability issues
Pipeline conditions	Leaks or holes / tears in the line
	Damage to the pipelines from passing equipment / snow removal
	Pipeline leaks on dam crests or slopes will begin to erode the embankment
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NORTH TAILINGS POND

- Minimum crest elevation = 1653.54 m (mine grid UTM)
- Minimum Freeboard = 1.2 m
- Mark any issues on the map below, including tailings wetted perimeter

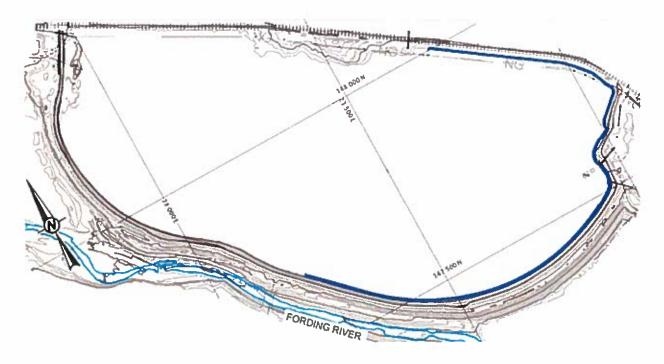


Structure	Condition Acceptable	Comments	
North Tailings Pond – Main	North Tailings Pond – Main Embankment		
Access	🗖 YES 🗹 NO	5	
Downstream Toe	YES NO		
Downstream Slope	YES D NO		
Dam Crest	VES DNO		
East Abutment			
West Abutment			
Rip Rap	YES D NO		
Old Tailings Line	YES D NO		
2013 Main Failure Area			
2013 South Failure Area			
Old Scraper Crossing	YES INO		
FRNTP2			

Cl. Lines River Crossing		
Cl. Lines inside Pond		
Steep Pipe Crossing		
Black PVC Pipe Crossing		
Barge Infrastructure		
Shandley Bridge	YES DINO	
Prisms (+ take photo)		
General Housekeeping		
Water Level Elevation	1650.22m	surveyed July 5,2017
Freeboard (m)	3.32 M	
Other Concerns	TYES NO	4
West Abutment Area		
Facilities Dump	YES DINO	
Pond Coal Stockpile	YES DINO	
Shandley Line		
Other Concerns	YES D NO	
Reservoir Area		
North Abutment Area	YES D NO	
Tailings Surface		
Dust Fences		
Tailings Wetted Perimeter		
Other Concerns		
Additional On-going Moni	toring (add as required	
		2. 1705

SOUTH TAILINGS POND

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- Minimum Freeboard = 1.2 m
- Mark any issues on the map below, including tailings wetted perimeter



Structure	Condition Acceptable	Comments
South Tailings Pond – Ma	ain Embankment	
Access	YES NO	
Downstream Toe	VES INO	7
Downstream Slope		
Dam Crest	YES NO	
South Abutment		
North Abutment		2
South Abutment Area		
Kilmarnock W		
Kilmamock E		
Fortis Gasline		
Clarified Line		

N025	and the second states of the s	
Barge & anchors	YES D NO	
Seepage Return Line	YES DINO	
General Housekeeping	VES D NO	6
Water Level Elevation	1636.839m	(-2pm)
Freeboard (m)	1.46m	
Other Concerns	YES D NO	
Railway Embankment Area		
Blackmore Culverts	YES DINO	
South Loop Culverts		
West & Main Dam Area		
South Seep	VES DINO	
West Seep		1
North Seep	YES INO	
Dredge Line		
Rip Rap	YES D NO	
Instrumentation		
Piezometers	YES DINO	
GPS Units	YES DINO	3
Slope Inclinometers		
Reservoir Area		
Tailings Surface		
Tailings Wetted Perimeter	YES INO	
Flowrates & Totalizers p	ump crew El	nvironment monitors there
Shandley Flowrate		
Seepage Return Flowrate		
Kilmarnock West		
Kilmarnock East	/	
Additional On-going Monito	oring (add as required	

ADDITIONAL COMMENTS:

on STP is pretly extensive, Environmenthous -bth commented they are concerned with birdt ready getting stuck in it. No issues at STP tailings discharge, berm TI and sandbags looked corcerns hoted tine on embankment crest or downstream. was below the sandbag berm area GPS 341, hole is such hoted hecked awurd unit or base arge volume of material has been stockpilled on the Liverpool Pad hove 0 al inspect with John EOK inning 0 he is on site er this week Thursday Access had to NTP has a Ut arge be cleaned up. houlders, needs 0 pended in low spot by wentewat had Water refilled in When ST cher berms are repaired. staining observed on works by wer seeh reck with Environmentabou

Page 12 of 13 2017-07-10

8) Coal stockpik is blocking access to 3Pit North, dam can only be viewed from above. However, there is no water in this facility, hot a major concern at this time. 9) Seeds have not yet sprouted from the hydroseeding done earlier in the month. Page 13 of 13 2017-07-10

Inspected By:

Inspection Date: <u>August 11,2017</u> Weather & Temperature: <u>Partly cloudy</u> smokey, 8°C

ACTION ITEMS

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4	Best Management Practice as a suggestion for continuous improvement towards industry best practices that could further reduce potential risks. This typically includes ongoing construction items within the appropriate construction cycle.		

Location	Item Description & Responsibility	Priority	Target Completion Date
STP lower accuss road	Repair berns to Im and fill hele near wastewater cells (construction in progress today)	3	IN PROGRESS TARGET: AUG 25, 2017
STP	Weekly monitoring of GP\$ 341 Dsend weekly emoul	4	weekly

Has per last week's inspection inspection item from MEM. complete within 30 days to

Page 1 of 5 2017-07-25



INSPECTION ITEMS

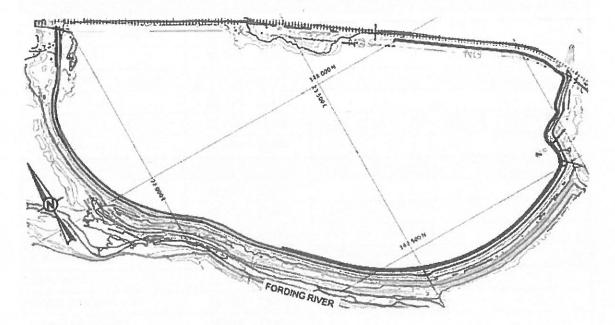
The dam crests, downstream slope, upstream slope, abutments, pipelines, instruments, culverts and other relevant structures shall be inspected. This form does not provide a complete list of potential issues. Details around observations and any other concerns should be noted in the "Additional Comments" section.

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Item Description	Potential Problems
Piping	Uncontrolled internal erosion of the embankment, resulting in rapid failure of the dam Any signs of piping should be reported immediately, as this is a serious risk to the structural integrity of the dam
Seepage	Increase in volume of flow Turbidity or sediment in the seepage indicates a potential piping issue (internal erosion) Seepage boils – check for sediment transport and flow rates. An increase in the number of boils indicates poor drainage conditions through the embankment
Sinkholes	Generally occurring on the dam crest or slopes, can indicate internal settlement of the embankment
External Erosion	Around, overtop or at the toe of the embankment Heavy rainfall events typically cause substantial erosion on the downstream slopes of earthen/tailings embankments
Cracks	Longitudinal (along the length of the dam) Transverse (across the dam) Cracks should be marked with paint/stakes along the length and survey pins on either side of the crack. These should be surveyed and monitored for
Settlement	Excessive settlement Survey pins should be installed in any new settled areas so that settlement can be tracked over time.
Sloughing / Slides	Heavy rainfall events can cause sloughing of material
Animal Activity	Burrowing vectors can potentially damage the dam Large animal activity (bears, elk, etc.) can pose a safety concern to those working in the area
Excessive growth	Vegetation can hide potential problems on the crest, slopes and toes of the dams. Excessive root infiltration due to vegetation also has the potential of damaging the structural integrity of the dam.
General housekeeping	Excessive debris / waste materials Dusting concerns from the tailings pond or embankment materials Access to the dams (due to snow, poor road conditions, etc.)
Standing water	On the crest or at the toe of the dam. Excessive water can cause increases in the saturation of the embankment fills, reducing the soil strength
Over-steepened slope	Steep slopes can lead to sloughing of material and general dam stability issues
Pipeline conditions	Leaks or holes / tears in the line Damage to the pipelines from passing equipment / snow removal Pipeline leaks on dam crests or slopes will begin to erode the embankment material very quickly, with the potential to develop into a dam breach scenario

SOUTH TAILINGS POND

- Minimum crest elevation = 1638.30 masl (mine grid UTM)
- Minimum Freeboard = 1.2 m
- Mark any issues on the map below, including tailings wetted perimeter



Structure	Condition Acceptable	Comments
South Tailings Pond - Main	Embankment	
Access	YES INO	
Downstream Toe	YES NO	
Downstream Slope	YES NO	
Dam Crest	YES NO	
Upstream Slope	YES NO	
South Abutment	YES NO	
North Abutment	YES DNO	
STP Tailings Discharge	VES NO	
Fortis Gasline	YES NO	puddle filled in
Till Berm / Sandbag Berm	VES NO	
South Abutment Area		

Page 3 of 5 2017-07-25



YES YES YES YES YES YES YES YES	NO NO NO NO	not o	hee	Ale		
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Teck

ADDITIONAL COMMENTS:

-GPS 341 continues to trund upward in average 3D displacement; however over the last fin days displacement has decreased - GPS appears to be unchanged - Water still collected near waste water cells -Signs of enotion water movement near NLP - ditches along lower occess road containing Amallamounty of still water 12:50 from jouthern - a ceff road updated 64 end of Dond to pipe pride > Surface of rodd if dirt Mepage and growth by Neet - embour Knew tailings upped for ditch of WOIKB at angle of repose > ditches verestablished toe fide of road dn

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Inspected By:	H.Brickner
Inspection Date:	Aug. 17/2017
Weather & Temperature:	Sunny, 26°C

ACTION ITEMS

6 1 - 1

Record any items of concern noted during the inspection; location of each action item shall be marked on the attached facility maps. If required, additional items can be included in the "Additional Comments" section.

Priority	Description
1	A high probability or actual dam safety issue considered immediately dangerous to life, health or the environment, or a significant regulatory concern.
2	If not corrected, could likely result in dam safety issues leading to injury, environmental impact or significant regulatory action; or, a repetitive deficiency that demonstrates a systematic breakdown of procedures.
3	Single occurrences of deficiencies or non-conformances that alone would not be expected to result in a dam safety issues.
4	Best Management Practice as a suggestion for continuous improvement towards industry best practices that could further reduce potential risks. This typically includes ongoing construction items within the appropriate construction cycle.

Location	Item Description & Responsibility	Priority	Target Completion Date
STP north abutment	Water ponded hear wantewater cells (horth end) No drainage from area	0	
north	cells (north end)	3	
abutment	No drainage from area		
	a v		1
		-	L
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INSPECTION ITEMS

The dam crests, downstream slope, upstream slope, abutments, pipelines, instruments, culverts and other relevant structures shall be inspected. This form does not provide a complete list of potential issues. Details around observations and any other concerns should be noted in the "Additional Comments" section.

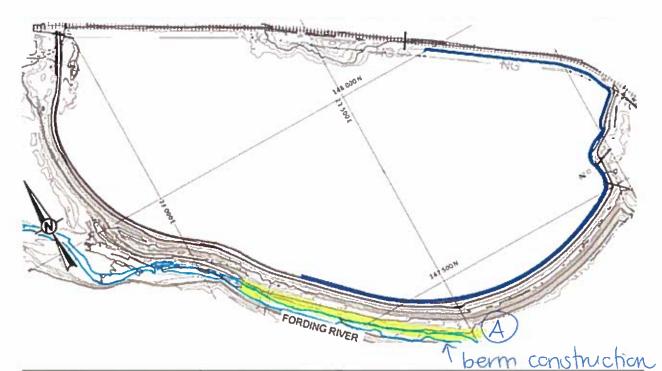
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Excessive growth	Vegetation can hide potential problems on the crest, slopes and toes of the
	dams. Excessive root infiltration due to vegetation also has the potential of
	damaging the structural integrity of the dam.
General housekeeping	
	Dusting concerns from the tailings pond or embankment materials
	Access to the dams (due to snow, poor road conditions, etc.)
Standing water	On the crest or at the toe of the dam. Excessive water can cause increases in
	the saturation of the embankment fills, reducing the soil strength
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Pipeline conditions	Leaks or holes / tears in the line
	Damage to the pipelines from passing equipment / snow removal
	Pipeline leaks on dam crests or slopes will begin to erode the embankment
	material very quickly, with the potential to develop into a dam breach scenario

SOUTH TAILINGS POND

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- Minimum crest elevation = 1638.30 masl (mine grid UTM)
- Minimum Freeboard = 1.2 m
- Mark any issues on the map below, including tailings wetted perimeter



Structure	Condition Acceptable	Comments
South Tailings Pond – Main	Embankment	
Access		1
Downstream Toe	YES D NO	
Downstream Slope		1,2
Dam Crest		4 Concern hoted.
Upstream Slope		
South Abutment	YES NO	
North Abutment		
STP Tailings Discharge		
Fortis Gasline		
Till Berm / Sandbag Berm		3
South Abutment Area		

	_
	-
	_
	_
	1
1636.78 m	_
	993 1998
oring (add as required)	

Teck

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1 1 11

Ditches along the lower wad on the west embankment/West-Dam have been deaned small swall was constr and re-graded A southwest corner to facilitate drainage from at the ditches Callow it to run-off and not pond Refer to approximate location out "A" Berms alongouter edge of West Dam lower wad were also completed. This way a request (STP) (on site July 25 alotechnical inspectors shain in vellow on map (approx.) Berm construction berm and tailings discharge area inspected he issues hoted. Road Crest and DS slope Maxam no issuer. Water at discharge location SIDE had below "lip" of tailings hot against sandbagt was berm Refer to photos. 4) Some water (more than has been seen previously was bonded the low area hear WW cells and allings discharge. Transcender ren is onea in but drainage ustill asked an Jater accumulation likely due to ISSUR. Drecit area again par WEEK check Page 5 of 5 2017-07-25



Inspected By:	H. Brickner	_
Inspection Date:	Aug. 24/2017	_
Weather & Temperature:	21°C mostly sunny	_

ACTION ITEMS

Record any items of concern noted during the inspection; location of each action item shall be marked on the attached facility maps. If required, additional items can be included in the "Additional Comments" section.

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4	Best Management Practice as a suggestion for continuous improvement towards industry best practices that could further reduce potential risks. This typically includes ongoing construction items within the appropriate construction cycle.

Location	Item Description & Responsibility	Priority	Target Completion Date
STP (South end)	Inspect emergency viprap stockpile to determine if it's usable (W/KWL)	2	Aug. 31, 2017

INSPECTION ITEMS

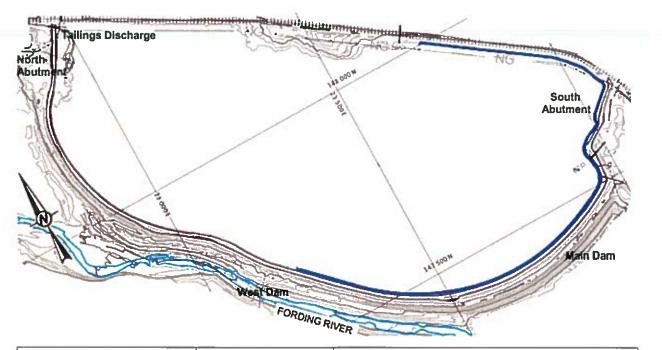
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Sinkholes	Generally occurring on the dam crest or slopes, can indicate internal settlement of the embankment
External Erosion	Around, overtop or at the toe of the embankment
	Heavy rainfall events typically cause substantial erosion on the downstream
	slopes of earthen/tailings embankments
Cracks	Longitudinal (along the length of the dam)
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Settlement	Excessive settlement
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Sloughing / Slides	Heavy rainfall events can cause sloughing of material
Animal Activity	Burrowing vectors can potentially damage the dam
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	working in the area
Excessive growth	Vegetation can hide potential problems on the crest, slopes and toes of the
Ũ	dams. Excessive root infiltration due to vegetation also has the potential of
	damaging the structural integrity of the dam.
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	Dusting concerns from the tailings pond or embankment materials
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Standing water	On the crest or at the toe of the dam. Excessive water can cause increases in
· · · · · · · · · · · · · · · · · · ·	the saturation of the embankment fills, reducing the soil strength
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- Mark any issues on the map below, including tailings wetted perimeter



Structure	Condition Acceptable	Comments
West Dam & North Abutment	Area	
Access	YES INO	train in the loop
Downstream Toe		
Downstream Slope	YES DINO	
Dam Crest		
Upstream Slope		
West Seep		
North Seep		
Lower Road (ditches/berms)		
North Abutment		1,4,5
STP Tailings Discharge		· · · · · · · · · · · · · · · · · · ·
FortisBC Gasline	YES INO	
Tailings Line		
Shandley Line	YES D NO	
Till Berm / Sandbag Berm	YES DINO	

General Housekeeping		
Main Embankment & South Ab	outment Area	
Access	🗹 YES 🗖 NO	
Downstream Toe	YES DINO	
Downstream Slope		
Dam Crest		
Upstream Slope		
South Seep	YES DINO	
Kilmamock W Line	YES NO	
Kilmarnock E Line	YES D NO	
South Abutment	YES NO	
FortisBC Gasline	🗹 YES 🗖 NO	
Clarified Line	YES 🗖 NO	
Barge & anchors	YES D NO	
Seepage Return Line	YES 🗖 NO	
General Housekeeping	YES 🗖 NO	4
Water Level Elevation	1636.94m	
Freeboard (m)	1.36m	good.
Railway Embankment Area		0
Blackmore Culverts	🗹 YES 🗖 NO	
South Loop Culverts	🗹 YES 🗖 NO	
Dredge Line	🗹 YES 🗖 NO	
Rip Rap	🗹 YES 🗖 NO	
Instrumentation		
Piezometers		
GPS Units		3
Slope Inclinometers	YES INO	
Reservoir Area		
Tailings Surface		
Tailings Wetted Perimeter		
Additional On-going Monito	oring (add as required	
check GRS 341	~	see comment 3, attached date
		L

Some fairly extensive durting was seen at the tailings discharge, wind was ustrong wi gusts. Refer to photos and videos. emergency riprap stockpile at the south 2) Inspected end of STP (nr. Main Dam), at 1 had been told by It was no longer a contractor (Transcendent) that Suitable to put in over due to weathering. could not see endence of weathering, will inspect wi Jason Miller from KWL when he's on site next week. with the checked GPS 341 (it had been the unit trend). Could hat see any endence of upward movement on the unit or In the surrounding area. check data for trends again (see attached hay installed lonced that Environment phopane banger at south abutment. Received no hobbication from them. Checked with Erika and they told her they plan to install 3 additional bangers. emaul and ask that they notity me if they are doing an Work on the dams * Wet area near WW cells was much drier than the week. Still need drainage plan earlier th Page 5 of 5 * Update Aug 25 saud 2017-08-24 installed in, 2 more on Main Dam and one

rear Wwwcells. Said he would notify me in future banywork.

Inspected By: H. Brickner Inspection Date: Aug. 29/2017 & Aug.31/2017 * & Temperature: Sunny/Smokey, 30°C & Sunny, 15°C Weather & Temperature: Sunny

ACTION ITEMS

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Location	Item Description & Responsibility	Priority	Target Completion Date
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			8

INSPECTION ITEMS

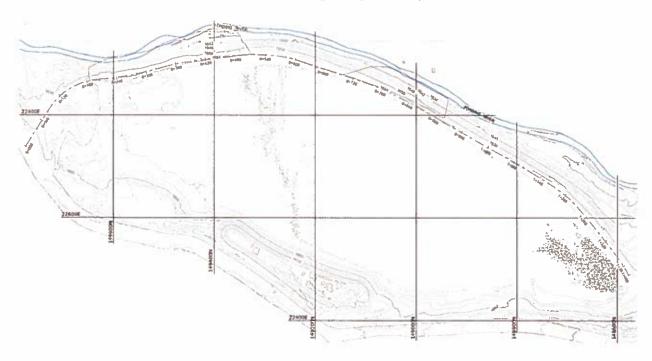
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	erosion) Seepage boils – check for sediment transport and flow rates. An increase in the
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Sinkholes	Generally occurring on the dam crest or slopes, can indicate internal settlement
	of the embankment
External Erosion	Around, overtop or at the toe of the embankment
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Claughing / Clides	be tracked over time.
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Excessive growth	dams. Excessive root infiltration due to vegetation also has the potential of
	damaging the structural integrity of the dam.
General housekeeping	
	Dusting concerns from the tailings pond or embankment materials
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Standing water	On the crest or at the toe of the dam. Excessive water can cause increases in
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Pipeline conditions	Leaks or holes / tears in the line
	Damage to the pipelines from passing equipment / snow removal
	Pipeline leaks on dam crests or slopes will begin to erode the embankment
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NORTH TAILINGS POND

- Minimum crest elevation = 1653.54 m (mine grid UTM)
- Minimum Freeboard = 1.2 m
- Mark any issues on the map below, including tailings wetted perimeter

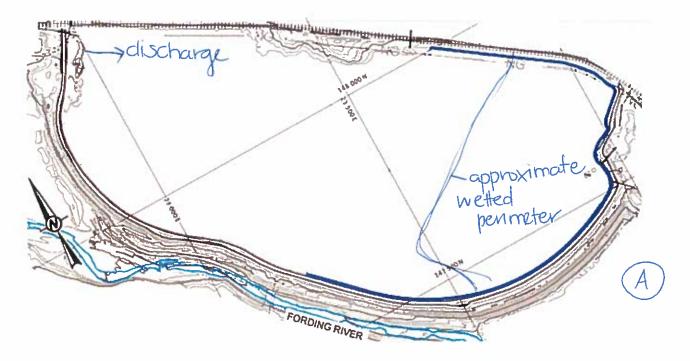


Structure	Condition Acceptable	Comments
North Tailings Pond - Main	Embankment	
Access		
Downstream Toe		
Downstream Slope		
Dam Crest		
East Abutment		
West Abutment		
Rip Rap		
Old Tailings Line	VES INO	
2013 Main Failure Area		
2013 South Failure Area		
Old Scraper Crossing	YES INO	
FRNTP2		

Cl. Lines River Crossing	VES 🗖 NO	
Cl. Lines inside Pond	YES DINO	
Steep Pipe Crossing	YES D NO	
Black PVC Pipe Crossing	YES D NO	
Barge Infrastructure	YES D NO	
Shandley Bridge	VES D NO	
Prisms (+ take photo)	YES D NO	
General Housekeeping	YES 🗖 NO	10- .
Water Level Elevation	650.0m	monthly survey Aug. 10
Freeboard (m)	3.54m	1 1 2 0
Other Concerns		6
West Abutment Area		
Facilities Dump	SI YES DI NO	
Pond Coal Stockpile	YES D NO	
Shandley Line		
Other Concerns	YES DINO	
Reservoir Area		
North Abutment Area		
Tailings Surface	YES D NO	
Dust Fences		
Tailings Wetted Perimeter		
Other Concerns	YES DINO	
Additional On-going Monito	oring (add as required	
Monitor drainage to NTP from Liverpool Pond system	\checkmark	ho concerns.

SOUTH TAILINGS POND

- Minimum crest elevation = 1638.30 masl (mine grid UTM)
- Minimum Freeboard = 1.2 m
- Mark any issues on the map below, including tailings wetted perimeter



Structure	Condition Acceptable	Comments
West Dam & North Abutment	Area	
Access	YES DINO	
Downstream Toe	VES DINO	
Downstream Slope	VES DNO	
Dam Crest		
Upstream Slope	YES NO	
West Seep		
North Seep		
Lower Road (ditches/berms)	YES DINO	
North Abutment		
STP Tailings Discharge	YES D NO	2
FortisBC Gasline	YES NO	
Tailings Line	YES INO	43
Shandley Line		

A - emergency uprap stockpile

Page 5 of 13 2017-08-24

Till Berm / Sandbag Berm	VES INO	2		
General Housekeeping	YES D NO	12		
Main Embankment & South Abutment Area				
Access	YES INO			
Downstream Toe	YES DINO			
Downstream Slope				
Dam Crest	YES DINO			
Upstream Slope	VES NO			
South Seep				
Kilmamock W Line				
Kilmarnock E Line				
South Abutment				
FortisBC Gasline	YES INO			
Clarified Line				
Barge & anchors	YES NO			
Seepage Return Line	YES NO			
General Housekeeping		53		
Water Level Elevation	16.36.70m	Aug. 31/2017		
Freeboard (m)	1.6m	U		
Railway Embankment Area				
Blackmore Culverts				
South Loop Culverts				
Dredge Line				
Rip Rap	YES NO			
Instrumentation				
Piezometers	YES INO			
GPS Units		checked 341, 10.		
Slope Inclinometers				
Reservoir Area	1			
Tailings Surface	VES DINO			
Tailings Wetted Perimeter				
Additional On-going Monit	oring (add as required)		

ADDITIONAL COMMENTS:

NTP. Riprap construction along issuer at Aug. 29 on SIt R NVer bahk Paan 6 br this work to issuer at Turnbull TSF, dredging as on-going hspected Henretta culvert crossing Nat 91P remain the creek. Hydroseeding doet ho OW have been successi vegetation Seem ha 10 ISSULLY a 3Pit North and Sou have Consider hankmar hau ∇ embankmen 3 annua access auth Accessible on toot only Unit 341 data was neviewed for months, attached. - Swift project in Iverbool Pad the good tor other remaina moist matena basma Kass the area ect. KPC 5 nc additional than 30m be considered a April lless required las a s ~lom onna m rea near WW cells ave has dived up Transcendent fix this when ST pam UD 5 Ubta dane. IS

 $F = \mathbb{R}^{2}$

) Pump secondary containment is slowly filling with
Water (one drip).
2) Area at STP discharge and till berm inspected.
no issues. Water level is lover than the base of
the berm
3) Leak has been repaired (Shandley line through
South Embankment). No concerns noted with the
tailings line
4) Consulting engineers from Norwest are on site
Aug. 31 to collect some samples from STP discharge
and Turnbull TSF dredge line. Environment
personnel will be assisting them with sampling
(there are sampling ports in both lines). Project
Engineering and Terrapure (dredge company) one
also aware of the sampling.
5) Jason Miller (KWL) on site provided comments on
the quality/durability of the tock in the STP
emergency nprap istockpile. He said it was not
suitable for long-term use in Fording River. Swift
project team is considering using it for a lock
project team is considering using it for a tock drain, and replacing the uprap with better quality material.
Indtenal.

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		Location Date Sample Type		FR_STPBARGE 28/11/2016 NP	FR_STPBARGE 28/11/2016 FD	FR_STPBARGE 13/06/2017 NP	FR_STPBARGE 04/07/2017 NP	FR_STPBARGE 12/07/2017 N	FR_STPBARGE 18/07/2017 N	FR_STPBAI 27/07/201 N
Fraction		Unit	Result	Result	Result	Result	Result	Result	Result	Result
1	ALUMINUM ANTIMONY	3	0.0045 0.00297	0.0036 0.00454	0.0034 0.00451	0.0042 0.00332	0.0047 0.00296	0.0048 0.00413	0.0040 0.00311	0.0034 0.00315
	ARSENIC	ŭ	0.00059	0.00054	0.00059	0.00095	0.00066	0.00080	0.00060	0.00068
	BARIUM	mg/l	0.104	0.127	0.133	0.135	0.0819	0.103	0.0828	0.0707
	BERYLLIUM	mg/l	< 0.000020	< 0.000020	< 0.000020	< 0.000020	< 0.000020	< 0.000020	< 0.000020	< 0.000020
	BISMUTH	mg/l	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050
	BORON BROMIDE	mg/l mg/l	0.028 < 0.25	0.026 < 0.25	0.026 < 0.25	0.026 < 0.050	0.026 0.051	0.030 0.083	0.025 < 0.25	0.028 < 0.25
	CADMIUM	ŭ	0.000215	0.000678	0.000710	0.000549	0.000458	0.000587	0.000363	0.000300
	CALCIUM	mg/l	121	141	143	95.1	109	104	108	112
	CARBON, DISSOLV	mg/l	0.51	0.53	< 0.50	1.28	1.16	1.95	1.54	1.74
	CHLORIDE	mg/l	3.69	5.00	5.02	2.35	4.49	6.00	2.8	3.4
	CHROMIUM	mg/l	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010
	COBALT COPPER	ŭ	0.00161 0.00063	0.00295 0.00122	0.00305	0.00178 0.00083	0.00145 0.00061	0.00220 0.00077	0.00147 0.00070	0.00143 0.00077
	FLUORIDE	ŭ	0.43	0.48	0.49	0.427	0.526	0.649	0.32	0.37
	IRON	mg/l	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
	LEAD	mg/l	0.000050	0.000128	0.000130	0.000143	0.000110	0.000140	0.000080	0.000077
	LITHIUM		0.0713	0.0903	0.0932	0.0629	0.0602	0.0710	0.0632	0.0617
	MAGNESIUM	ŭ	70.8	72.3	74.6	56.5	63.0	61.3	63.2	70.2
	MANGANESE	Ŭ	0.0548	0.109	0.114	0.0791	0.0539	0.0738	0.0414	0.0428
	MERCURY MOLYBDENUM	mg/l mg/l	< 0.0000050 0.0179	< 0.0000050 0.0304	< 0.0000050 0.0303	< 0.000050 0.0203	< 0.0000050 0.0172	< 0.0000050 0.0282	< 0.0000050 0.0191	< 0.0000050 0.0179
	NICKEL	ŭ	0.00820	0.0125	0.0131	0.00535	0.00591	0.00551	0.00737	0.00706
	POTASSIUM	ŭ	6.32	7.19	7.51	5.25	5.39	6.95	5.71	5.35
	SELENIUM		73.6	81.3	82.7	54.8	59	55.5	65.6	73.5
	SILICON	ŭ	2.06	1.95	2.02	1.86	1.86	2.07	2.04	1.96
	SILVER	mg/l	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010
	SODIUM	mg/l	4.98	7.00	7.34	4.71	3.98	4.42	3.66	4.10
	STRONTIUM	U U	0.208	0.243	0.246	0.176	0.217	0.242	0.204	0.207
	SULFATE (AS SO4)		298 0.000042	377	376 0.000051	236 0.000039	254 < 0.000010	256 0.000072	261 0.000049	282 0.000052
	TIN	mg/l mg/l	0.000042 < 0.00010	0.000053 < 0.00010	< 0.000051	< 0.000039	< 0.000010	< 0.000072	< 0.000049	< 0.000052
	TITANIUM	mg/l mg/l	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010
	URANIUM	ŭ	0.0111	0.0162	0.0161	0.00836	0.00952	0.0109	0.00929	0.00987
	VANADIUM	3	0.00143	0.00190	0.00202	0.00081	0.00108	0.00126	0.00092	0.00118
	ZINC	0	0.0032	0.0257	0.0281	0.0074	0.0054	0.0044	0.0055	0.0061
	ACIDITY TO pH 8.3	U U	2.4	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	2.2	1.8
	ALKALINITY, BICAF		243	243	254	220	237	261	340	256
	ALKALINITY, CARB	mg/l	< 1.0	6.6	< 1.0	11.6	14.8	10.0	< 1.0	< 1.0
	ALKALINITY, HYDR	mg/l	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
	ALKALINITY, TOTA	lmg/l	243	249	254	231	252	271	340	256
	Cation - Anion Balar					-2.3	-1.7	-4.6	-10.3	-2.9
	CONDUCTIVITY, FI			1094						
	CONDUCTIVITY, LA		1100	1240	1240	876	987	949	1000	1010
	DISSOLVED OXYG DISSOLVED OXYG	0		9.21 67.0						
	Hardness, Total or D		595	650	663	470	532	511	531	570
	ION BALANCE	///g/i	-1.7	-3.8	-3.0	470	552	511	551	570
	MAJOR ANION SUN		12.7	14.7	14.8	10.2	11.4	11.7	13.5	12.4
	MAJOR CATION SL		12.3	13.6	13.9	9.79	11.0	10.6	10.9	11.7
	NITRATE NITROGE		21.5	23.4	23.4	8.41	12.3	9.76	15.7	18.4
	NITRITE NITROGEI	mg/l	0.266	0.421	0.418	0.275	0.200	0.446	0.316	0.292
	NITROGEN, AMMO		0.745	1.88	1.98	0.796	0.76	0.92	0.503	0.673
	ORTHO-PHOSPHA		< 0.0010	< 0.0010	< 0.0010	0.0284	0.0183	0.0214	0.0076	< 0.0010
	OXIDATION-REDUC			-28.5						
	OXIDATION-REDUC		302	318	315	458	299	293	300	342
	pH, Field pH, LAB	ph units	8.19	8.14 8.31	8.28	8.36	8.37	0.00	0.00	8.07
	PHOSPHORUS		0.0099	0.0185	0.0222	0.0871	0.0475	8.32 0.0387	8.23 0.0429	0.0322
	TEMPERATURE, FI		0.0000	2.4	0.0222	0.0071	0.0475	0.0007	0.0420	0.0322
	TOTAL DISSOLVED		820	908	925	604	718	710	691	680
	TOTAL KJELDAHL		1.22	2.44	2.77	2.03	1.97	1.37	1.18	1.56
	TOTAL SUSPENDE	U U	17.5	22.0	20.9	22.3	47.4	28.5	45.3	144
	TURBIDITY, LAB	ntu	10.8	15.9	14.9	12.8	31.2	27.4	32.3	129
	ALUMINUM	0	0.0287	0.0346	0.0285	0.0288	0.0409	0.0222	0.0405	0.051
	ANTIMONY	U U	0.00309	0.00477	0.00464	0.00329	0.00289	0.00397	0.00306	0.00301
	ARSENIC		0.00057	0.00061	0.00061	0.00101	0.00073	0.00077	0.00078	0.00072
	BARIUM	.	0.107	0.138	0.137	0.135	0.0864	0.105	0.0867	0.0801
	BERYLLIUM	mg/l	< 0.000020	< 0.000020	< 0.000020	< 0.000020	< 0.000020	< 0.000020	< 0.000020	< 0.00010
	BISMUTH BORON	mg/l mg/l	< 0.000050 0.028	< 0.000050 0.028	< 0.000050 0.027	< 0.000050 0.025	< 0.000050 0.025	< 0.000050 0.032	< 0.000050 0.026	< 0.00025 < 0.050
	CADMIUM	U U	0.028	0.028	0.027	0.025	0.025	0.032	0.026	< 0.050
	CALCIUM	mg/l	119	147	147	91.6	105	102	109	115
	CHROMIUM		0.00014	0.00067	0.00033	0.00010	0.00011	< 0.00010	0.00016	< 0.00050
	COBALT		0.00152	0.00321	0.00315	0.00183	0.00148	0.00223	0.00161	0.00158
	COPPER	mg/l	0.00121	0.00169	0.00149	0.00121	0.00101	0.00112	0.00118	< 0.0025
	IRON	0	0.021	0.040	0.026	0.029	0.056	0.027	0.137	0.072
	LEAD	0	0.000111	0.000258	0.000208	0.000236	0.000264	0.000238	0.000319	0.00030
	LITHIUM		0.0735	0.0934	0.0929	0.0591	0.0594	0.0696	0.0619	0.0630
	MAGNESIUM		60.7	76.0	76.4	57.0	63.1	60.2	62.6	72.1
	MANGANESE		0.0518	0.120	0.118	0.0835	0.0526	0.0772	0.0499	0.0564
	MERCURY	mg/l	< 0.000025	< 0.0000050	< 0.0000050	< 0.000050	< 0.000025	< 0.000050	< 0.0000050	< 0.000025
	MOLYBDENUM	Ŭ	0.0172	0.0317	0.0314	0.0202	0.0176	0.0284	0.0192	0.0180
	NICKEL POTASSIUM	U U	0.00757	0.0134 7.56	0.0136 7.80	0.00651 5.12	0.00645 5.20	0.00574 6.78	0.00772 5.49	0.0076 5.50
	SELENIUM		5.32 68.7	7.56 82.6	7.80 79.8	5.12 53.9	5.20 57.9	6.78 51	5.49 62.3	5.50 66.3
	SILICON	mg/l	2.06	2.14	2.09	1.89	57.9 1.94	2.14	2.06	2.01
	SILVER	mg/l	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000050
	SODIUM		4.59	7.44	7.53	4.72	3.83	4.37	3.58	3.99
	STRONTIUM		0.197	0.256	0.254	0.174	0.213	0.243	0.200	0.206
	THALLIUM		0.000044	0.000056	0.000054	0.000044	0.000050	0.000073	0.000052	0.000053
	TIN	mg/l	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00050
	TITANIUM	mg/l	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
	TOTAL ORGANIC C		5.89	8.58	8.57	9.99	28.7	6.81	31.3	28.6
			0.0100	0.0164	0.0162	0.00974	0.00069	0.0114	0.00000	0.00045

	1	TOTAL ORGANIC C	mg/i	5.09	0.00	0.07	9.99	20.7	0.01	31.3	20.0
	Т	URANIUM	mg/l	0.0100	0.0164	0.0162	0.00874	0.00968	0.0114	0.00909	0.00945
ſ	Т	VANADIUM	mg/l	0.00169	0.00235	0.00232	0.00152	0.00155	0.00151	0.00167	< 0.0025
ľ	Т	ZINC	mg/l	0.0056	0.0314	0.0236	0.0076	0.0065	0.0045	0.0095	< 0.015

		Location Date Sample Type	15/09/2016	FR_STPNSEEP 28/11/2016 NP	FR_STPNSEEP 12/06/2017 NP	FR_STPNSEEP 04/07/2017 NP	FR_STPNSEEP 12/07/2017 N	FR_STPNSEEP 18/07/2017 N	FR_STPNSEE 27/07/2017 N
Fraction	Analyte	Unit	Result	Result	Result	Result	Result	Result	Result
2	ALUMINUM	mg/l	< 0.0030	< 0.0030	< 0.0010	< 0.0010	< 0.0010	< 0.0010	< 0.0010
<u>כ</u> כ	ANTIMONY ARSENIC	mg/l mg/l	0.00015 < 0.00010	0.00012	0.00016	0.00018	0.00014	0.00016	0.00016
)	BARIUM	mg/l	0.106	0.0919	0.0619	0.0820	0.0881	0.0960	0.103
)	BERYLLIUM	mg/l	< 0.000020	< 0.000020	< 0.000020	< 0.00020	< 0.000020	< 0.000020	< 0.000020
)	BISMUTH	mg/l	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050
	BORON	mg/l	0.022	0.016	0.015	0.021	0.022	0.022	0.024
	BROMIDE	mg/l	< 0.25	< 0.25	0.062	0.050	0.066	< 0.25	0.063
	CADMIUM	mg/l	0.000109	0.0000475	0.000165	0.000235	0.000216	0.000230	0.000243
)	CALCIUM	mg/l	113	99.7	84.4	98.6	107	103	112
)	CARBON, DISSOL		1.01	0.85	1.38	2.28	1.62	1.29	1.79
	CHLORIDE CHROMIUM	mg/l	14.7 0.00010	19.5 < 0.00010	7.85 < 0.00010	8.09 < 0.00010	8.22 < 0.00010	7.9	9.08 < 0.00010
1	COBALT	mg/l mg/l	0.00049	0.00016	0.00029	0.00044	0.00045	0.00050	0.00055
	COPPER	mg/l	< 0.00050	< 0.00050	< 0.00029	0.00024	< 0.00020	0.00023	< 0.00020
)	FLUORIDE	mg/l	0.22	0.20	0.219	0.180	0.195	0.16	0.204
)	IRON	mg/l	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
)	LEAD	mg/l	< 0.000050	0.000064	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050
1	LITHIUM	mg/l	0.0426	0.0293	0.0316	0.0407	0.0433	0.0456	0.0431
	MAGNESIUM	mg/l	40.8	31.0	33.9	39.4	39.9	41.2	45.6
	MANGANESE	mg/l	0.00398	0.00129	0.00459	0.00607	0.00691	0.00773	0.00744
	MERCURY	mg/l	< 0.000050	< 0.0000050	< 0.000050	< 0.000050	< 0.0000050	< 0.000050	< 0.0000050
	MOLYBDENUM	mg/l	0.000901	0.00101	0.00107	0.00104	0.000977	0.000956	0.000947
		mg/l	0.00194	0.00089	0.00097	0.00142	0.00153	0.00163	0.00190
	POTASSIUM SELENIUM	mg/l	2.10 7.16	1.74	1.84 8.93	2.15 8.27	2.12	2.42	2.39 8.26
	SELENIUM	ug/l mg/l	7.16 2.73	14.4 2.30	8.93	8.27 2.15	8.38 2.26	7.93 2.39	8.26
	SILICON	mg/l	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010
	SODIUM	mg/l	5.33	4.75	3.86	4.96	4.95	5.31	5.88
	STRONTIUM	mg/l	0.197	0.168	0.138	0.175	0.183	0.189	0.197
	SULFATE (AS SO4		185	135	124	174	177	186	196
	THALLIUM	mg/l	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010	0.000010	< 0.000010
	TIN	mg/l	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010
	TITANIUM	mg/l	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
	URANIUM	mg/l	0.00271	0.00176	0.00278	0.00326	0.00266	0.00309	0.00309
	VANADIUM	mg/l	< 0.00050	< 0.00050	< 0.00050	< 0.00050	< 0.00050	< 0.00050	< 0.00050
	ZINC	mg/l	< 0.0030	< 0.0030	0.0018	0.0019	0.0015	0.0015	0.0022
	ACIDITY TO pH 8.3	3 mg/l	6.0	1.1	< 1.0	2.5	4.9	< 1.0	< 1.0
	ALKALINITY, BICA	Fmg/l	255	215	217	243	291	288	260
	ALKALINITY, CARE	3 mg/l	< 1.0	< 1.0	10.4	< 1.0	< 1.0	18.0	< 1.0
	ALKALINITY, HYDF	R mg/l	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
	ALKALINITY, TOTA	Almg/I	255	215	228	243	291	306	260
	Cation - Anion Bala				-2.5	-3.2	-6.0	-8.6	-1.0
	CONDUCTIVITY, F			644.1	576.2				
	CONDUCTIVITY, L		843	736	652	788	811	831	840
	DISSOLVED OXYG	v		8.86	8.26				
	DISSOLVED OXYG			71.5	66.4				
	Hardness, Total or I	ů.	451	377	350	408	431	428	467
	ION BALANCE	%	-1.7	-2.6					
	MAJOR ANION SU		9.63	8.20	7.58	8.99	10.0	10.5	9.84
	MAJOR CATION S		9.30	7.78	7.22	8.43	8.89	8.84	9.65
	NITRATE NITROGI	0	3.39	7.44	3.06	3.82	3.97	4.19	4.22
	NITRITE NITROGE		< 0.0050	< 0.0050	0.0017	0.0020	0.0029	0.0065	0.0020
	NITROGEN, AMMO		< 0.0050	< 0.0050	< 0.0050	0.0077	0.0100	0.0059	< 0.0050
	ORTHO-PHOSPHA	J.	< 0.0010	0.0012	< 0.0010	0.0016	0.0026	0.0020	< 0.0010
	OXIDATION-REDU OXIDATION-REDU		322	-41.5 309	164.2 507	318	334	232	334
		ph units	322	7.79	7.23	310	334	232	334
	pH, Field pH, LAB	ph units	8.05	8.25	8.34	8.17	8.15	8.40	7.88
	PHOSPHORUS		0.0029	< 0.0020	0.0022	< 0.0020	0.0049	0.0055	0.0069
		mg/l	0.0029			< 0.0020	0.0049	0.0055	0.0009
	TEMPERATURE, F TOTAL DISSOLVE		597	6.2 444	6.0 409	548	520	539	598
	TOTAL DISSOLVE	J.	0.124	0.149	409 0.460	0.470	0.250	0.292	0.282
	TOTAL KJELDAHL	U U	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
	TURBIDITY, LAB	ntu	0.29	0.28	0.25	< 1.0	0.18	< 1.0	0.29
	ALUMINUM	mg/l	0.29	0.0290	0.25	0.0033	< 0.0030	< 0.0030	0.29
	ANTIMONY	mg/l	0.0040	0.00014	0.00040	0.00016	0.00016	0.00015	0.00016
	ARSENIC	mg/l	< 0.00010	< 0.00010	0.00017	0.00014	0.00010	0.00014	< 0.00010
	BARIUM	mg/l	0.109	0.0986	0.0595	0.0815	0.0815	0.0883	0.0978
	BERYLLIUM	mg/l	< 0.000020	< 0.000020	< 0.000020	< 0.000020	< 0.000020	< 0.000020	< 0.000020
	BISMUTH	mg/l	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050
	BORON	mg/l	0.025	0.017	0.015	0.020	0.022	0.023	0.025
	CADMIUM	mg/l	0.000120	0.0000647	0.000169	0.000219	0.000195	0.000214	0.000222
	CALCIUM	mg/l	119	101	82.0	95.8	103	101	110
	CHROMIUM	mg/l	0.00013	0.00015	0.00014	0.00011	0.00012	< 0.00010	0.00011
	COBALT	mg/l	0.00055	0.00020	0.00029	0.00045	0.00045	0.00049	0.00055
	COPPER	mg/l	< 0.00050	< 0.00050	< 0.00050	< 0.00050	< 0.00050	< 0.00050	< 0.00050
	IRON	mg/l	< 0.010	0.014	< 0.010	< 0.010	< 0.010	< 0.010	0.011
	LEAD	mg/l	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050
	LITHIUM	mg/l	0.0466	0.0307	0.0301	0.0403	0.0409	0.0433	0.0436
	MAGNESIUM	mg/l	37.0	32.9	33.9	39.5	39.5	38.9	44.8
	MANGANESE	mg/l	0.00512	0.00225	0.00543	0.00645	0.00797	0.00751	0.00795
	MERCURY	mg/l	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050	< 0.000050
	MOLYBDENUM	mg/l	0.000989	0.00109	0.00108	0.00104	0.000965	0.000899	0.000966
	NICKEL	mg/l	0.00208	0.00105	0.00116	0.00151	0.00150	0.00162	0.00171
	POTASSIUM	mg/l	2.21	1.88	1.76	2.08	2.11	2.22	2.25
	SELENIUM	ug/l	7.16	14.9	8.7	7.73	7.59	7.55	7.48
	SILICON	mg/l	2.86	2.41	1.99	2.18	2.32	2.31	2.39
	SILVER	mg/l	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010
	SODIUM	mg/l	5.66	5.11	3.83	4.82	4.89	4.97	5.62
	STRONTIUM	mg/l	0.201	0.172	0.137	0.173	0.174	0.182	0.194
		mg/l	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010	< 0.000010
	THALLIUM			0.00010				< 0.00010	< 0.00010
	TIN	mg/l	< 0.00010	< 0.00010	< 0.00010	< 0.00010	< 0.00010		
	TIN TITANIUM	mg/l mg/l	< 0.00010 < 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
	TIN TITANIUM TOTAL ORGANIC (mg/l mg/l Cmg/l	< 0.00010 < 0.010 0.96	< 0.010 1.00	< 0.010 1.17	< 0.010 2.72	< 0.010 1.25	< 0.010 1.35	< 0.010 1.99
	TIN TITANIUM	mg/l mg/l	< 0.00010 < 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010

Sample Type Ni Fraction Analyte Unit Result D ALUMINUM mg/l 0.0041 D ANTIMONY mg/l 0.00033 D ARSENIC mg/l 0.00033 D BARIUM mg/l 0.00020 D BERYLLIUM mg/l < 0.000020 D BORON mg/l < 0.0000134 D BORON mg/l < 0.000134 D BROMIDE mg/l < 0.0000134 D CARBON, DISSOLY 2.21 D CHLORIDE mg/l 1.04 D CHROMIUM mg/l < 0.00014 D CHROMIUM mg/l < 0.00014 D CHORIDE mg/l < 0.000014 D CARBON, DISSOLY < 0.000014 D CHORIDE mg/l < 0.000014 D KLORIDE mg/l < 0.000014 D KEQN mg/l<	0
D ALUMINUM mg/l 0.0041 D ANTIMONY mg/l 0.00033 D ARSENIC mg/l 0.00034 D BARIUM mg/l 0.00014 D BARIUM mg/l 0.00020 D BERYLLIUM mg/l 0.02005 D BORON mg/l 0.020 D BROMIDE mg/l 0.020 D BROMIDE mg/l 0.020 D CALCIUM mg/l 0.000134 D CARBON, DISSOLY mg/l 1.04 D CHROMUM mg/l 0.00010 D COBALT mg/l 0.00010 D COBALT mg/l 0.000174 D LEAD mg/l 0.00012 D LEAD mg/l 0.00012 D LEAD mg/l 0.00012 D MARGNESIUM mg/l 0.00021 D MARGNESIUM mg/l	0
D ANTIMONY mg/l 0.00033 D ARSENIC mg/l 0.00034 D BARIUM mg/l 0.00014 D BERYLLIUM mg/l < 0.000020 D BERYLLIUM mg/l < 0.000020 D BORON mg/l < 0.000134 D CALCIUM mg/l < 0.050 D CALCIUM mg/l < 0.000134 D CALCIUM mg/l < 0.00010 D CARBON, DISSOL mg/l < 0.00010 D CALORIDE mg/l < 0.000174 D COPPER mg/l < 0.000021 D IRON mg/l < 0.000021 D MAGNESIUM mg/l < 0.000021 D MAROANESE mg/l < 0	0
D ARSENIC mg/l 0.00014 D BARIUM mg/l 0.00014 D BERYLLIUM mg/l 0.00026 D BISMUTH mg/l 0.00006 D BORON mg/l 0.00006 D BORON mg/l 0.020 D BROMIDE mg/l 0.000134 D CALCIUM mg/l 0.000134 D CARBON, DISSOLV mg/l 1.04 D CHLORIDE mg/l 1.04 D CHROMIUM mg/l 0.00010 D COPPER mg/l 0.00074 D COPPER mg/l 0.00050 D IRON mg/l 0.0121 D MARGNESIUM mg/l 0.0121 D MARGANESE mg/l 0.000127 D MARGNESIUM mg/l 0.000127 D MARCURY mg/l 0.000127 D	0
D BARIUM mg/l 0.0895 D BERYLLIUM mg/l < 0.000020	0
D BERYLLIUM mg/l < 0.000020 D BISMUTH mg/l < 0.000050	0
BISMUTH mg/l < 0.00050 BORON mg/l 0.020 BROMIDE mg/l < 0.050	0
b BORON mg/l 0.020 D BROMIDE mg/l < 0.050	
b BORON mg/l 0.020 D BROMIDE mg/l < 0.050	
BROMIDE mg/l < 0.050 CADMIUM mg/l 0.0000134 CALCIUM mg/l 60.8 CARBON, DISSOLV mg/l 1.04 CHLORIDE mg/l 1.04 CHLORIDE mg/l < 0.00010	
CADMIUM mg/l 0.000134 0 CALCIUM mg/l 60.8 0 CARBON, DISSOLVmg/l 2.21 0 CHLORIDE mg/l 1.04 0 CHROMIUM mg/l < 0.00010	
D CALCIUM mg/l 60.8 D CARBON, DISSOLV mg/l 2.21 D CHLORIDE mg/l 1.04 D CHROMIUM mg/l < 0.00010	
D CARBON, DISSOLV mg/l 2.21 D CHLORIDE mg/l 1.04 D CHROMIUM mg/l <0.00010	
CHLORIDE mg/l 1.04 CHROMIUM mg/l < 0.00010	
CHROMIUM mg/l < 0.00010 COBALT mg/l < 0.00010	
COBALT mg/l < 0.00010 COPPER mg/l 0.00074 FLUORIDE mg/l 0.259 IRON mg/l < 0.010	
COPPER mg/l 0.00074 0 FLUORIDE mg/l 0.259 0 IRON mg/l < 0.010	
FLUORIDE mg/l 0.259 IRON mg/l < 0.010)
IRON mg/l < 0.010 LEAD mg/l < 0.00050)
LEAD mg/l < 0.000050 LITHIUM mg/l 0.0121 MAGNESIUM mg/l 25.1 MANGANESE mg/l 0.00021 MERCURY mg/l 0.00021 MERCURY mg/l 0.000127 MOLYBDENUM mg/l 0.00127 MOLYBDENUM mg/l 0.00127 POTASSIUM mg/l 2.70 SELENIUM ug/l 4.91 SILICON mg/l 0.633 SILVER mg/l <0.000012)
b LITHIUM mg/l 0.0121 b MAGNESIUM mg/l 25.1 b MANGANESE mg/l 0.00021 b MERCURY mg/l 0.00021 b MERCURY mg/l 0.00415 b MOLYBDENUM mg/l 0.00415 c NICKEL mg/l 0.00127 c POTASSIUM mg/l 2.70 c SELENIUM ug/l 4.91 c SELON mg/l 0.633 c SILVER mg/l <0.000012	2
LITHIUM mg/l 0.0121 MAGNESIUM mg/l 25.1 MANGANESE mg/l 0.00021 MERCURY mg/l 0.00005 MOLYBDENUM mg/l 0.00415 MOLYBDENUM mg/l 0.00415 MICKEL mg/l 0.00127 POTASSIUM mg/l 2.70 SELENIUM ug/l 4.91 SILTCON mg/l 0.633 SILVER mg/l <0.000012	
MAGNESIUM mg/l 25.1 MANGANESE mg/l 0.00021 MERCURY mg/l 0.00005 MOLYBDENUM mg/l 0.00415 NICKEL mg/l 0.00127 POTASSIUM mg/l 2.70 POTASSIUM mg/l 4.91 SELENIUM ug/l 4.91 SIL/CON mg/l 0.633 SULFAR mg/l 0.633 SULFAR mg/l 0.122 SULFATE (AS SO4) mg/l 88.9 SULFATE (AS SO4) mg/l 88.9 THALLIUM mg/l 0.000012 TIN mg/l 0.00012 URANIUM mg/l 0.00012 URANIUM mg/l 0.00012 URANIUM mg/l 0.00122 VANADIUM mg/l 0.00122 VANADIUM mg/l 0.00030 ZINC mg/l 1.0 ALKALINITY, DR Mg/l 1.0 ALKALINITY, CARB mg/l 1.0	
Description MANGANESE mg/l 0.00021 MERCURY mg/l <0.000000	
Description MERCURY mg/l < 0.00000000000000000000000000000000000	
MOLYBDENUM mg/l 0.00415 NICKEL mg/l 0.00127 POTASSIUM mg/l 2.70 SELENIUM ug/l 4.91 SILICON mg/l 0.633 SILVER mg/l 0.633 SODIUM mg/l 1.20 STRONTIUM mg/l 0.122 SULFATE (AS SO4) mg/l 88.9 THALLIUM mg/l 0.000012 TIN mg/l 0.000012 TIN mg/l 0.00012 VANADIUM mg/l 0.00012 VANADIUM mg/l 0.00122 VANADIUM mg/l <0.00050	50
NICKEL mg/l 0.00127 POTASSIUM mg/l 2.70 SELENIUM ug/l 4.91 SILICON mg/l 0.633 SILVER mg/l 0.633 SODIUM mg/l 1.20 STRONTIUM mg/l 0.122 SULFATE (AS SO4) mg/l 88.9 THALLIUM mg/l 0.000012 TIN mg/l <0.00012	50
DO POTASSIUM mg/l 2.70 DO SELENIUM ug/l 4.91 DO SILICON mg/l 0.633 DO SILVER mg/l 1.20 SODIUM mg/l 0.122 DO STRONTIUM mg/l 0.000012 DO THALLIUM mg/l 0.000012 DO TIN mg/l <0.00010	
Description SELENIUM ug/l 4.91 Description SILICON mg/l 0.633 Description SILVER mg/l <0.000010	
SILICON mg/l 0.633 SILVER mg/l < 0.00010	
SILICON mg/l 0.633 SILVER mg/l < 0.00010	
SILVER mg/l < 0.00010 SODIUM mg/l 1.20 STRONTIUM mg/l 0.122 SULFATE (AS SO4) mg/l 88.9 THALLIUM mg/l 0.000012 TIN mg/l 0.000012 TIN mg/l < 0.00010	
SODIUM mg/l 1.20 STRONTIUM mg/l 0.122 SULFATE (AS SO4) mg/l 88.9 THALLIUM mg/l 0.000012 TIN mg/l 0.000012 TIN mg/l <0.00010	2 C
D STRONTIUM mg/l 0.122 D SULFATE (AS SO4) mg/l 88.9 D THALLIUM mg/l 0.000012 D TIN mg/l <0.00010	
SULFATE (AS SO4) mg/l 88.9 D THALLIUM mg/l 0.000012 D TIN mg/l <0.00010	
D THALLIUM mg/l 0.000012 D TIN mg/l < 0.00010	
D TIN mg/l < 0.00010 D TITANIUM mg/l < 0.010	
D TITANIUM mg/l < 0.010 D URANIUM mg/l 0.00122 D VANADIUM mg/l < 0.00050	
D URANIUM mg/l 0.00122 D VANADIUM mg/l < 0.00050	
D VANADIUM mg/l < 0.00050 ZINC mg/l < 0.0030	
D ZINC mg/l < 0.0030 I ACIDITY TO pH 8.3 mg/l < 1.0	
A ACIDITY TO pH 8.3 mg/l < 1.0 I ALKALINITY, BICA mg/l 174 I ALKALINITY, CARB mg/l < 1.0	
I ALKALINITY, BICAF mg/l 174 I ALKALINITY, CARB mg/l < 1.0	
ALKALINITY, BICAF mg/l 174 A ALKALINITY, CARB mg/l < 1.0	
ALKALINITY, CARB mg/l < 1.0 ALKALINITY, HYDR mg/l < 1.0	
Image: ALKALINITY, HYDR mg/l < 1.0 Image: ALKALINITY, TOTA mg/l 174 Image: ALKALINITY, FI us/cm 427.5 Image: ALKALINITY, LI us/cm 427.5 Image: ALKALINITY, LI us/cm 477 Image: ALKALINITY, LI us/cm 427.5 Image: ALKALINITY, LI us/cm 427.5 Image: ALKALINITY, LI us/cm 420 Image: ALKALINITY, LI us/cm 5.37 Image: ALKALINITY, LI us/cm 5.22	
ALKALINITY, TOTA mg/l 174 I CONDUCTIVITY, Fl us/cm 427.5 I CONDUCTIVITY, L us/cm 477 I DISSOLVED OXYG mg/l 8.20 I DISSOLVED OXYG % 60.0 I Hardness, Total or E mg/l 255 I ION BALANCE % -1.4 MAJOR ANION SUI meq/l 5.37 5.22	
Image: CONDUCTIVITY, Fl us/cm 427.5 Image: CONDUCTIVITY, L4 us/cm 477 Image: CONDUCTIVITY, L4 us/cm 8.20 Image: CONDUCTIVITY, L4 us/cm 8.20 Image: CONDUCTIVITY, L4 us/cm 60.0 Image: CONDUCTIVITY, L4 us/cm 7 Image: CONDUCTIVITY, L4 us/cm <td></td>	
Image: CONDUCTIVITY, L4 us/cm 477 Image: DISSOLVED OXYG mg/l 8.20 Image: DISSOLVED OXYG % 60.0 Image: DISSOLVED OXYG % 61.0 Image: DISSOLVED OXYG % 61.0 </td <td></td>	
Image: Mark Mark Mark Mark Mark Mark Mark Mark	
DISSOLVED OXYG % 60.0 Hardness, Total or [mg/l] 255 ION BALANCE % -1.4 MAJOR ANION SUI meq/l 5.37 MAJOR CATION SU meq/l 5.22	
Hardness, Total or Emg/l 255 ION BALANCE % -1.4 MAJOR ANION SUI meq/l 5.37 MAJOR CATION SU meq/l 5.22	
ION BALANCE % -1.4 I MAJOR ANION SUI meq/l 5.37 I MAJOR CATION SU meq/l 5.22	
I MAJOR ANION SUI meq/l 5.37 I MAJOR CATION SUmeq/l 5.22	
MAJOR CATION SUmeq/I 5.22	
MAJOR CATION SUmeq/I 5.22	
I NITRITE NITROGEI mg/l < 0.0030	
~	
I NITROGEN, AMMO mg/l < 0.0050	
I ORTHO-PHOSPHA mg/I < 0.0010	
I OXIDATION-REDU(mv -42.6	
OXIDATION-REDU(mv 322	
pH, Field ph units 7.95	
pH, LAB ph units 8.28	
PHOSPHORUS mg/l 0.0063	
TEMPERATURE, FI deg c 2.4	
I TOTAL DISSOLVEI mg/l 282	
I TOTAL KJELDAHL mg/l 0.169	
TOTAL SUSPENDE mg/l 1.8	
TURBIDITY, LAB ntu 4.20	
ALUMINUM mg/l 0.0590	
ANTIMONY mg/l 0.00035	
ARSENIC mg/l 0.00018	
BARIUM mg/l 0.0865	
BARIUM mg/l 0.0865 BERYLLIUM mg/l < 0.000020	<u> </u>
BISMUTH mg/l < 0.000050	
BORON mg/l 0.021	
CADMIUM mg/l 0.0000242	
CALCIUM mg/l 64.8	
CHROMIUM mg/l 0.00020	
COBALT mg/l < 0.00010	
COPPER mg/l 0.00102	
IRON mg/l 0.036	
LEAD mg/l 0.000055	
LEAD mg/l 0.000055	

*** This Location is the pooled water that we spoke of yesterday that is weest of the Wastewater Cells

		J.	
Т	LITHIUM	mg/l	0.0130
Т	MAGNESIUM	mg/l	24.9
Т	MANGANESE	mg/l	0.00326
Т	MERCURY	mg/l	< 0.0000050
Т	MOLYBDENUM	mg/l	0.00426
Т	NICKEL	mg/l	0.00142
Т	POTASSIUM	mg/l	2.68
Т	SELENIUM	ug/l	4.93
Т	SILICON	mg/l	0.778
Т	SILVER	mg/l	< 0.000010
Т	SODIUM	mg/l	1.17
Т	STRONTIUM	mg/l	0.129
Т	THALLIUM	mg/l	0.000011
Т	TIN	mg/l	< 0.00010
Т	TITANIUM	mg/l	< 0.010
Т	TOTAL ORGANIC C	mg/l	2.50
Т	URANIUM	mg/l	0.00123
Т	VANADIUM	mg/l	< 0.00050
Т	ZINC	mg/l	< 0.0030

		Date Sample Type	04/07/2017 N	12/07/2017 N	ER_STPNWWELL4 18/07/2017 N	27/07/2017 N
Fraction D	Analyte ALUMINUM	Unit mg/l	Result < 0.0010	Result < 0.0010	Result < 0.0010	Result < 0.0010
D	ANTIMONY	mg/l	0.00021	0.00018	0.00021	0.00020
D	ARSENIC	mg/l	0.00012	< 0.00010	< 0.00010	0.00011
D D	BARIUM	mg/l mg/l	0.0415 < 0.000020	0.0336	0.0371	0.0362
D	BISMUTH	mg/l	< 0.000020	< 0.000020	< 0.000020	< 0.000020
D	BORON	mg/l	0.052	0.052	0.047	0.056
D	BROMIDE	mg/l	< 0.25	< 0.25	< 0.25	< 0.25
D D	CADMIUM	mg/l mg/l	0.00112 140	0.00110 135	0.00114 125	0.00128 125
D		mg/l	0.84	0.96	1.21	1.72
D	CHLORIDE	mg/l	3.8	2.9	2.6	< 2.5
D	CHROMIUM	mg/l	< 0.00010	< 0.00010	< 0.00010	< 0.00010
D D	COBALT COPPER	mg/l	0.00143 0.00088	0.00142	0.00155 0.00084	0.00146 0.00086
D	FLUORIDE	mg/l mg/l	0.36	0.38	0.39	0.00086
D	IRON	mg/l	< 0.010	< 0.010	< 0.010	< 0.010
D	LEAD	mg/l	0.000175	0.000144	0.000177	0.000205
D D	LITHIUM MAGNESIUM	mg/l mg/l	0.121 86.4	0.122 81.2	0.112 78.8	0.112 82.4
D	MANGANESE	mg/l	0.00599	0.00472	0.0158	0.00711
D	MERCURY	mg/l	< 0.0000050	< 0.0000050	< 0.0000050	< 0.0000050
D	MOLYBDENUM	mg/l	0.00126	0.00131	0.00133	0.00140
D	NICKEL	mg/l	0.00431	0.00411	0.00466	0.00424
D D	POTASSIUM SELENIUM	mg/l ug/l	4.02 2.03	4.00 1.28	3.98 0.968	4.05 0.605
D	SILICON	mg/l	1.82	1.73	1.81	1.84
D	SILVER	mg/l	< 0.000010	< 0.000010	< 0.000010	< 0.000010
D	SODIUM	mg/l	6.74	6.80	7.12	7.73
D	STRONTIUM	mg/l	0.267	0.253	0.248	0.245
D D	SULFATE (AS SO4) THALLIUM	mg/l mg/l	337 0.000041	293 0.000044	280 0.000042	252 0.000055
D	TIN	mg/l	< 0.00010	< 0.00010	< 0.00010	< 0.00010
D	TITANIUM	mg/l	< 0.010	< 0.010	< 0.010	< 0.010
D	URANIUM	mg/l	0.0136	0.0121	0.0110	0.0113
D	VANADIUM	mg/l	< 0.00050	< 0.00050	< 0.00050	< 0.00050
D		mg/l	0.0083	0.0076	0.0075	0.0089
N N	ACIDITY TO pH 8.3 ALKALINITY, BICAR	, and a second s	4.9 419	4.8 421	8.9 388	3.0 405
N	ALKALINITY, CARB	v	< 1.0	< 1.0	< 1.0	< 1.0
N	ALKALINITY, HYDR	•	< 1.0	< 1.0	< 1.0	< 1.0
N	ALKALINITY, TOTA	mg/l	419	421	388	405
N	Cation - Anion Balar		-3.8	-3.3	-2.3	0
N	CONDUCTIVITY, LA		1230	1170	1120	1090
N N	Hardness, Total or D MAJOR ANION SUM		707 15.7	671 14.7	637 13.8	651 13.4
N	MAJOR CATION SU		14.5	13.8	13.1	13.4
N	NITRATE NITROGE		2.05	1.66	1.35	0.934
Ν	NITRITE NITROGEI		< 0.0050	< 0.0050	< 0.0050	< 0.0050
N	NITROGEN, AMMO	v	< 0.0050	0.0064	< 0.0050	< 0.0050
N N	ORTHO-PHOSPHA OXIDATION-REDU	•	< 0.0010 325	0.0021 339	0.0013 350	0.0012 344
N	pH, LAB	ph units	7.62	8.06	8.25	7.72
N	PHOSPHORUS	mg/l	0.0034	0.0054	0.0056	0.0235
N	TOTAL DISSOLVED	mg/l	913	817	745	711
N		mg/l	0.193	0.250	0.123	0.102
N	TOTAL SUSPENDE	Ť	1.5	< 1.0	< 1.0	1.5
N T	TURBIDITY, LAB	ntu ma/l	0.66	0.59	0.46	0.94
<u>і</u> Т	ALUMINUM ANTIMONY	mg/l mg/l	0.0056 0.00020	0.0036 0.00021	< 0.0030 0.00020	0.0101 0.00020
Т	ARSENIC	mg/l	0.00017	0.00017	0.00017	0.00013
Т	BARIUM	mg/l	0.0396	0.0342	0.0357	0.0345
T	BERYLLIUM	mg/l	< 0.000020	< 0.000020	< 0.000020	< 0.000020
 	BISMUTH	mg/l	< 0.000050	< 0.000050	< 0.000050	< 0.000050
<u>г</u>	BORON CADMIUM	mg/l mg/l	0.053 0.00118	0.054 0.000989	0.051 0.00110	0.058 0.00122
· T	CALCIUM	mg/l	138	133	124	123
Τ	CHROMIUM	mg/l	< 0.00010	< 0.00010	< 0.00010	< 0.00010
Т	COBALT	mg/l	0.00170	0.00185	0.00164	0.00145
T	COPPER	mg/l	0.00083	0.00072	0.00084	0.00086
 		mg/l	0.016	< 0.010	< 0.010	0.019 0.000222
<u> -</u> T	LEAD LITHIUM	mg/l mg/l	0.000196 0.118	0.000194 0.118	0.000194	0.000222
T	MAGNESIUM	mg/l	80.6	83.9	79.5	80.5
Τ	MANGANESE	mg/l	0.0143	0.00968	0.0172	0.00962
Т	MERCURY	mg/l	< 0.0000050	< 0.000050	< 0.000050	< 0.000050
T	MOLYBDENUM	mg/l	0.00122	0.00130	0.00140	0.00140
<u>і</u> т		mg/l	0.00441 3.77	0.00426	0.00459 4.12	0.00414 3.86
Т	POTASSIUM SELENIUM	mg/l ug/l	3.77	3.99 1.24	4.12 0.888	0.536
T	SILICON	mg/l	1.83	1.84	1.82	1.86
Т	SILVER	mg/l	< 0.000010	< 0.000010	< 0.000010	< 0.000010
Τ	SODIUM	mg/l	6.41	6.96	7.05	7.28
T	STRONTIUM	mg/l	0.262	0.247	0.244	0.239
 		mg/l	0.000035	0.000038	0.000042	0.000051
 	TIN TITANIUM	mg/l	< 0.00010	< 0.00010	< 0.00010	< 0.00010
<u>і</u> Гт	TOTAL ORGANIC C	mg/l mg/l	< 0.010 1.00	< 0.010 1.12	< 0.010 1.41	< 0.010 1.61
Т	URANIUM	mg/l	0.0141	0.0125	0.0113	0.0114
Т	VANADIUM	mg/l	< 0.00050	< 0.00050	0.00058	< 0.00050
	ZINC	mg/l	0.0089	0.0077	0.0078	0.0083

		Date Sample Type	04/07/2017 N	12/07/2017 N	AFR_STPNWWELL5. 18/07/2017 N	27/07/2017 N
Fraction D	Analyte ALUMINUM	Unit ma/l	Result 0.0018	Result 0.0018	Result 0.0027	Result 0.0036
D	ANTIMONY	mg/l	0.00024	0.00021	0.00023	0.00023
D	ARSENIC	mg/l	0.00023	0.00026	0.00031	0.00032
D	BARIUM	mg/l	0.143	0.137	0.182	0.167
D	BERYLLIUM	mg/l	< 0.000020	< 0.000020	< 0.000020	< 0.000020
D D	BISMUTH BORON	mg/l mg/l	< 0.000050 0.035	< 0.000050 0.036	< 0.000050 0.027	< 0.000050 0.028
D	BROMIDE	mg/l	< 0.25	< 0.25	< 0.25	< 0.25
D	CADMIUM	mg/l	0.00106	0.000958	0.000798	0.000796
D	CALCIUM	mg/l	174	172	172	171
D	CARBON, DISSOL		3.80	4.04	6.10	6.89
D	CHLORIDE	mg/l	3.5	2.6	< 2.5	< 2.5
D D	CHROMIUM COBALT	mg/l mg/l	< 0.00010 0.00261	< 0.00010 0.00280	< 0.00010 0.00301	< 0.00010 0.00294
D	COPPER	mg/l	0.00130	0.00115	0.00132	0.00129
D	FLUORIDE	mg/l	0.22	0.24	0.16	0.19
D	IRON	mg/l	0.016	0.017	0.037	0.037
D	LEAD	mg/l	0.000103	< 0.000050	0.000088	< 0.000050
D		mg/l	0.0850	0.0874	0.0570	0.0526
D D	MAGNESIUM MANGANESE	mg/l mg/l	75.2 0.963	74.0 1.30	59.7 1.31	58.8 1.26
D	MERCURY	mg/l	< 0.0000050	< 0.0000050	< 0.0000050	< 0.0000050
D	MOLYBDENUM	mg/l	0.00186	0.00203	0.00197	0.00232
D	NICKEL	mg/l	0.0103	0.0104	0.00956	0.00964
D	POTASSIUM	mg/l	2.54	2.53	2.36	2.04
D	SELENIUM	ug/l	0.505	0.447	0.389	0.379
D	SILICON	mg/l	2.71	2.60	2.88	2.64
D D	SILVER SODIUM	mg/l mg/l	< 0.000010 5.96	< 0.000010 5.91	< 0.000010 5.28	< 0.000010 5.38
D	STRONTIUM	mg/l	5.96 0.255	0.248	5.28 0.230	5.38 0.224
D	SULFATE (AS SO4)		342	310	279	272
D	THALLIUM	mg/l	0.000036	0.000039	0.000032	0.000032
D	TIN	mg/l	< 0.00010	< 0.00010	< 0.00010	< 0.00010
D	TITANIUM	mg/l	< 0.010	< 0.010	< 0.010	< 0.010
D	URANIUM	mg/l	0.00721	0.00624	0.00456	0.00440
D	VANADIUM	mg/l	< 0.00050	< 0.00050	< 0.00050	< 0.00050
D	ZINC	mg/l	0.0142	0.0125	0.0121	0.0118
N	ACIDITY TO pH 8.3	v	9.2	11.4	17.8	6.6
N N	ALKALINITY, BICAR ALKALINITY, CARB		460 < 1.0	456 < 1.0	432 < 1.0	431 < 1.0
N	ALKALINITY, HYDR		< 1.0	< 1.0	< 1.0	< 1.0
N	ALKALINITY, TOTA		460	456	432	431
N	Cation - Anion Balar	· ·	-3.9	-2.0	-2.2	-2.1
N	CONDUCTIVITY, LA		1290	1260	1170	1130
Ν	Hardness, Total or I		744	734	676	669
N	MAJOR ANION SU		16.5	15.7	14.5	14.3
N	MAJOR CATION SU		15.2	15.0	13.8	13.7
N	NITRATE NITROGE	v	0.515	0.355	0.422	0.130
N N		v	0.0058	0.0050	< 0.0050	< 0.0050
N	NITROGEN, AMMO ORTHO-PHOSPHA	- V	0.0080 0.0016	0.0111 0.0023	0.0136 0.0034	0.0082 0.0025
N	OXIDATION-REDU		255	276	350	288
N	pH, LAB	ph units	7.36	7.91	8.28	7.49
N	PHOSPHORUS	mg/l	0.0024	< 0.0040	0.0102	0.0143
Ν	TOTAL DISSOLVED	mg/l	926	904	820	797
N	TOTAL KJELDAHL	v	0.253	0.344	0.250	0.250
Ν	TOTAL SUSPENDE	, v	1.3	1.9	1.0	1.5
N	TURBIDITY, LAB	ntu ma//	1.62	1.14	1.55	1.91
। T	ALUMINUM ANTIMONY	mg/l	0.0132 0.00024	0.0132	0.0104 0.00024	0.0188 0.00027
<u>г</u>	ARSENIC	mg/l mg/l	0.00024	0.00025	0.00024	0.00027
T	BARIUM	mg/l	0.138	0.138	0.157	0.166
Т	BERYLLIUM	mg/l	< 0.000020	< 0.000020	< 0.000020	< 0.000020
Τ	BISMUTH	mg/l	< 0.000050	< 0.000050	< 0.000050	< 0.000050
Т	BORON	mg/l	0.035	0.036	0.028	0.030
T	CADMIUM	mg/l	0.00102	0.000922	0.000813	0.000917
<u> </u> 	CALCIUM	mg/l	172	169	165	168
 		mg/l	0.133	< 0.00010	< 0.00010	0.00013
1 T	COBALT COPPER	mg/l	0.00316 0.00703	0.00361 0.00126	0.00282	0.00467 0.00145
<u>г</u>	IRON	mg/l mg/l	2.10	0.00126	0.00136	0.00145
T	LEAD	mg/l	0.000114	0.000116	0.000088	0.000108
Т	LITHIUM	mg/l	0.0809	0.0824	0.0547	0.0543
Т	MAGNESIUM	mg/l	68.6	72.7	54.6	58.1
T	MANGANESE	mg/l	0.990	1.38	1.17	1.69
Т	MERCURY	mg/l	0.0000058	< 0.0000050	< 0.000050	< 0.0000050
T	MOLYBDENUM	mg/l	0.00441	0.00214	0.00204	0.00239
। -	NICKEL	mg/l	0.0122	0.0109	0.00923	0.0116
 		mg/l	2.37	2.41	2.10	1.98
1 T	SELENIUM	ug/l	0.483 2.75	0.377 2.71	0.385 2.78	0.37 2.76
<u>і</u> Т	SILICON SILVER	mg/l mg/l	2.75	2.71	< 0.000010	< 0.000010
<u>г</u>	SODIUM	mg/l	< 0.000010 5.41	< 0.000010 5.81	4.75	< 0.000010 5.16
<u>.</u> T	STRONTIUM	mg/l	0.245	0.240	0.222	0.223
Т	THALLIUM	mg/l	0.000033	0.000041	0.000031	0.000048
Т	TIN	mg/l	< 0.00010	< 0.00010	< 0.00010	< 0.00010
Τ	TITANIUM	mg/l	< 0.010	< 0.010	< 0.010	< 0.010
Т	TOTAL ORGANIC C	mg/l	4.27	4.48	6.63	7.61
Т	URANIUM	mg/l	0.00772	0.00645	0.00471	0.00462
T T	VANADIUM	mg/l	0.00103	0.00051	0.00068	< 0.00050
	ZINC	mg/l	0.0136	0.0134	0.0116	0.0137

		Location Date Sample Type	04/07/2017	GAFR_STPNWWELL6 12/07/2017 N	18/07/2017 N	AFR_STPNWWELL6A 27/07/2017 N
Fraction	Analyte	Unit	Result	Result	Result	Result
D D	ALUMINUM ANTIMONY	mg/l mg/l	0.0015 0.00017	0.0013 0.00017	< 0.0010 0.00017	0.0017 0.00018
D	ARSENIC	mg/l	0.00013	0.00015	0.00015	0.00013
D	BARIUM	mg/l	0.0766	0.0685	0.0591	0.0615
D	BERYLLIUM	mg/l	< 0.000020	< 0.000020	< 0.000020	< 0.000020
D D	BISMUTH	mg/l	< 0.000050 0.050	< 0.000050	< 0.000050	< 0.000050 0.052
D	BORON BROMIDE	mg/l mg/l	< 0.25	0.049 < 0.25	0.047 < 0.25	< 0.25
D	CADMIUM	mg/l	0.00113	0.000965	0.00103	0.000973
D	CALCIUM	mg/l	143	142	129	128
D		mg/l	1.83	2.72	2.03	2.68
D	CHLORIDE	mg/l	3.4	2.5	< 2.5	< 2.5
D D	CHROMIUM COBALT	mg/l	< 0.00010 0.00168	< 0.00010 0.00168	< 0.00010 0.00166	< 0.00010 0.00171
D	COPPER	mg/l mg/l	0.00168	0.00168	0.00166	0.00058
D	FLUORIDE	mg/l	0.35	0.40	0.40	0.48
D	IRON	mg/l	< 0.010	< 0.010	< 0.010	0.011
D	LEAD	mg/l	0.000118	0.000110	0.000150	0.000141
D		mg/l	0.117	0.117	0.111	0.109
D D	MAGNESIUM MANGANESE	mg/l mg/l	87.7 0.173	83.9 0.125	81.1 0.137	81.3 0.116
D	MERCURY	mg/l	< 0.0000050	< 0.0000050	< 0.0000050	< 0.0000050
D	MOLYBDENUM	mg/l	0.00127	0.00132	0.00128	0.00139
D	NICKEL	mg/l	0.00425	0.00414	0.00359	0.00381
D	POTASSIUM	mg/l	3.63	3.76	4.10	3.68
D	SELENIUM	ug/l	0.727	0.625	0.598	0.571
D D	SILICON SILVER	mg/l mg/l	1.93 < 0.000010	1.84	1.97	1.89 < 0.000010
D	SODIUM	mg/l	6.79	6.79	7.31	7.61
D	STRONTIUM	mg/l	0.274	0.263	0.255	0.248
D	SULFATE (AS SO4	mg/l	345	316	274	268
D	THALLIUM	mg/l	0.000031	0.000034	0.000028	0.000033
D	TIN	mg/l	< 0.00010	< 0.00010	< 0.00010	< 0.00010
D		mg/l	< 0.010	< 0.010 0.0123	< 0.010	< 0.010
D	URANIUM VANADIUM	mg/l	0.0137		0.0117	0.0121
D D	ZINC	mg/l mg/l	< 0.00050 0.0095	< 0.00050	< 0.00050 0.0077	< 0.00050 0.0082
N	ACIDITY TO pH 8.3	ĕ	3.5	7.7	9.0	1.6
N	ALKALINITY, BICA	v	426	399	383	421
N	ALKALINITY, CARE		< 1.0	< 1.0	20.8	< 1.0
Ν	ALKALINITY, HYDF		< 1.0	< 1.0	< 1.0	< 1.0
Ν	ALKALINITY, TOTA		426	399	403	421
N	Cation - Anion Bala		-3.6	-1.1	-1.1	-2.0
N N	CONDUCTIVITY, L/ Hardness, Total or I		1270 720	1200 699	1150 656	1090 654
N	MAJOR ANION SU		15.9	14.7	13.8	14.0
N	MAJOR CATION SU		14.8	14.4	13.5	13.5
N	NITRATE NITROGE	· · ·	0.780	0.726	0.586	0.288
N	NITRITE NITROGE	v	< 0.0050	< 0.0050	< 0.0050	< 0.0050
Ν	NITROGEN, AMMC	- V	< 0.0050	0.0110	0.0089	< 0.0050
N	ORTHO-PHOSPHA	- V	0.0013	0.0021	0.0016	0.0019
N	OXIDATION-REDU		279	311	351	293
N	pH, LAB PHOSPHORUS	ph units	7.59	7.72	8.37	7.66
N N	TOTAL DISSOLVE	mg/l	0.0065 881	0.0133 889	0.0083 768	0.0080 745
N		mg/l	0.192	0.250	0.118	0.090
N	TOTAL SUSPENDE	0	1.9	< 1.0	< 1.0	< 1.0
N	TURBIDITY, LAB	ntu	1.19	0.93	0.50	0.74
Т	ALUMINUM	mg/l	0.0080	0.0092	< 0.0030	0.0035
Т	ANTIMONY	mg/l	0.00016	0.00018	0.00017	0.00018
	ARSENIC	mg/l	0.00019	0.00018	0.00020	0.00016
 	BARIUM	mg/l	0.0748	0.0700	0.0571	0.0589
<u> '</u> т	BERYLLIUM BISMUTH	mg/l mg/l	< 0.000020 < 0.000050	< 0.000020 < 0.000050	< 0.000020 < 0.000050	< 0.000020 < 0.000050
т	BORON	mg/l	< 0.000050 0.049	0.050	0.048	< 0.000050 0.054
Т	CADMIUM	mg/l	0.00119	0.000974	0.00102	0.000969
Т	CALCIUM	mg/l	145	140	123	125
T	CHROMIUM	mg/l	0.00010	< 0.00010	< 0.00010	< 0.00010
Т	COBALT	mg/l	0.00180	0.00176	0.00161	0.00164
T	COPPER	mg/l	0.00060	0.00067	0.00057	0.00063
		mg/l	0.020	0.018	0.013	0.014
	LEAD LITHIUM	mg/l	0.000164 0.113	0.000148	0.000161	0.000149 0.109
μ <u>΄</u>	MAGNESIUM	mg/l mg/l	0.113 81.2	0.114 84.6	76.9	0.109 80.7
Т	MANGANESE	mg/l	0.194	0.141	0.120	0.110
Т	MERCURY	mg/l	< 0.0000050	< 0.0000050	< 0.0000050	< 0.0000050
Τ	MOLYBDENUM	mg/l	0.00133	0.00139	0.00131	0.00144
Ť	NICKEL	mg/l	0.00407	0.00420	0.00356	0.00364
т	POTASSIUM	mg/l	3.47	3.69	3.77	3.52
1 	SELENIUM	ug/l	0.669	0.565	0.592	0.543
	SILICON	mg/l	1.96	1.97	1.95	1.93
<u>।</u> न	SILVER SODIUM	mg/l	< 0.000010 6.25	< 0.000010	< 0.000010	< 0.000010 7.37
<u> -</u> т	STRONTIUM	mg/l mg/l	6.25 0.271	6.78 0.262	6.90 0.244	0.243
Т	THALLIUM	mg/l	0.271	0.202	0.244	0.243
Т	TIN	mg/l	< 0.00010	< 0.00010	< 0.00010	< 0.00010
Т	TITANIUM	mg/l	< 0.010	< 0.010	< 0.010	< 0.010
Т	TOTAL ORGANIC (2.17	3.32	2.44	2.68
Т	URANIUM	mg/l	0.0147	0.0129	0.0123	0.0123
T	VANADIUM	mg/l	< 0.00050	< 0.00050	< 0.00050	< 0.00050
Т	ZINC	mg/l	0.0089	0.0080	0.0070	0.0076

		Location Date Sample Type	15/09/2016	P FR_STPSWSEEP 28/11/2016 NP	P FR_STPSWSER 12/06/2017 NP
Fraction	Analyte	Unit	Result	Result	Result
		mg/l	< 0.0030	< 0.0030	< 0.0010
)	ANTIMONY ARSENIC	mg/l mg/l	< 0.00010 < 0.00010	< 0.00010 < 0.00010	< 0.00010 < 0.00010
)	BARIUM	mg/l	0.0820	0.0771	0.0840
	BERYLLIUM	mg/l	< 0.000020	< 0.000020	< 0.000020
	BISMUTH	mg/l	< 0.000050	< 0.000050	< 0.000050
	BORON	mg/l	0.027	0.027	0.030
)	BROMIDE CADMIUM	mg/l	< 0.25	< 0.25	< 0.25
)	CALCIUM	mg/l mg/l	0.000403 145	0.000411	0.000351
)	CARBON, DISSOLV	<u>v</u>	1.01	1.00	1.05
)	CHLORIDE	mg/l	8.24	6.84	5.9
)	CHROMIUM	mg/l	< 0.00010	< 0.00010	< 0.00010
)	COBALT	mg/l	0.00102	0.00094	0.00091
	COPPER	mg/l	< 0.00050	< 0.00050	< 0.00020
)	FLUORIDE	mg/l	0.35 < 0.010	0.30	0.23
)	IRON LEAD	mg/l mg/l	< 0.000050	< 0.000050	< 0.00050
)		mg/l	0.0998	0.0952	0.0976
)	MAGNESIUM	mg/l	80.8	73.7	78.6
)	MANGANESE	mg/l	0.693	0.421	0.355
)	MERCURY	mg/l	< 0.0000050	< 0.000050	< 0.0000050
)	MOLYBDENUM	mg/l	0.00226	0.00217	0.00211
)	NICKEL	mg/l	0.00558	0.00538	0.00483
)		mg/l	6.59	5.73	5.98
)	SELENIUM SILICON	ug/l	0.067 2.67	< 0.050 2.63	< 0.050 2.52
)	SILICON	mg/l mg/l	2.67	< 0.000010	< 0.000010
<u>, </u>	SODIUM	mg/l	6.70	6.75	6.83
)	STRONTIUM	mg/l	0.224	0.217	0.220
)	SULFATE (AS SO4)		386	358	353
)	THALLIUM	mg/l	0.000028	0.000021	0.000027
)	TIN	mg/l	< 0.00010	< 0.00010	< 0.00010
)	TITANIUM	mg/l	< 0.010	< 0.010	< 0.010
)	URANIUM	mg/l	0.00543	0.00562	0.00512
)	VANADIUM	mg/l	< 0.00050	< 0.00050	< 0.00050
)		mg/l	< 0.0030	< 0.0030	0.0024
1	ACIDITY TO pH 8.3		9.3	2.5	< 1.0
1	ALKALINITY, BICAR	0	327	333	350
l	ALKALINITY, CARB		< 1.0	< 1.0	6.4
۰ ۱	ALKALINITY, HYDR		< 1.0 327	< 1.0 333	< 1.0 356
۱ ۱	Cation - Anion Balan		521	333	-2.8
۱ <u> </u>	CONDUCTIVITY, FI			1000	1031
1	CONDUCTIVITY, LA		1200	1150	1150
1	DISSOLVED OXYG		1200	8.45	6.49
١	DISSOLVED OXYG	v		67.1	62.7
١	Hardness, Total or D	mg/l	695	656	671
J	ION BALANCE	%	-1.5	-2.7	
١	MAJOR ANION SUN		14.8	14.3	14.7
1	MAJOR CATION SU		14.4	13.6	13.9
1	NITRATE NITROGE		< 0.025	< 0.025	0.272
1	NITRITE NITROGEN		< 0.0050	< 0.0050	< 0.0050
1	NITROGEN, AMMOI	•	< 0.0050	< 0.0050	< 0.0050
J	ORTHO-PHOSPHAT	v	< 0.0010	< 0.0010	< 0.0010
J	OXIDATION-REDUC		200	-45.7	20.2
<u>.</u> 1	OXIDATION-REDUC pH, Field	ph units	326	331 7.61	493 7.41
<u> </u>	pH, LAB	ph units	7.98	8.24	8.30
I	PHOSPHORUS	mg/l	< 0.0020	0.0029	< 0.0020
I	TEMPERATURE, FI	0	< 0.0020	5.5	13.6
1	TOTAL DISSOLVED		933	839	830
J	TOTAL KJELDAHL		0.082	0.135	< 0.050
1	TOTAL SUSPENDE		3.4	2.7	< 1.0
1	TURBIDITY, LAB	ntu	0.23	0.36	0.20
-	ALUMINUM	mg/l	0.0058	0.139	< 0.0030
-	ANTIMONY	mg/l	< 0.00010	< 0.00010	< 0.00010
-	ARSENIC	mg/l	0.00011	0.00022	0.00018
	BARIUM	mg/l	0.0857	0.0986	0.0842
-	BERYLLIUM	mg/l	< 0.000020	< 0.000020	< 0.000020
	BISMUTH	mg/l	< 0.000050	< 0.000050	< 0.000050
	BORON	mg/l	0.027 0.000478	0.028	0.030
	CADMIUM CALCIUM	mg/l	0.000478	0.000809	0.000409
	CHROMIUM	mg/l mg/l	142 < 0.00010	0.00028	< 0.00010
- -	COBALT	mg/l	0.00108	0.00116	0.00102
-	COPPER	mg/l	< 0.00050	< 0.00050	< 0.00050
-	IRON	mg/l	0.088	0.716	0.038
	LEAD	mg/l	< 0.000050	0.000217	< 0.000050
	LITHIUM	mg/l	0.101	0.0973	0.0942
	MAGNESIUM	mg/l	74.2	77.7	79.3
	MANGANESE	mg/l	0.792	1.02	0.369
· · · · · · · · · · · · · · · · · · ·	MERCURY	mg/l	< 0.0000050	< 0.0000050	< 0.0000050
· · · · · · · · · · · · · · · · · · ·	MOLYBDENUM	mg/l	0.00219	0.00250	0.00210
	NICKEL	mg/l	0.00572	0.00667	0.00533
	POTASSIUM	mg/l	6.01	5.99	5.82
	SELENIUM	ug/l	0.082	0.198	0.067
	SILICON	mg/l	2.72	2.91	2.50
	SILVER	mg/l	< 0.000010	< 0.000010	< 0.000010
	SODIUM	mg/l	6.52	7.13	7.00
	STRONTIUM	mg/l	0.214	0.228	0.221
	THALLIUM	mg/l	0.000029	0.000043	0.000027
	TIN	mg/l	< 0.00010	< 0.00010	< 0.00010
	TIT & & U		< 0.010	< 0.010	< 0.010
		mg/l			4 45
	TOTAL ORGANIC C	mg/l	1.23	1.62	1.15
- - - -		•			1.15 0.00535 0.00058

ANTIMONY ARSENIC BARIUM BERYLLIUM BISMUTH BORON BROMIDE CADMIUM CALCIUM CALCIUM CARBON, DISSOLV CHLORIDE CHROMIUM COBALT COPPER FLUORIDE IRON LEAD	Sample Type Unit mg/l mg/l mg/l mg/l mg/l mg/l mg/l mg/l	Result < 0.0030 < 0.00010 < 0.00010 0.0845 < 0.000020 < 0.000050 0.026 < 0.25 0.000403 138 1.01 7.84	Result < 0.0030 < 0.00010 < 0.00010 0.105 < 0.000020 < 0.000050 0.028 < 0.25 0.000677	Result < 0.0010 < 0.00010 < 0.00010 0.111 < 0.000020 < 0.000050 0.030 < 0.25
ANTIMONY ARSENIC BARIUM BERYLLIUM BISMUTH BORON BROMIDE CADMIUM CALCIUM CARBON, DISSOLV CHLORIDE CHROMIUM COBALT COPPER FLUORIDE IRON LEAD	mg/l mg/l mg/l mg/l mg/l mg/l mg/l mg/l	< 0.00010 < 0.00010 0.0845 < 0.000020 < 0.000050 0.026 < 0.25 0.000403 138 1.01 7.84	< 0.00010 < 0.00010 0.105 < 0.000020 < 0.000050 0.028 < 0.25 0.000677	< 0.00010 < 0.00010 0.111 < 0.000020 < 0.000050 0.030 < 0.25
ARSENIC BARIUM BERYLLIUM BISMUTH BORON BROMIDE CADMIUM CALCIUM CARBON, DISSOLV CHLORIDE CHROMIUM COBALT COPPER FLUORIDE IRON LEAD	mg/l mg/l mg/l mg/l mg/l mg/l mg/l mg/l	< 0.00010 0.0845 < 0.000020 < 0.000050 0.026 < 0.25 0.000403 138 1.01 7.84	< 0.00010 0.105 < 0.000020 < 0.000050 0.028 < 0.25 0.000677	< 0.00010 0.111 < 0.000020 < 0.000050 0.030 < 0.25
BARIUM BERYLLIUM BISMUTH BORON BROMIDE CADMIUM CALCIUM CARBON, DISSOLV CHLORIDE CHROMIUM COBALT COPPER FLUORIDE IRON LEAD	mg/l mg/l mg/l mg/l mg/l mg/l mg/l mg/l	0.0845 < 0.000020 < 0.000050 0.026 < 0.25 0.000403 138 1.01 7.84	0.105 < 0.000020 < 0.000050 0.028 < 0.25 0.000677	0.111 < 0.000020 < 0.000050 0.030 < 0.25
BISMUTH BORON BROMIDE CADMIUM CALCIUM CARBON, DISSOLV CHLORIDE CHROMIUM COBALT COPPER FLUORIDE IRON LEAD	mg/l mg/l mg/l mg/l mg/l mg/l mg/l mg/l	< 0.000050 0.026 < 0.25 0.000403 138 1.01 7.84	< 0.000050 0.028 < 0.25 0.000677	< 0.000050 0.030 < 0.25
BORON BROMIDE CADMIUM CALCIUM CARBON, DISSOLV CHLORIDE CHROMIUM COBALT COPPER FLUORIDE IRON LEAD	mg/l mg/l mg/l mg/l mg/l mg/l mg/l	0.026 < 0.25 0.000403 138 1.01 7.84	0.028 < 0.25 0.000677	0.030 < 0.25
BROMIDE CADMIUM CALCIUM CARBON, DISSOLV CHLORIDE CHROMIUM COBALT COPPER FLUORIDE IRON LEAD	mg/l mg/l mg/l mg/l mg/l mg/l mg/l	< 0.25 0.000403 138 1.01 7.84	< 0.25 0.000677	< 0.25
CADMIUM CALCIUM CARBON, DISSOLV CHLORIDE CHROMIUM COBALT COPPER FLUORIDE IRON LEAD	mg/l mg/l mg/l mg/l mg/l mg/l	0.000403 138 1.01 7.84	0.000677	
CARBON, DISSOLV CHLORIDE CHROMIUM COBALT COPPER FLUORIDE IRON LEAD	mg/l mg/l mg/l mg/l	1.01 7.84		0.000778
CHLORIDE CHROMIUM COBALT COPPER FLUORIDE IRON LEAD	mg/l mg/l mg/l	7.84	136	137
CHROMIUM COBALT COPPER FLUORIDE IRON LEAD	mg/l mg/l	-	0.79 6.25	0.91 7.2
COBALT COPPER FLUORIDE IRON LEAD	mg/l	< 0.00010	< 0.00010	< 0.00010
FLUORIDE IRON LEAD	mg/l	0.00127	0.00140	0.00143
IRON LEAD		< 0.00050	< 0.00050	< 0.00020
LEAD	mg/l	0.38	0.35	0.14
	mg/l mg/l	< 0.010 < 0.000050	< 0.010 < 0.000050	< 0.010 < 0.000050
LITHIUM	mg/l	0.0975	0.0990	0.0989
	mg/l	76.8	70.1	78.2
	mg/l	0.579	0.879	0.828
	mg/l	< 0.0000050	< 0.000050	< 0.000050
	mg/l mg/l	0.00264 0.00562	0.00287	0.00259 0.00654
	mg/l	6.58	5.66	5.97
SELENIUM	ug/l	< 0.050	0.054	0.216
SILICON	mg/l	3.06	2.63	2.79
		< 0.000010	< 0.000010	< 0.000010
		-		6.53 0.212
				206
	-	0.000029	0.000058	0.000055
TIN	mg/l	< 0.00010	< 0.00010	< 0.00010
	mg/l	< 0.010	< 0.010	< 0.010
				0.00618
	-			< 0.00050 0.0024
-				< 1.0
		342		386
		< 1.0	< 1.0	< 1.0
		< 1.0	< 1.0	< 1.0
	· ·	342	356	386
			070	4.4 1007
		1160		1120
		1100		2.67
	•		19.7	23.1
				663
		-		10.0
				12.6 13.7
				4.96
		< 0.0050	< 0.0050	0.0053
		0.0132	0.0433	0.0432
	•	< 0.0010	0.0014	< 0.0010
		0.07		80.5
		307		464 7.19
		8 30		8.24
			-	< 0.0020
	deg c		8.0	8.5
		891	777	790
				0.053
	*			< 1.0 0.13
		-		< 0.0030
ANTIMONY	-	< 0.0000	< 0.00010	< 0.00010
		0.00013	< 0.00010	0.00016
BARIUM	mg/l	0.0924	0.135	0.112
		< 0.000020	< 0.000020	< 0.000020
				< 0.000050
				0.029 0.000788
		144	142	134
	-	< 0.00010	< 0.00010	< 0.00010
COBALT	mg/l	0.00144	0.00199	0.00145
	-	< 0.00050	< 0.00050	< 0.00050
	· ·			< 0.010
				< 0.000050 0.0945
	-	71.2	76.5	79.2
		0.823	1.89	0.853
MERCURY	mg/l	< 0.0000050	< 0.0000050	< 0.0000050
		0.00275	0.00312	0.00264
		0.00613		0.00699
	-			5.80 0.221
	•			0.221 2.84
				< 0.000010
		6.04	7.16	6.63
		0.215	0.220	0.212
THALLIUM	mg/l	0.000031	0.000081	0.000057
		< 0.00010	< 0.00010	< 0.00010
				< 0.010
	U U			1.12 0.00647
	0			0.00069
	SELENIUM SILICON SILVER SODIUM STRONTIUM SULFATE (AS SO4) THALLIUM TIN TITANIUM URANIUM VANADIUM ZINC ACIDITY TO pH 8.3 ALKALINITY, BICAR ALKALINITY, BICAR ALKALINITY, BICAR ALKALINITY, CARB ALKALINITY, HYDR ALKALINITY, TOTAL Cation - Anion Balan CONDUCTIVITY, FIE CONDUCTIVITY, LA DISSOLVED OXYGE Hardness, Total or D ION BALANCE MAJOR ANION SUW MAJOR CATION SU MAJOR CATION SU MAJOR CATION SU NITRATE NITROGEN NITROGEN, AMMOI ORTHO-PHOSPHAT OXIDATION-REDUC OXIDATION-REDUC OXIDATION-REDUC OXIDATION-REDUC OXIDATION-REDUC DISSOLVED TOTAL KJELDAHL N TOTAL DISSOLVED TOTAL KJELDAHL N TOTAL SUSPENDEI TURBIDITY, LAB ALUMINUM ANTIMONY ARSENIC BARIUM BERYLLIUM BISMUTH BORON CADMIUM CALCIUM CHROMIUM COBALT COPPER IRON LEAD LITHIUM MAGNESIUM MAGNESIUM SELENION SILICON	SELENIUM ug/l SIL/CON mg/l SIL/CON mg/l SILVER mg/l SODIUM mg/l STRONTIUM mg/l STRONTIUM mg/l THALLIUM mg/l TITANIUM mg/l URANIUM mg/l VANADIUM mg/l ALKALINITY, BICAR mg/l ALKALINITY, BICAR mg/l ALKALINITY, BICAR mg/l ALKALINITY, TOTAL mg/l Cation - Anion Balan % CONDUCTIVITY, LAus/cm DISSOLVED OXYGE mg/l DISSOLVED OXYGE mg/l ION BALANCE % MAJOR ANION SUM meq/l MAJOR ANION SUM meq/l MAJOR ANION SUM meq/l NITRITE NITROGE mg/l NITROGEN, AMMOT mg/l OXIDATION-REDUC mv OXIDATION-REDUC mv OXIDATION-REDUC mv PH, Field ph units pH, LAB ph units pH, SEIDON mg/l TOTAL SUSPENDE mg/l TOTAL SUSPENDE mg/l TOTAL SUSPENDE mg/l TOTAL SUSPENDE mg/l CADMIUM ARSENIC mg/l <t< td=""><td>SELENIUM ug/l < 0.050</td> SILICON mg/l 3.06 SILVER mg/l < 0.000010</t<>	SELENIUM ug/l < 0.050	SELEINUM ug/l < 0.050



APPENDIX F

Tailings Storage Facility Registry



Mine Name: Fording River Operations Permit No: No. C-3 (and amendments)

General Mine Information				
Owner/company	Teck Resources Ltd.			
Nearest community	Elkford			
Region	Elk Valley / East Kootenay			
Ore(s) mined	Coal			
Mine operational status	Operational			
Number of tailings impoundments	4			

13F Docume	intation
Date of last DSI	3 October 2017
Date of last DSR	November 2014
Date of next DSR	2019
Date of OMS update	December 2015, currently under review
Date of EPRP update	Draft September 2017, under review
Date of EPRP test	December 2015
Date of dam breach and inundation study	28 November 2014
Tailings Management system (name)	FRO Tailings Management System
Tailings management system (last audit)	Legal compliance audit Aug. 2016
TSF risk assessment last reviewed	To be completed by end of 2017
Water balance and water management plan (last update)	2017
Contract of the second states	2012 8 2012

TSF Information	
TSF name	South Tailings Pond
TSF operating status	Active, in use
Year facility was last used (if closed)	Currently in use
Number of dams	2
Engineer of record	John Cunning (Golder Associates Ltd.)
TSF qualified person	Heather Brickner
Spillway present	no
Spillway date of last maintenance	n/a
Quantitative Performance Objectives (QPOs)	yes
Volume of impoundment	12.1 million m ³

Dam Information		
Dam name	Main Dam	
Height of dam	35 m	
Consequence classification	Very High	
Slope	1.5 to 1.75H : 1V	
minimum factor of safety (long term steady state)	1.5	
minimum factor of safety (pseudo-static)	1.2	
Permitted elevation	1,637.85 m	
Current elevation	1,637.85 m	
Seismic design (AEP)	1/2 between 1/2,475 and 1/10,000 or Maximum Credible Earthquate	
Flood design (AEP)	2/3 between 1/1,000 and PMF event	
Type of dam construction (upstream, downstream, centre)	downstream	
Type of dam core (till core, rock fill, cyclone sand, etc.)	till core	

Dam Information		
Dam name	West Dam	
Height of dam	35 m	
Consequence classification	Very High	
Slope	1.5 to 1.75H : 1V	
minimum factor of safety (long term steady state)	1.6	
minimum factor of safety (pseudo-static)	1.0	
Permitted elevation	1,637.85 m to 1640 m	
Current elevation	1,637.85 m to 1640 m	
Seismic design (AEP)	1/2 between 1/2,475 and 1/10,000 or Maximum Credible Earthquate	
Flood design (AEP)	2/3 between 1/1,000 and PMF event	
Type of dam construction (upstream, downstream, centre)	downstream	
Type of dam core (till core, rock fill, cyclone sand, etc.)	till core	

Notes: Elevations reported in the Elk Valley Elevation Datum.

Sources: 2017 Dam Safety Inspection for North Tailings Pond and South Tailings Pond (Golder, March 2018) C-3 Permit Amendments

C-3 Permit Amendments	
General Mine Information	
Owner/company	
Nearest community	name of closest community
Region	mining region name - Southcoast, Northwest, Northeast, Kootenays, Okanagan
Ore(s) mined	
Mine operational status	Operating, Closed or Care and Maintenance
Number of tailings impoundments	

Date of lists as-built **TSF nome TSF operating TSF**

Valume of impoundment Dam norma Height of dam Consequence dassification Stope minimum factor of safety (long-term steady state) minimum factor of safety (long-term steady state) minimum factor of safety (long-term steady state) Permitted elevation Construction (long-term steady state) For defension (long-term steady state) Type of dam construction (upstream, downstream, centre) Type of dam core (till core, rock fill, cyclone sand, etc.)

du/mm/yyyy of lasi inspection performed du/mm/yyyy of last inspection performed yyy of nex LSON. TEO SEN sow due every 5 years as per Code requirements du/mm/yyyy when DSR last updated du/mm/yyyy when sais CRPR test du/mm/yyyy when sais CRPR test du/mm/yyyy when last rok assessment completed du/mm/yyyy when ast rok assessment completed du/mm/yyyy when as-built completed for TSG

name of TSF please fill in one box per TSF on site current status of TSF - operating or closed. If intention is to use facility in future, please include projected date of re-start wy Tba TSF tast received tailings manuel of tappener and Record for the TSF name of TSF qualified person yes or no ad/mm/kywy of last maintenance yes or no (include in OMS manual and DS) volume of impoundment in cubic meters

provide dam name if applicable - please fill in one box per dam on mine site current hight of dam in m (meaured too of dops to crest of dam) consequence (calibration of the dam Maximum dops angle (ex, 7±10) (minrum FOG (puence-static analyse) (minrum FOG (puence-static analyse) provide hightest permitted elevation of the dam in m provide surrent elevation of the dam in m Annual Caccedance Probability (Fosimic Gesign) upstream, downstream or centre till care, no.011, (culow same or other

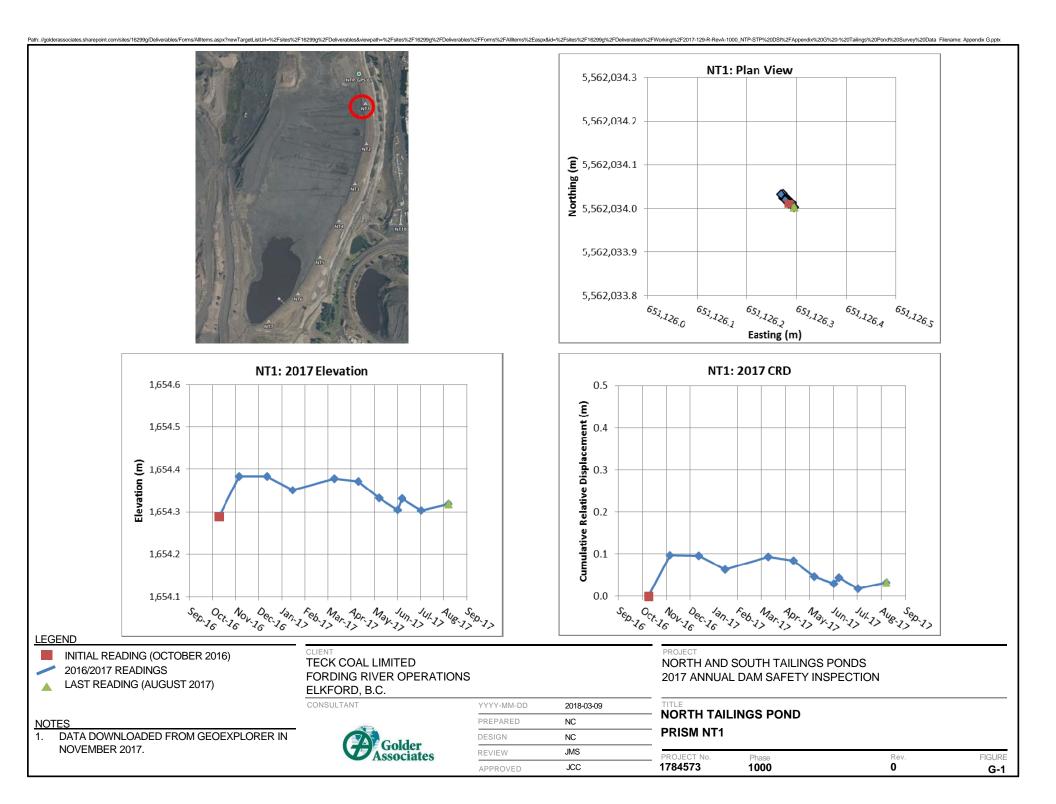
TSF Information	
TSF name	North Tailings Pond
TSF operating status	Inactive
Year facility was last used (if closed)	2006
Number of dams	1
Engineer of record	John Cunning (Golder Associates Ltd.)
TSF qualified person	Heather Brickner
Spillway present	no
Spillway date of last maintenance	n/a
Quantitative Performance Objectives (QPOs)	yes
Volume of impoundment	3.8 million m ³
Dam name	North Tailings Pond Dam
Height of dam	24 m
Consequence classification	Very High
Slope	
minimum factor of safety (long term steady state)	1.5 to 1.75H : 1V
minimum factor of safety (pseudo-static)	1.5 to 1.75H : 1V 1.5
Permitted elevation	1.5
Permitted elevation Current elevation	15 12
	1.5 1.2 1653.09 m 1,652.6 m
Current elevation Seismic design (AEP)	1.5 1.2 1653.09 m
Current elevation	1.5 1.2 1653.09 m 1,652.6 m 1/2 between 1/2,475 and 1/20,000 or Maximum Cardible Earthquate

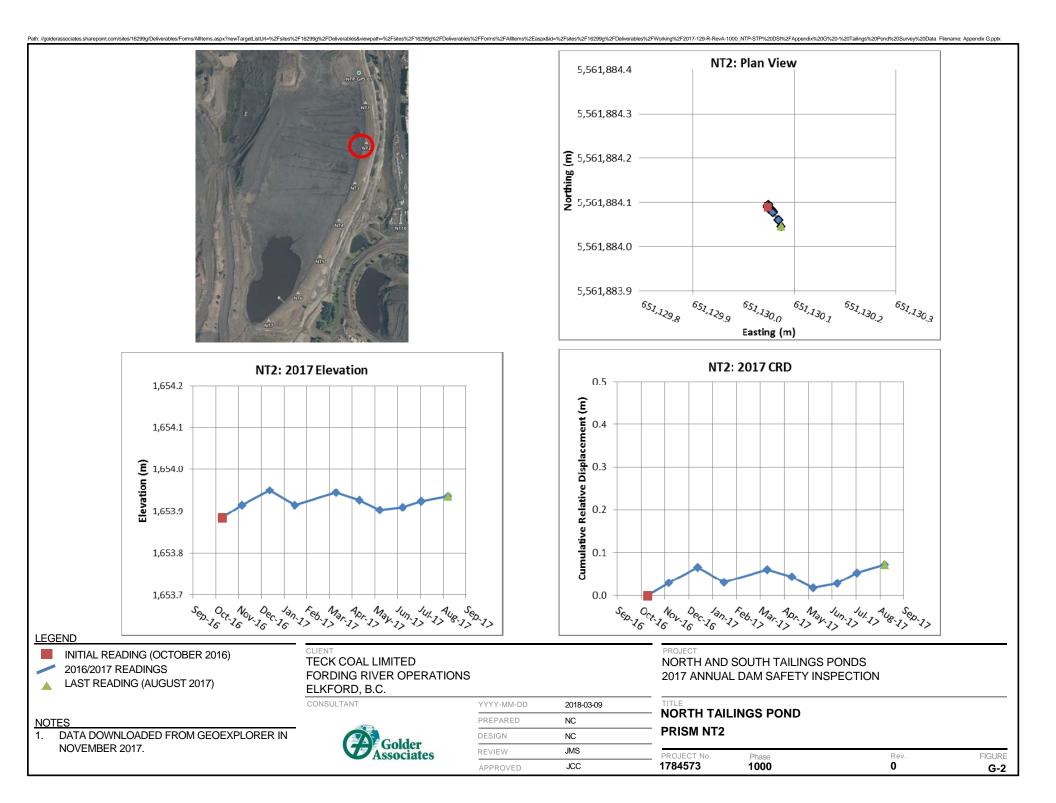


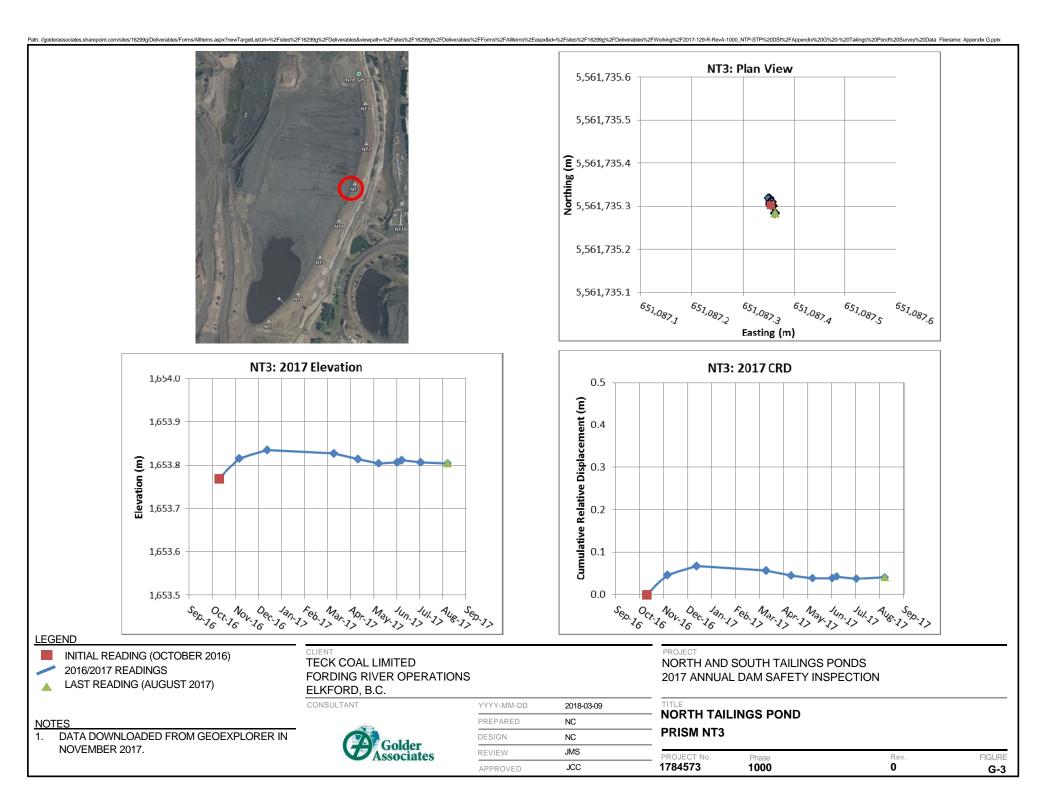
APPENDIX G

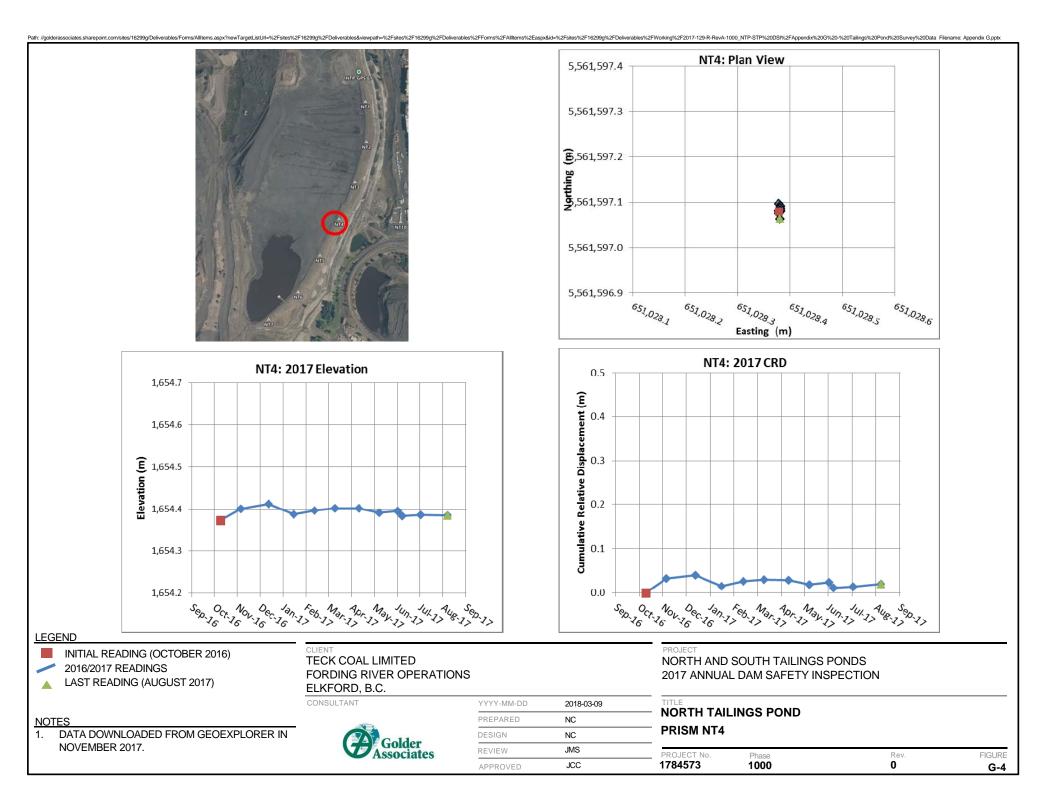
Tailings Pond Survey Data (Prisms and GPS)

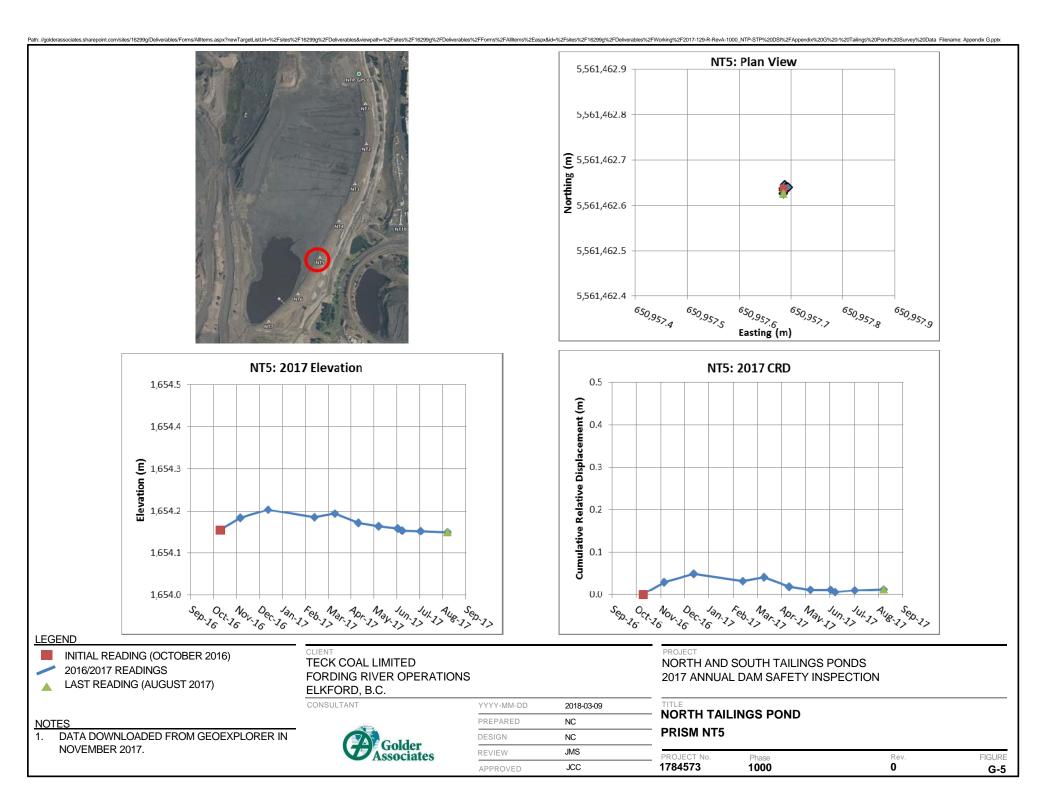


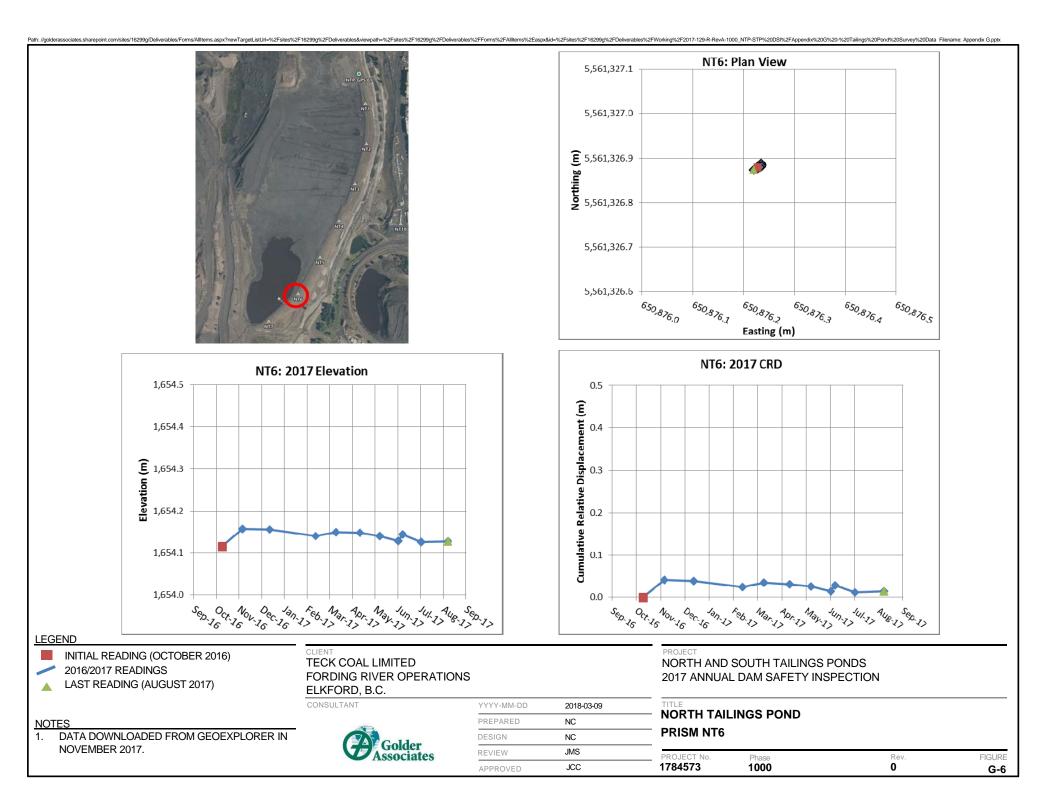


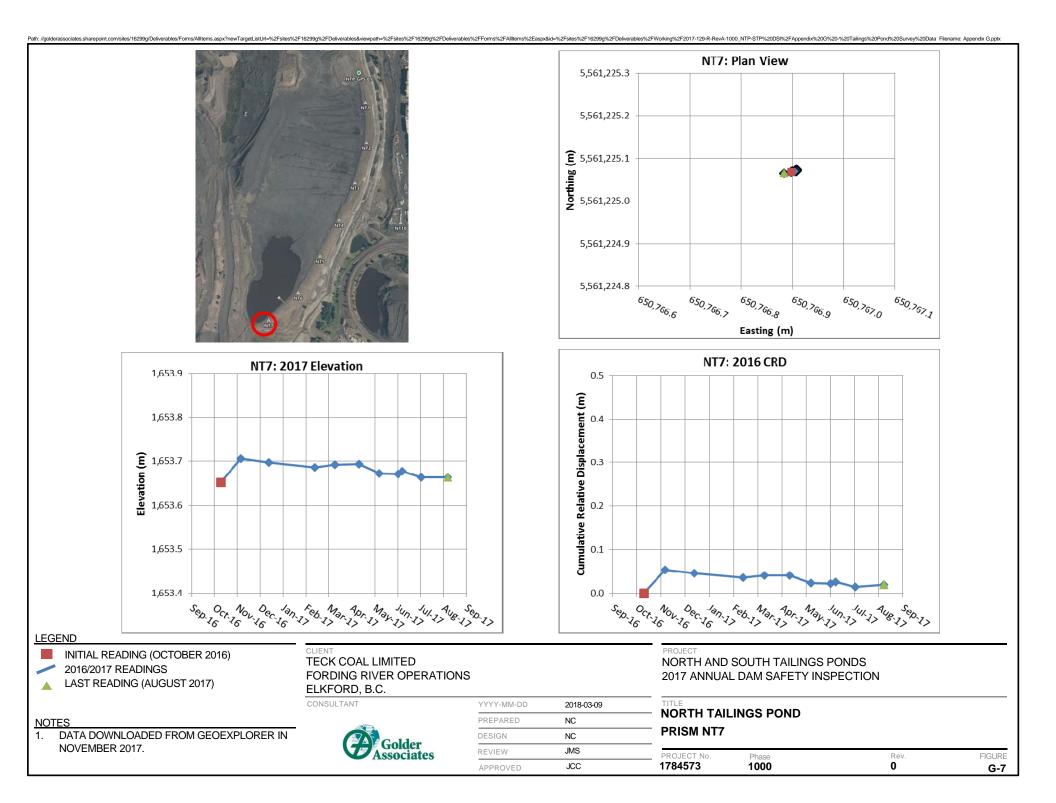


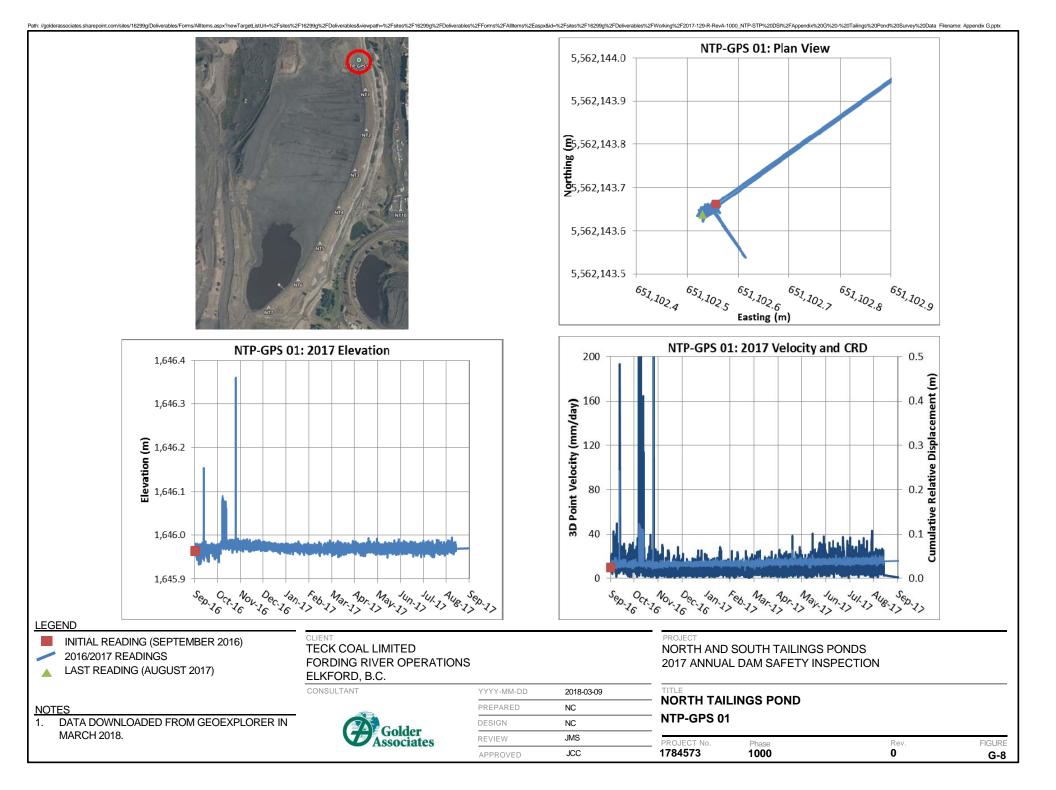


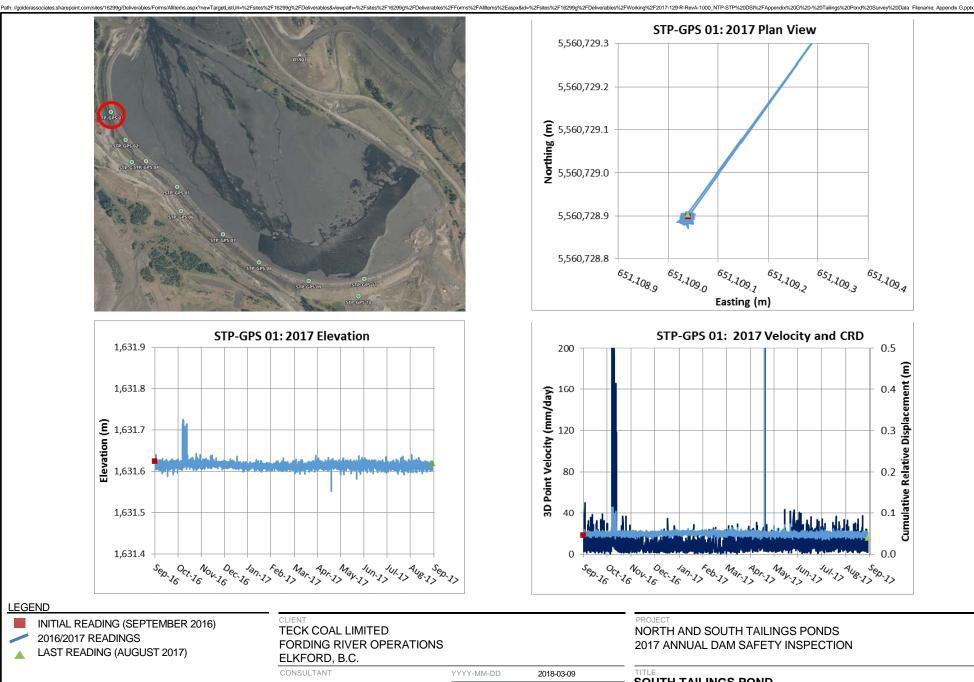














DATA DOWNLOADED FROM GEOEXPLORER IN 1. NOVEMBER 2017.



YYYY-MM-DD	2018-03-09
PREPARED	NC
DESIGN	NC
REVIEW	JMS
APPROVED	JCC

NORTH AND SOUTH TAILINGS PONDS 2017 ANNUAL DAM SAFETY INSPECTION

Apr.1>

SOUTH TAIL STP-GPS 01	INGS POND		
PROJECT No.	Phase	Rev.	FIGUR
1784573	1000	0	G-9

May-12 10

^{651,109.3}

Latab detail

141.17

651,109.4

0.5

0.4

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0.2

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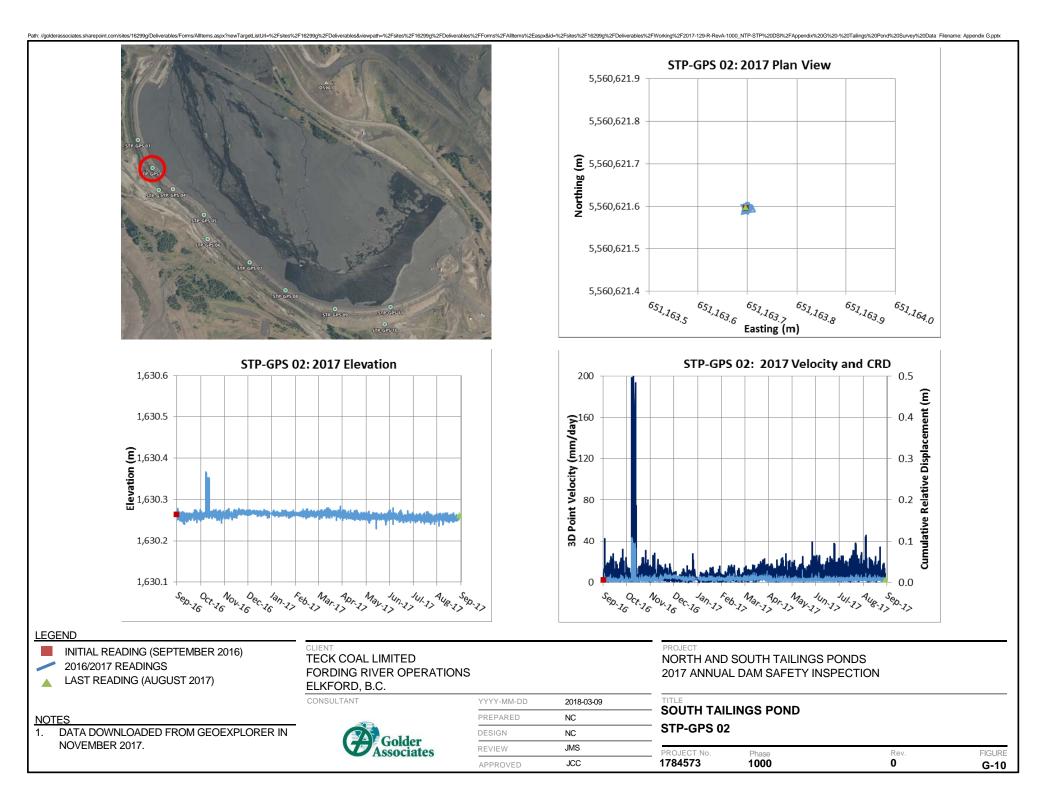
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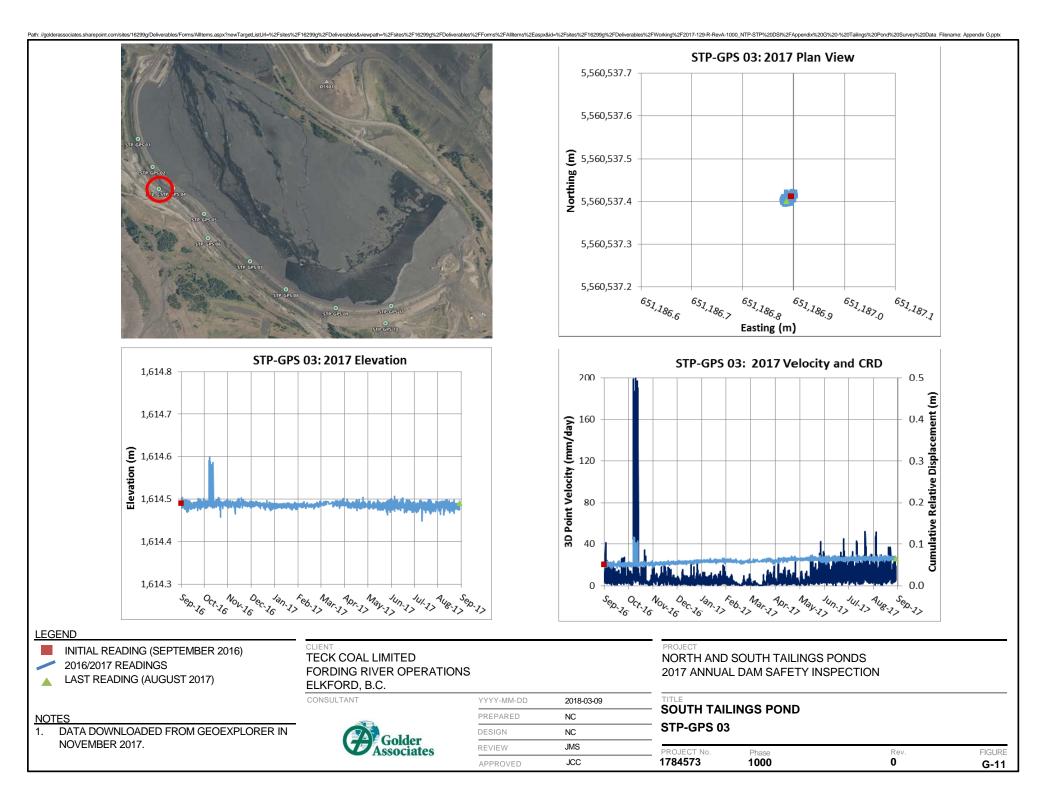
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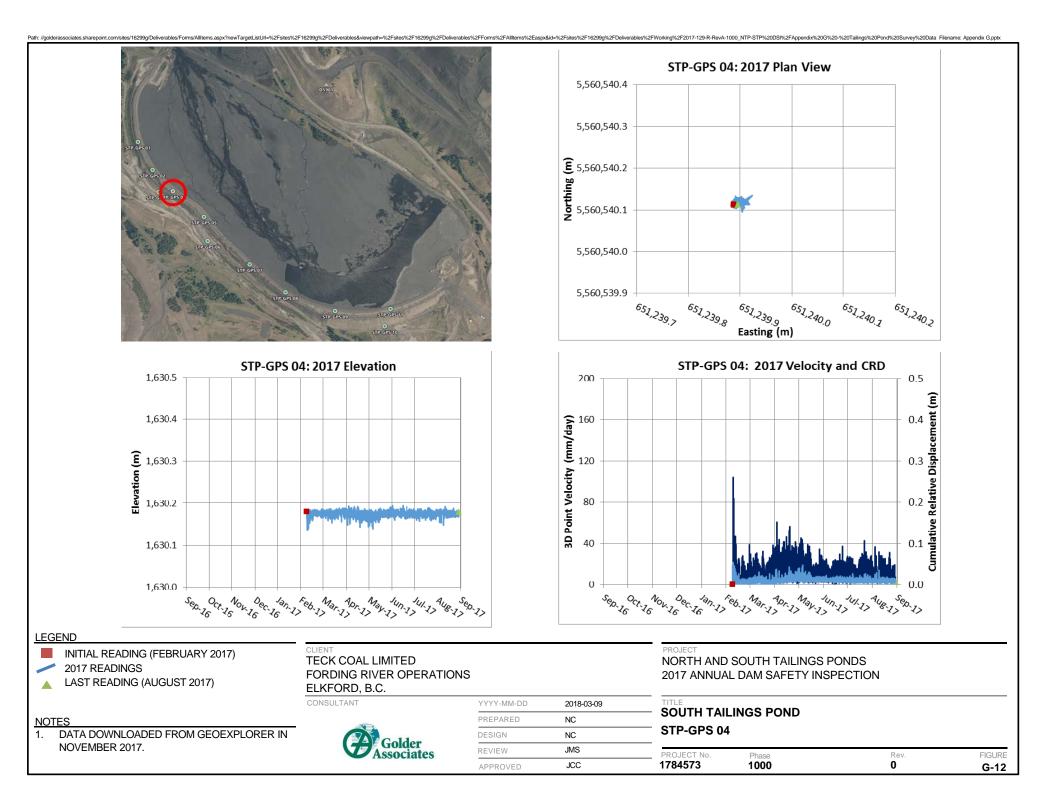
Aug-1>

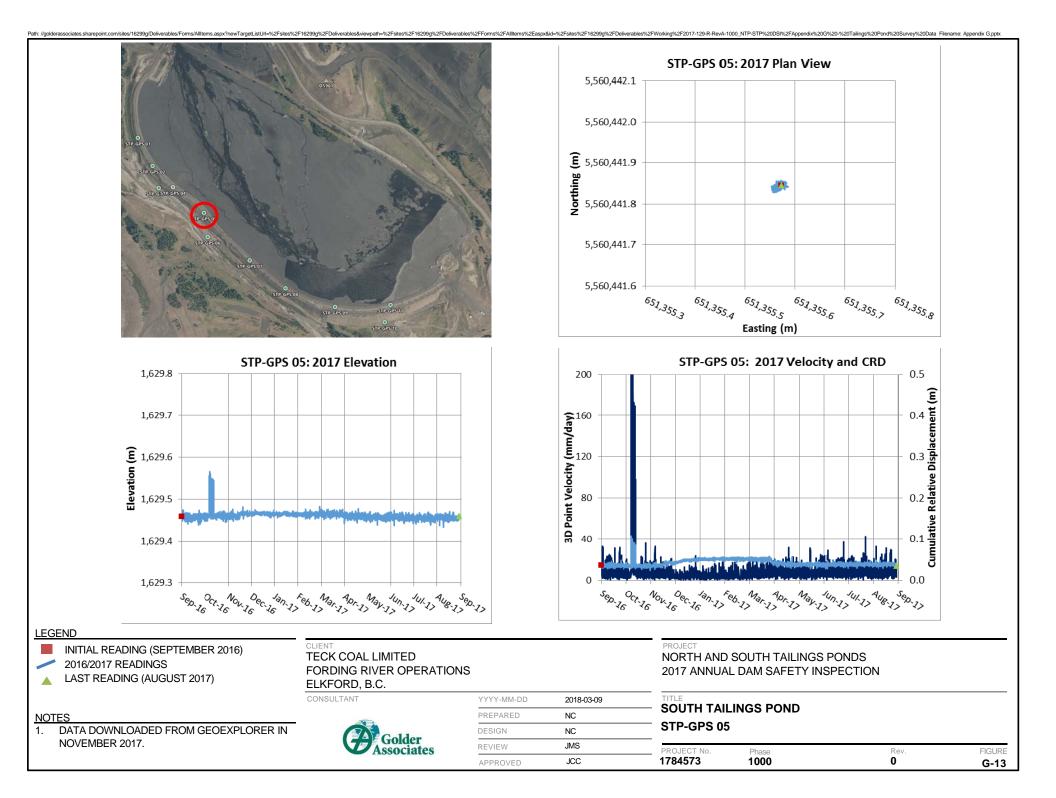
Cumulative Relative Displacement (m)

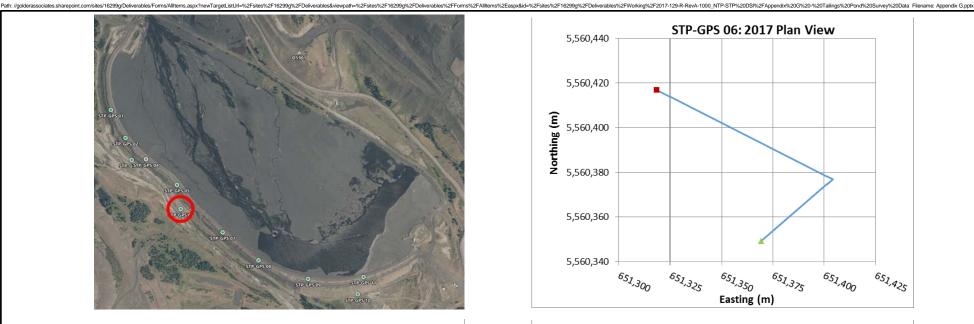
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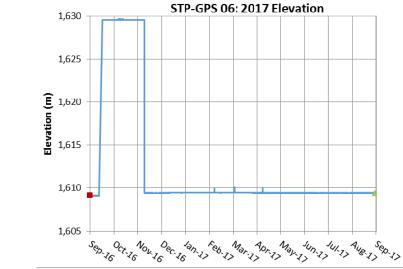












TECK COAL LIMITED

ELKFORD, B.C. CONSULTANT

FORDING RIVER OPERATIONS

Golder Associates

YYYY-MM-DD

PREPARED

DESIGN

REVIEW

APPROVED

2018-03-09

NC

NC

JMS

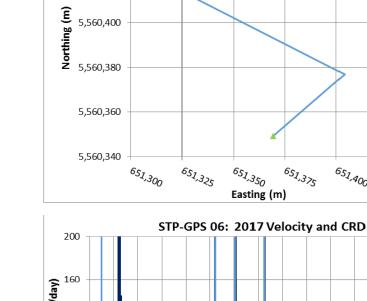
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LEGEND

- **INITIAL READING (SEPTEMBER 2016)** 2016/2017 READINGS
- LAST READING (AUGUST 2017)

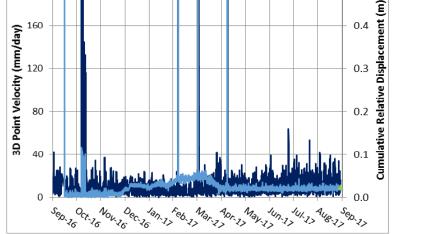
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- DATA DOWNLOADED FROM GEOEXPLORER IN 1. NOVEMBER 2017.
- 2. SCALE OF PLAN VIEW AND ELEVATION PLOTS ENLARGED TO SHOW MOVEMENT OF GPS



5,560,440

5,560,420



STP-GPS 06: 2017 Plan View

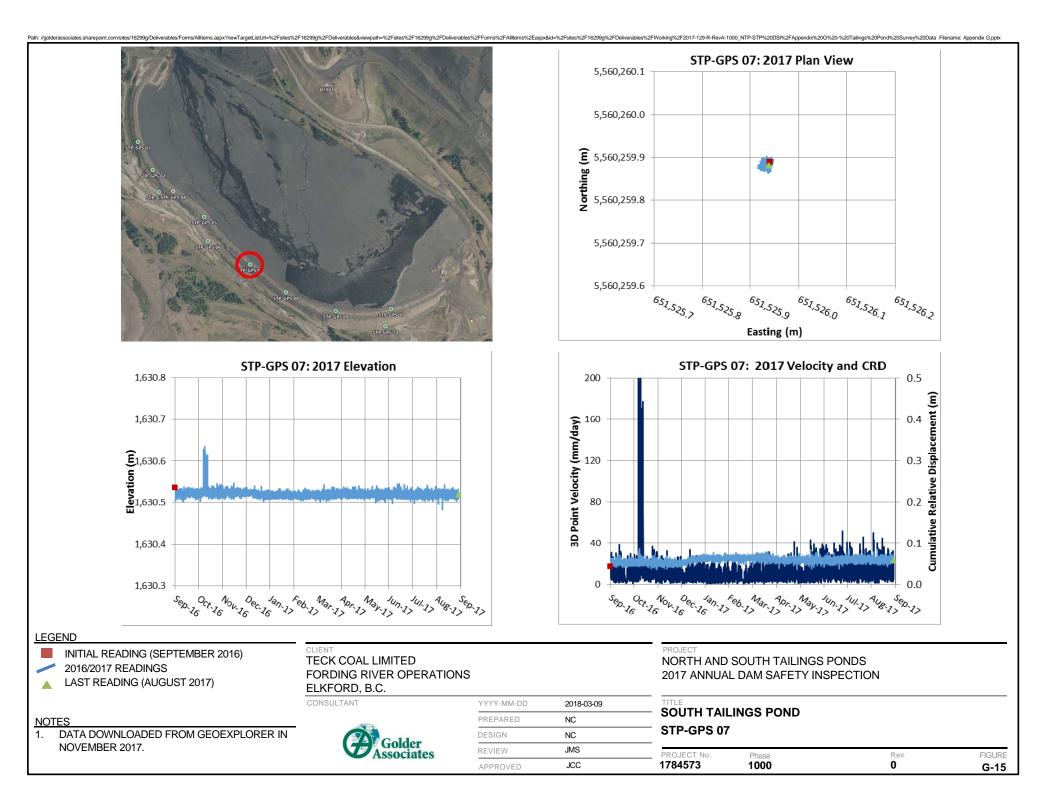
PROJE NORTH AND SOUTH TAILINGS PONDS 2017 ANNUAL DAM SAFETY INSPECTION

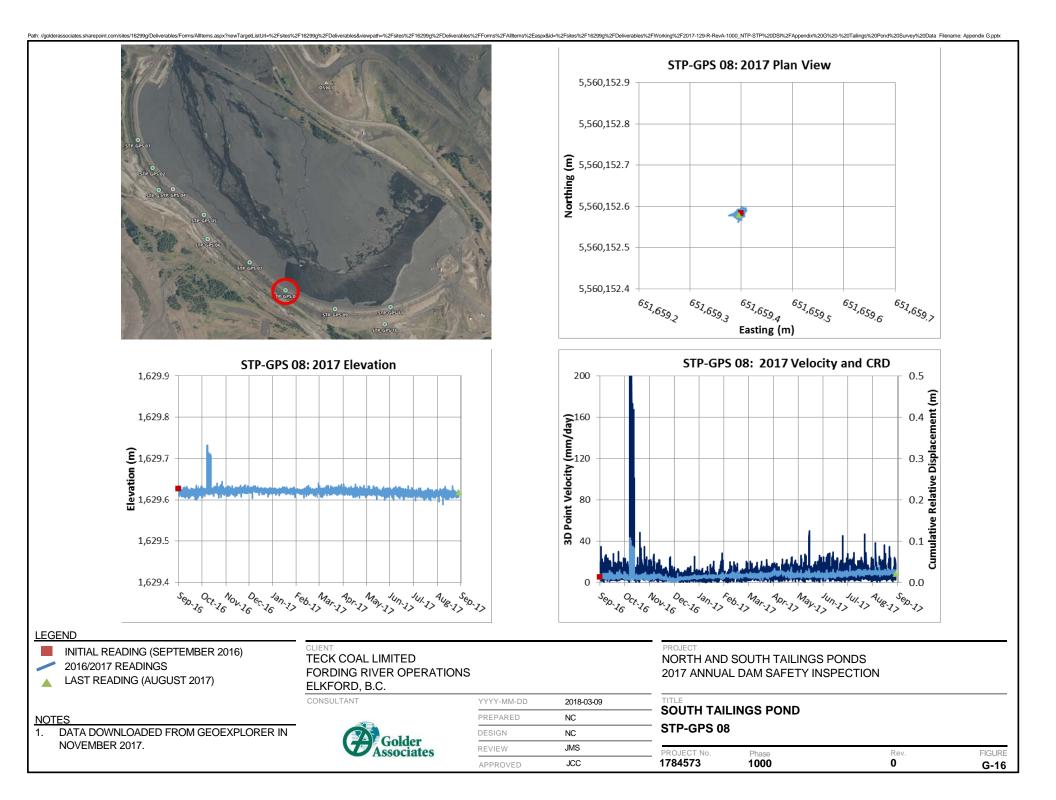
SOUTH TAIL STP-GPS 06	LINGS POND		
PROJECT No.	Phase	Rev.	FIGU
1784573	1000	0	G-1

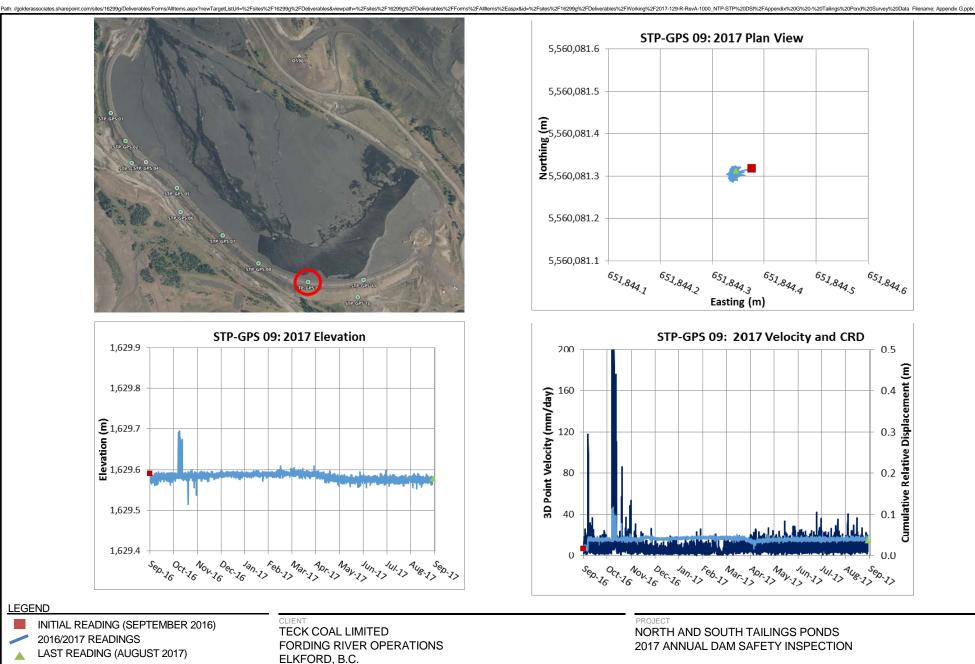
65_{1,400}

⁶⁵1,425

0.5







⁶⁵1,844.4 ⁶⁵1,844.5 ^{651,844.6} 651,844.3 Easting (m) STP-GPS 09: 2017 Velocity and CRD 0.5 Cumulative Relative Displacement (m) 0.4 0.3 0.2 0.1 أرابه ألليه بتقاليا ورازم ف

NOTES

DATA DOWNLOADED FROM GEOEXPLORER IN 1. NOVEMBER 2017.



YYYY-MM-DD	2018-03-09
PREPARED	NC
DESIGN	NC
REVIEW	JMS
APPROVED	JCC

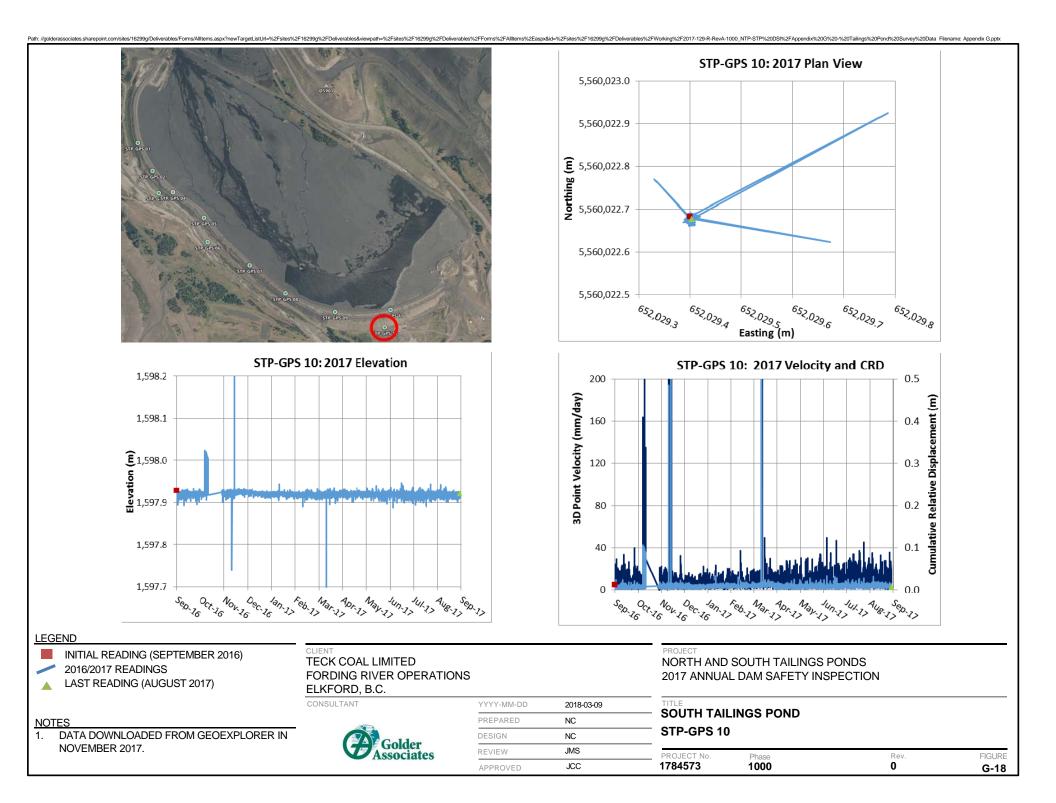
NORTH AND SOUTH TAILINGS PONDS 2017 ANNUAL DAM SAFETY INSPECTION

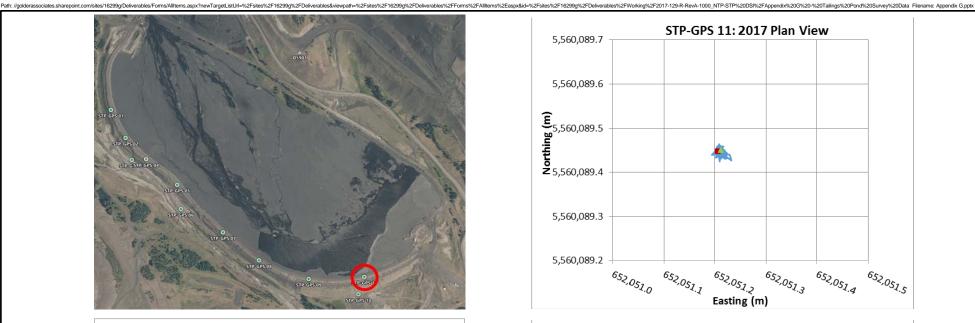
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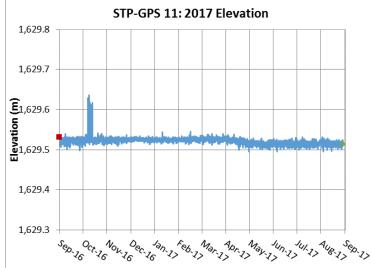
SOUTH TAIL	LINGS POND)		
PROJECT No.	Phase	Rev.	FIGUR

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Sep.1> AU8-17







CLIENT

LEGEND

INITIAL READING (SEPTEMBER 2016) 2016/2017 READINGS

LAST READING (AUGUST 2017)

NOTES

DATA DOWNLOADED FROM GEOEXPLORER IN 1. NOVEMBER 2017.

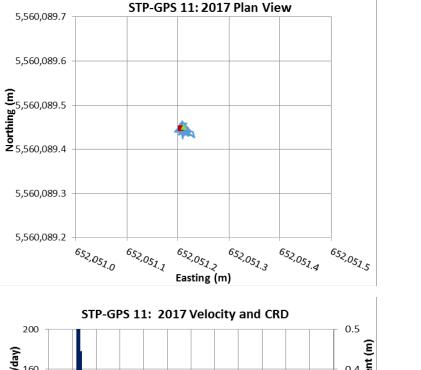


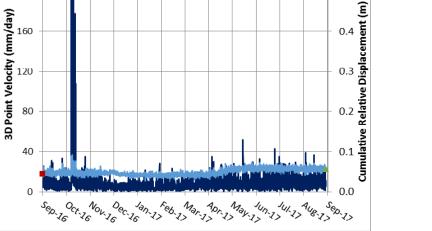
FORDING RIVER OPERATIONS

TECK COAL LIMITED

ELKFORD, B.C. CONSULTANT

YYYY-MM-DD	2018-03-09	
PREPARED	NC	
DESIGN	NC	
REVIEW	JMS	
APPROVED	JCC	_





PROJEC NORTH AND SOUTH TAILINGS PONDS 2017 ANNUAL DAM SAFETY INSPECTION

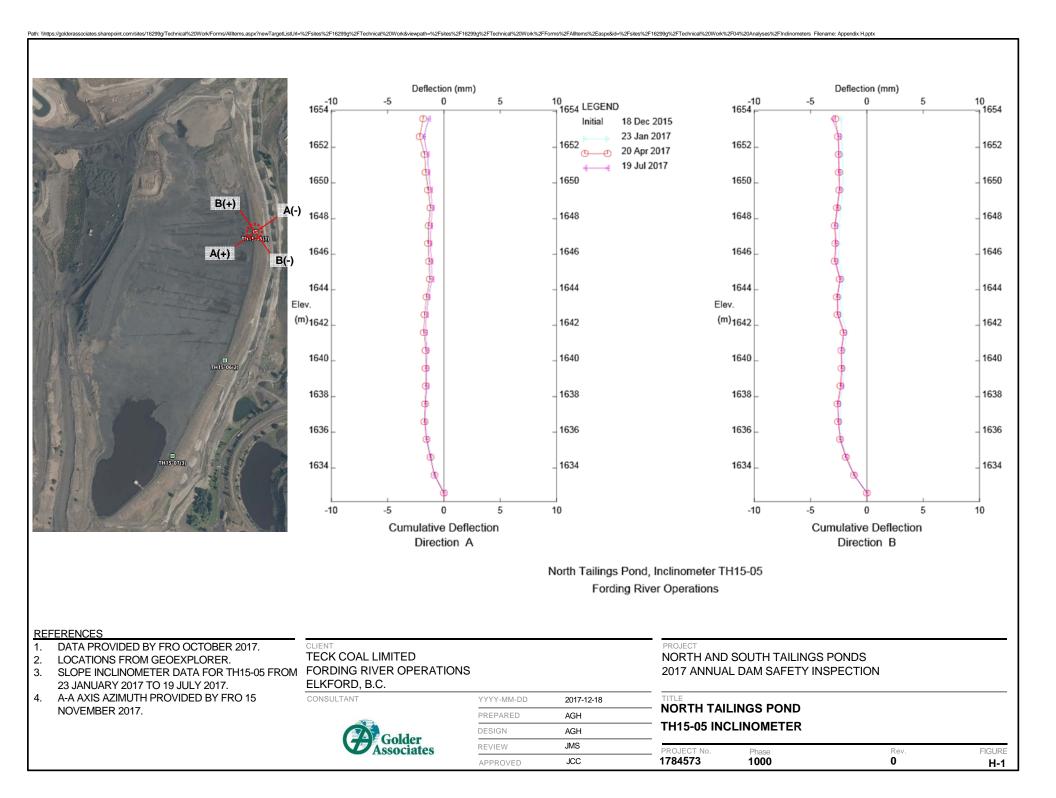
SOUTH TAIL STP-GPS 11	LINGS POND		
PROJECT No.	Phase	Rev.	FIGURE

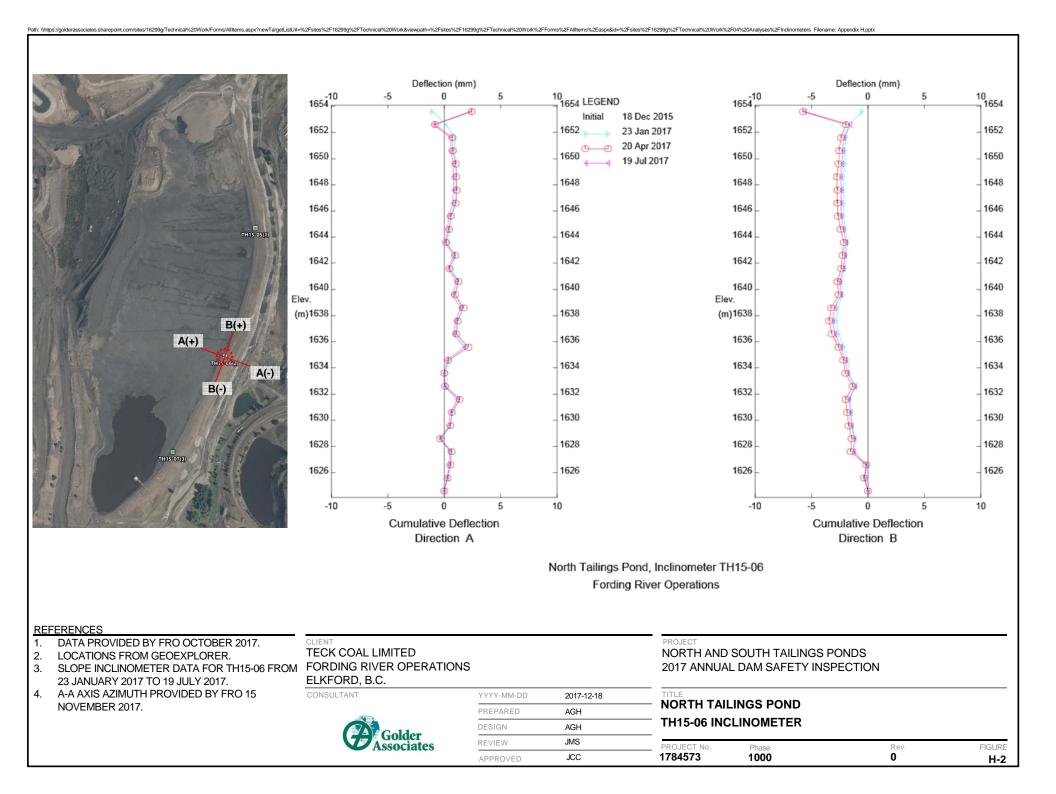


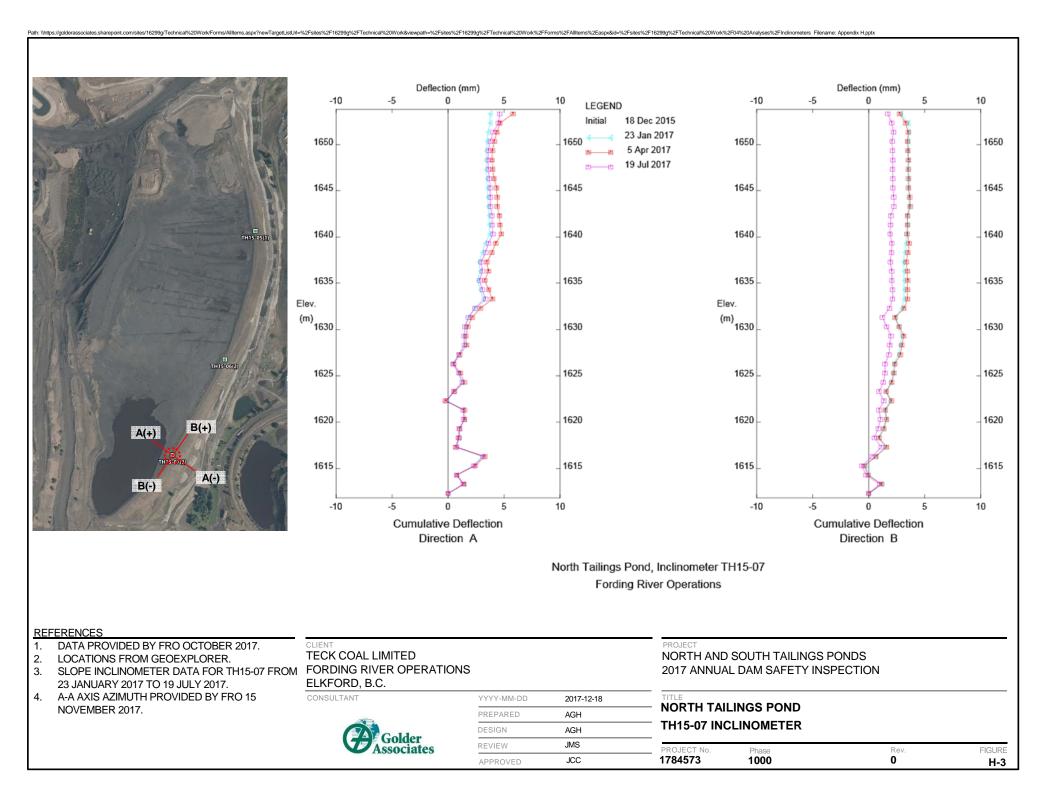
APPENDIX H

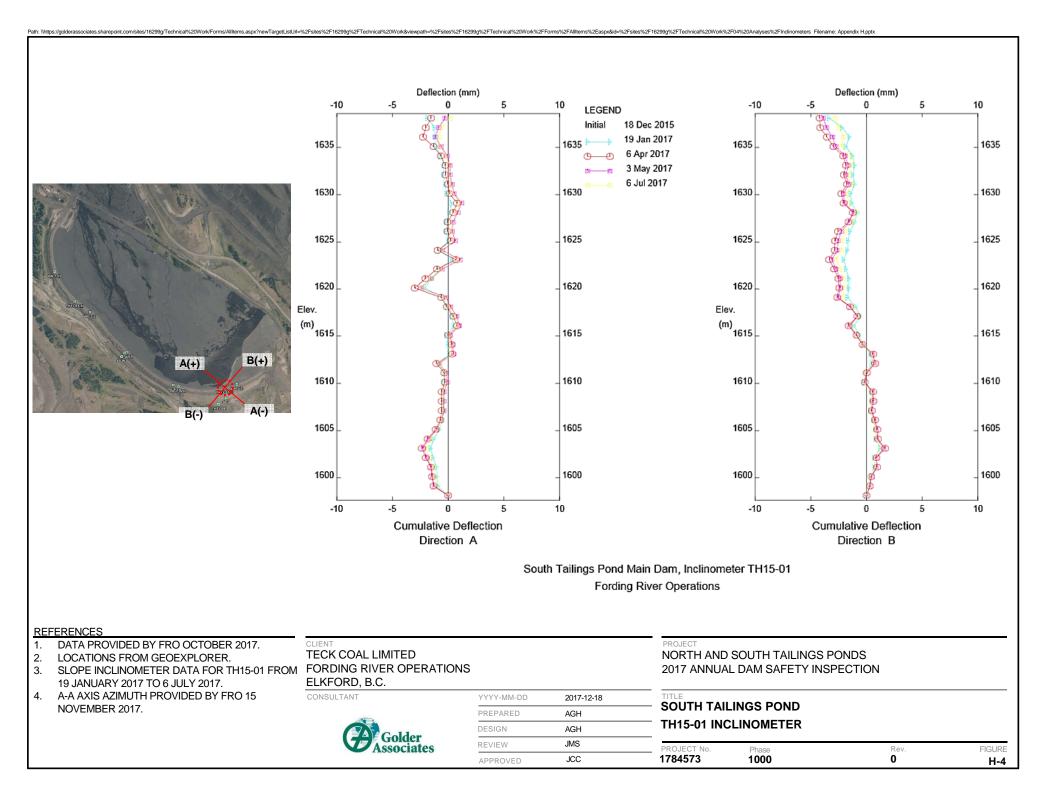
Tailings Pond Inclinometer Data

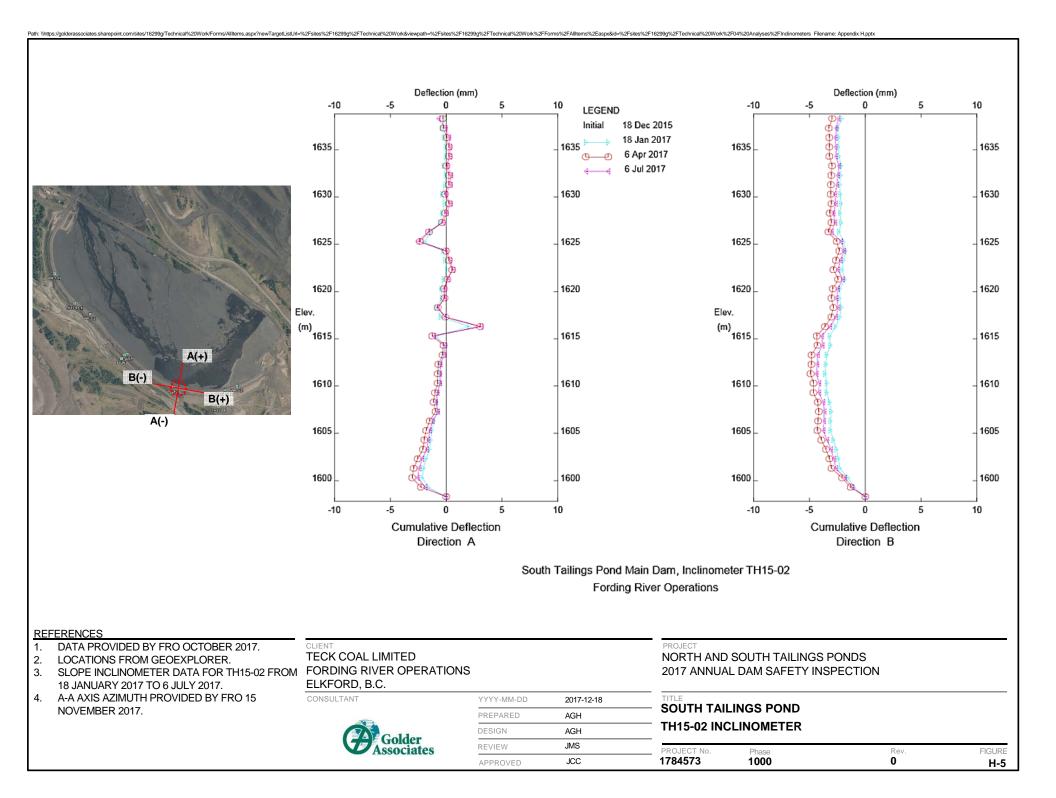


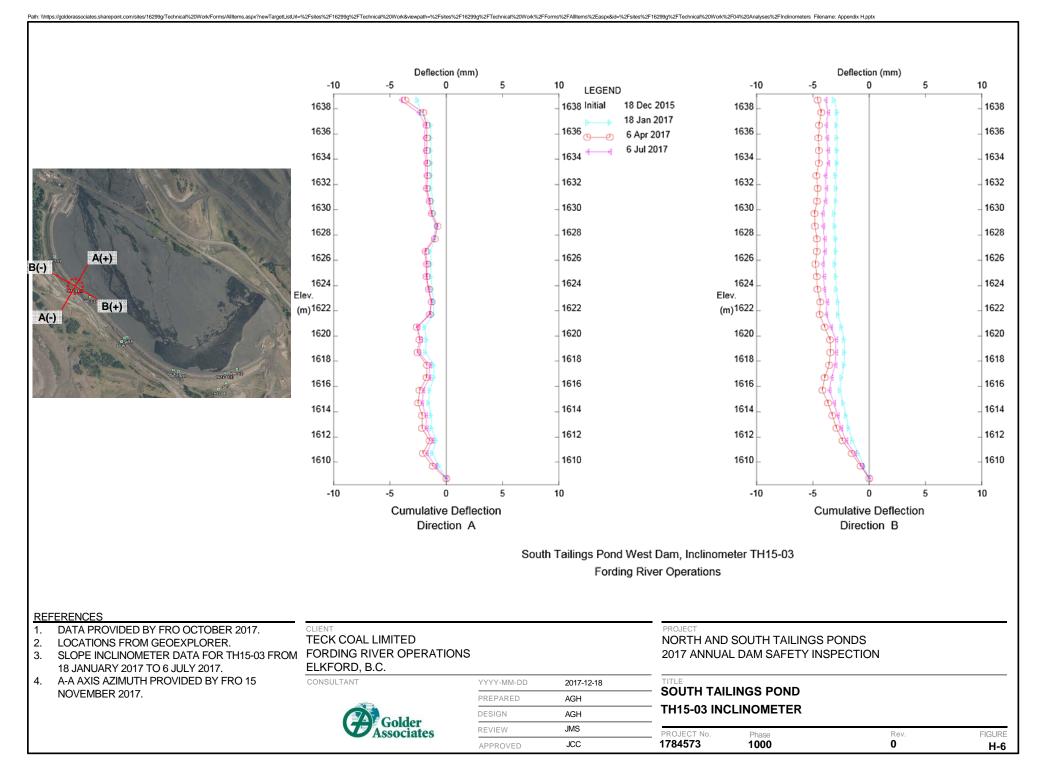


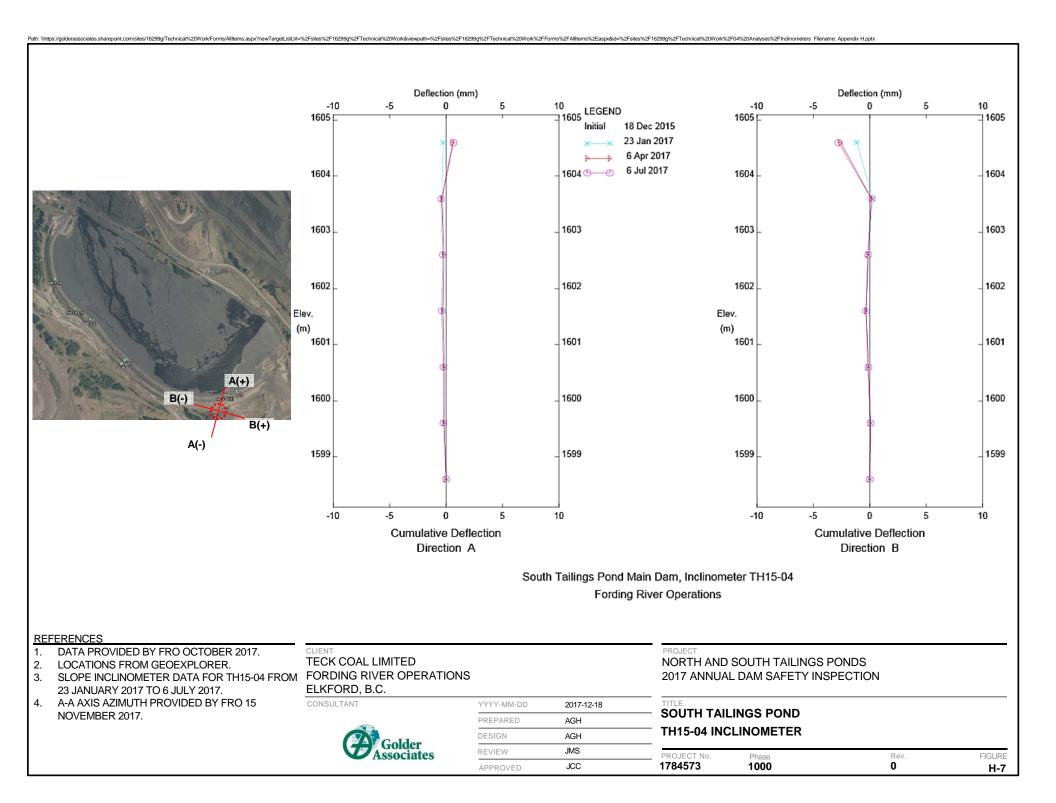














APPENDIX I

2017 Riprap Inspection by Kerr Wood Leidal Associates Ltd.





Okanagan 202 - 3334 30th Avenue Vernon, BC VIT 2C8 T 250 503 0841 F 250 503 0847

Technical Memorandum

DATE: March 25, 2018

- TO: Heather Brickner Teck Coal Limited
- CC: Julia Steele, P.Eng. Golder Associates Ltd.

FROM: Jason Miller, P.Eng.

RE: TECK COAL LIMITED – FORDING RIVER OPERATIONS 2017 NTP and STP Riprap Inspection Our File 0008.245-300

Introduction

Teck Coal Ltd. – Fording River Operations (FRO) retained Kerr Wood Leidal Associates Ltd. (KWL) to complete an inspection of the riprap along the North Tailings Pond (NTP) and South Tailings Pond (STP). Jason Miller, P.Eng. of KWL is the design engineer of record for bank protection works along the NTP and STP.

The riprap inspection is a component of the Annual Dam Safety Inspection (DSI) currently being completed by Golder Associates Ltd. (Golder). Golder is the Engineer of Record (EoR) for the tailings facilities at FRO. This technical memorandum summarizes the findings of KWL's riprap inspection and will be appended to the Golder 2017 Annual DSI.

Background

KWL has a long history working at FRO. KWL was involved in the design and construction of the Fording River diversion to allow the construction of the STP. KWL has also provided hydrotechnical support to FRO following major flood events on the Fording River.

A severe flood on the Fording River in June 2013 caused extensive damage to FRO infrastructure, and necessitated emergency mitigation works. Post-flood works included design and construction of a new riprap revetment to protect the NTP and part of the STP. Construction of bank protection works occurred in 2013 and 2014. Upon completion, continuous bank protection works had been constructed along the Fording River channel where it flows along the toe of the NTP dam, and along about one-third of the channel where it flows along the toe of the STP dam.



In 2016, KWL updated the Fording River hydraulic model and designed upgrades to the 2013 bank protection works that would further protect the NTP and STP against high flows on the Fording River¹. Based on the high priority assigned to these upgrades and feasibility constraints for 2016 construction, design proceeded using the 200-year return period flood. FRO is continuing a parallel process to establish an appropriate Fording River design flow for long-term upgrading and operation of its tailings dams.

The 2016 bank protection works for NTP and STP were constructed through two construction seasons, 2016 and 2017.

Activities Since 2016 Riprap Inspection

In August/September 2017, the remaining NTP bank protection works were completed as per the 2016 design (KWL, 2016). This work included:

- Excavating and placing riprap to scour depth from Sta. 0+165 to 0+205;
- Raising the existing riprap to design height and transitioning to existing ground between Sta. 0+930 and 1+075; and
- Raising a section of riprap to design height between Sta. 0+510 and 0+580.

Stationing cited above refers to the Golder Tailings Dam baseline shown in Figure 1.

The works were reviewed by KWL during construction to confirm general conformance with the design. The bank protection generally meets the design gradation with an average size of 1,200 mm and a thickness of 1.8 m to 2.4 m. The rock was sourced from mining operations at FRO and is expected to have similar quality as the rock placed in previous years. Rock that appeared to be weathering or broke easily when handled was discarded during riprap placement. The buried section of riprap is set back from the river and has a 2H:1V slope. Sections of the riprap that were raised in 2017 comprise the river bank and have a 1.5H:1V slope.

Field Inspection

An initial site visit was conducted on September 12, 2017 by Jason Miller, P.Eng. of KWL to assess the condition of the NTP and STP riprap bank protection works. The assessment began at the north end of the NTP and moved downstream to the STP. During the initial inspection, the 2017 riprap upgrades were in progress at the south end of the NTP. The initial assessment therefore focussed on the condition of the 2016 riprap works. A supplemental inspection was conducted on October 4, 2017 to review the completed 2017 riprap works.

NTP Inspection

Riprap extends from upstream of the NTP to about Sta. 1+075 of the Golder NTP dam baseline as shown on Figure 1. Visual inspection of the lower riprap slope was impeded by gravel placed over the riprap during 2013 construction. The upper riprap slope placed during 2016/2017 was visible and appears to be well-interlocked. The exposed toe of the revetment was also observed and appears to be in good condition with no visual signs of scour or displacement. Riprap was visible on the entire slope in one short section approximately between Sta. 0+750 and Sta. 0+830. The exposed riprap is in good condition and appears to remain well interlocked. The slope of the exposed riprap is about 1.5H:1V.

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¹ Kerr Wood Leidal Associates Ltd. 2016 Bank Protection Design for NTP/STP – Design Brief. Prepared for Teck Coal Ltd. – Fording River Operations. January 2017.



Gravel-covered sections of the revetment were checked for signs of movement such as cracks or openings in the gravel along the slope that would indicate voids developing within the revetment or settlement of the upper riprap. No visual signs of movement were observed over the length of the revetment.

The buried section of riprap (Sta. 0+100 to 0+200) is not visible. The ground covering the riprap was checked for signs of movement such as cracks or settlement. No visual signs of movement were observed over the length of the buried riprap.

Some of the locally-supplied rock is known to weather and degrade. This year's inspection did not include any test holes to review rock degradation below the visible rock layer. However, work completed on raising the riprap at the south end of the protection work (Sta. 0+930 to 1+075) required exposing the top of the riprap placed in 2013. The exposed riprap appeared to well-interlocked with smaller material filling the voids between the riprap and did not appear to show significant signs of weathering.

Generally, the NTP riprap has been constructed to +/- 0.1 m of the design elevation; however, there are a few areas where the riprap is up to 0.4 m lower than the design elevation (refer to profile on record drawings in in completion report²). This reduces the freeboard in these areas from the design freeboard of 1.0 m to 0.6 m. A reduced freeboard means that the revetment has a reduced capacity to handle variations from the design conditions; 0.6 m freeboard is considered the minimum acceptable freeboard for many flood protection projects throughout BC. Particular attention should be paid to these areas on regular inspection. Signs of settling or subsidence should be confirmed by survey and levels of protection should be raised if required. FRO should take advantage of future opportunities to cost-effectively achieve the design freeboard (e.g., if future work is required along the river side slope of the NTP).

STP

A riprap revetment protects the STP embankment toe from Sta. 0+240 to 0+685 of the Golder STP dam baseline (refer to Figure 2). Most of the riprap slope is exposed and visible along the length of the revetment, with the exception of a 20 m length at the upstream end which is covered in finer rock (200 mm minus riprap). The riprap is well interlocked with smaller riprap filling the voids of the larger riprap. The riprap slope is about 2H:1V.

The top of the riprap apron is covered in river gravel and is not visible for inspection; its condition is assumed similar to that observed along the revetment slope. The gravel-covered apron was checked for signs of movement such as cracks or openings in the gravel that would indicate voids or settlement developing within the toe apron. No signs of movement were observed. The Fording River currently flows on the opposite side of the channel for most of the length with the exception of the downstream end where the floodplain narrows to the edge of the channel. The Fording River was not flowing directly against the riprap during the inspection.

There is some weathering (cracking and flaking) of individual riprap pieces along the entire length of the STP protection works. Currently, the degradation is intermittent and has not affected the overall integrity of the protection works; however, should additional rock continue to degrade, the average size (mass) of the riprap will decrease and rock interlocking may be compromised. Both of these processes can reduce the level of protection provided by the riprap. Remedial work may be required if future inspections confirm ongoing weathering and degradation.

² Kerr Wood Leidal Associates Ltd. 2016/2017 Bank Protection for NTP/STP – Completion Report. Prepared for Teck Coal Ltd. – Fording River Operations. December 2017.



General Observations

All riprap used for NTP and STP bank protection works was salvaged from toes of spoils or sorted from spoils or hauled directly from the pit. The resistance to weathering is therefore expected to vary locally throughout both revetments. Over time, inspections may identify pockets of more resistant and/or less resistant material. More frequent monitoring should occur in areas where a significant portion of the riprap slope (i.e., more than the occasional rock) is found to be showing signs of degradation. Each annual inspection should review the inspection history and highlight potential changes.

Summary and Recommendations

Exposed riprap along the NTP and STP is generally in good condition and is designed to provide erosion protection during the 200-year return period flood. There is the occasional riprap piece that has degraded from weathering. These pieces are located intermittently along the length of the STP. This is expected to be the case for NTP riprap as well. The field assessment did not identify any evidence that raises concerns about the performance of concealed (i.e., buried or gravel-covered) riprap, and its condition is assumed to be comparable or better than that of equivalent exposed sections.

Inspections of the riprap should be completed at least annually. The riprap should continue to be monitored for weathering during these annual inspections. Mitigative action (e.g., riprap replacement) may be required if several rocks in close proximity to one another show evidence of degradation. Supplementary inspections should be conducted after high water events on the Fording River, which could include freshet or precipitation driven events. Any deficient sections should be repaired as soon as possible to limit further degradation and risk to the NTP or STP.

There are a few areas along the NTP riprap where the riprap is up to 0.4 m lower than the design elevation. This reduces the freeboard in these areas to 0.6 m. Particular attention should be paid to these areas on regular inspection. Signs of settling or subsidence should be confirmed by survey and levels of protection should be raised if required. Teck should seek opportunities to cost-effectively achieve the intended 1 m freeboard (e.g., by combining with an independent but adjacent construction project).

Design of the riprap erosion protection works is based on the 200-year return period flood, which is subject to numerous uncertainties. For example, the energy of the flood can significantly change channel conditions. In addition, larger floods are possible, including the breach of an upstream valley-spanning structure, the Fording River Multiplate embankment. The design and status of the NTP and STP riprap should be reviewed and revised as needed within the context of FRO's larger review of design and performance requirements for the NTP and STP tailings dams.

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Closure

We trust this provides a satisfactory assessment of the riprap protection along the NTP and STP. Should you have any questions, please contact the undersigned.

KERR WOOD LEIDAL ASSOCIATES LTD.

Prepared by:



Reviewed by:

David Roche, M.A.Sc., P.Eng. Senior Water Resources Engineer

Jason Miller, P.Eng. Water Resources Engineer

JM/

Encl.: Photos, Figure 1, Figure 2

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Revision History

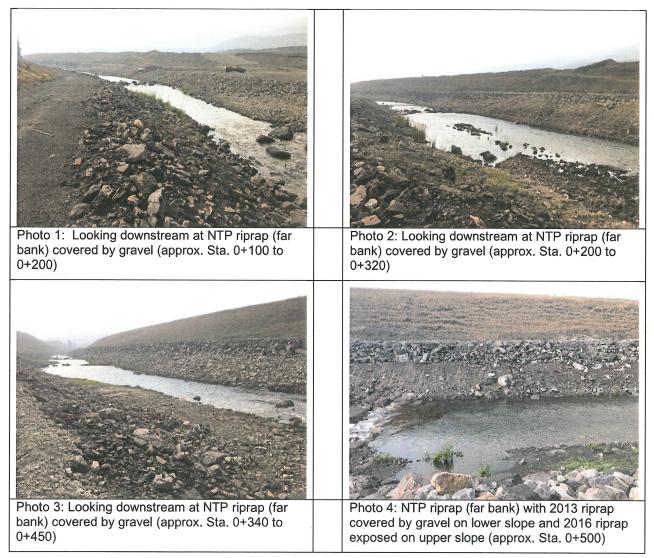
Revision #	Date	Status	Revision Description	Author
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OQM Organizational Quality

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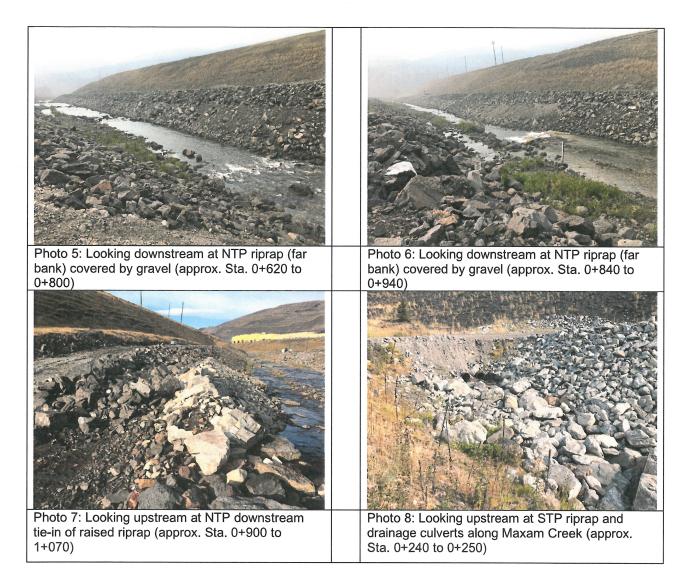


Photos



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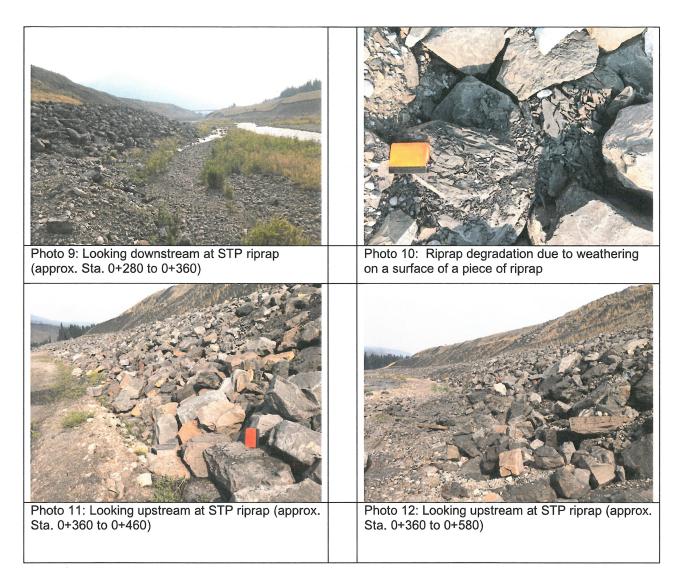




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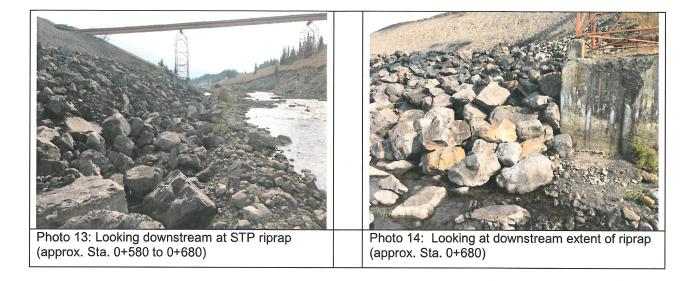




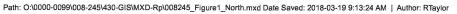
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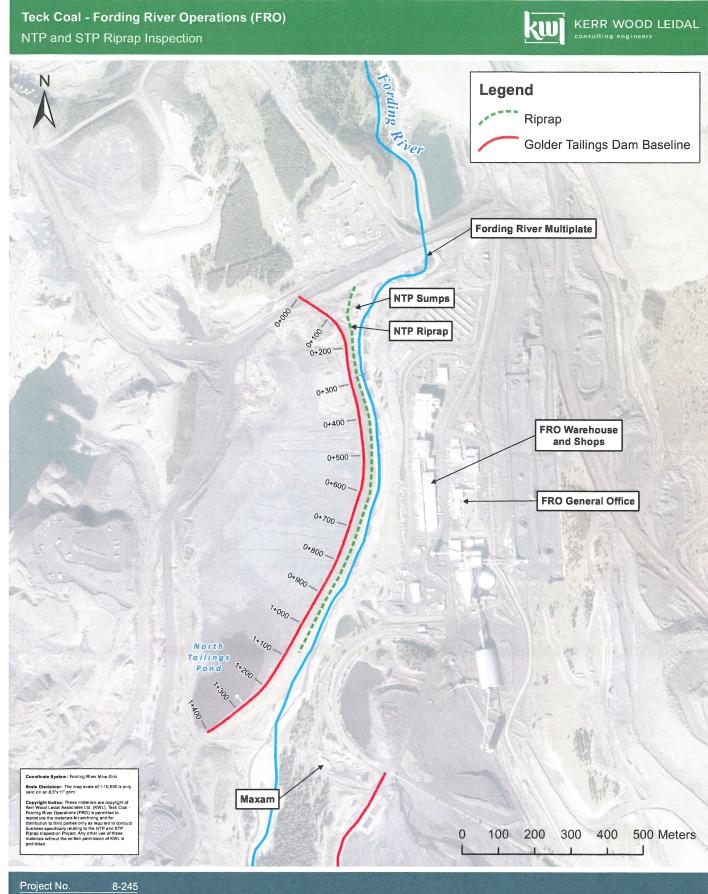
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North Tailings Pond

Date

Scale

March 2018

1:10,000

Figure 1

1:10,000

Scale

Teck Coal - Fording River Operations (FRO) KERR WOOD LEIDAL KW NTP and STP Riprap Inspection Legend Golder Tailings Dam Baseline Riprap -----Fording River FRO Warehouse and Shops Maxam 0+000 -0+233 -0+200 -0+100 0+100 0+200 0+300 STP Riprap 0+400 FRO Gate House 0+500 0+600 South Tailings 0+700 Pond 0+800 0+900 1+000 1+100 1+200 1+300 Fording River Road 1*400 Coordin ate System: Fording River Mine Grid +800 1+863 Scale Discialmer: The map scale of 1:10,000 is only valid on an 8.5*x11* print. e materials are copyright of ates Ltd. (KWL). Teck Coal -i (FRO) is permitted to or archide-Copyright Korr Week 0 100 200 300 400 500 Meters 8-245 Project No. March 2018 Date

South Tailings Pond

Figure 2

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